

**Drainage Conformance Letter**  
**For**  
**CONVENANCE STORE AND CAR WASH**  
**LOT 1 AND LOT 2 OF CHAMBERS AND HESS, FILING NO. 1**  
**SEC OF CHAMBERS RD AND S. RED SKY DR, PARKER, COLORADO**

05/27/2021

**Prepared For Developer:**

**Twin Star Energy, LLC**

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Contact: Mark Perrino  
303-898-2603

**Prepared By:**



**EES**

**Entitlement and Engineering  
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**ENGINEERS STATEMENT**

This Drainage Conformance Letter for Lot 1 and Lot 2 of Chambers and Hess, Filing No. 1 Development, was prepared by me or under my direct supervision in accordance with the provisions of the Town of Parker Storm Drainage & Environmental Criteria Manual. I understand that the Town of Parker and its designated town authority do not and will not assume liability for drainage facilities designed by others.

Landis Gordon PE

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**Colorado P.E. License No. 56446**

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**Seal and Date**

### **Introduction**

This Drainage Conformance Letter has been prepared for the proposed Convenience Store and care wash located at Lot 1 and Lot 2 of Chambers and Hess Filing No. 1. The purpose of this letter is to show that the proposed development conforms to the Storm Drainage and Environmental Criteria Manual for the Town of Parker, and the Chambers and Hess Final Drainage Report, prepared by Rick Engineering, and dated January 25, 2021, hereinafter called the "Master Drainage Report." This is the underlying Master Plan for the overall development encompassing the site. All infrastructure will be installed prior to this project's construction and is labeling "existing" for purposes of this report.

### **Location**

The project site located at the southeast corner of Chambers Road and South Red Sky Drive, Parker, Colorado. This development is located in the Southeast Quarter of Section 29, Township 6 South, Range 66 West 6th P.M. Town of Parker, County of Douglas, State of Colorado. The site is bounded by Sliceroo Drive to the east, South Red Sky Drive to the north, Chambers Road to the west, and a vacant lot to the south.

### **Existing Conditions**

Lot 1 occupies approximately 0.93 acres of overlot graded vacant land, covered with native grasses and weeds. The project site was studied in the Master Drainage Report, this site lies within Basin A1.

Lot 2 occupies approximately 0.67 acres of overlot graded vacant land, covered with native grasses and weeds. The project site was studied in the Master Drainage Report, this site lies within Basin A2.

The existing site generally slopes Northwest to Southeast. As part of the Chambers and Hess master planned development, internal streets will be built East, adjacent to this Lot. The existing underground storm drain system of the Overall Development will convey stormwater to an existing detention pond A from the Final Drainage Report for the Parker 234 Subdivision.

### **Proposed Conditions**

An existing stormwater line is located along the east side of the property. This line has an existing area inlets on the east side of Lot 1 and the east side of Lot 2. Only the Lot 2 existing area inlet will be re-used for this proposed design. A proposed Type R curb inlet will be cut into this existing storm line on the south

curb line of the east access off Sliceroo. Building downspouts will be piped underground to the storm inlets. The Convenience store and Car wash drives and parking lots will flow to the existing storm sewer system. This existing storm line will serve as the discharge from the site, in the proposed condition. The existing storm sewer system has been sized to receive flows from the developed Lots.

The impervious percentage assigned to basin A1 and A2 was 75%. The impervious percentage for the proposed development is calculated to be 56%.

The proposed 9.96 cfs calculated for the Convenience Store and Car Wash development is less than the masterplan runoff is 10.80 cfs.

### **Conclusion**

Since the proposed development is lower in both imperviousness and runoff than masterplanned for the Parker 234 Subdivision, the existing drainage facilities are sufficient to convey the runoff from the proposed Lot 1 and Lot 2 sites, without onsite water quality or detention, and not negatively impact adjacent properties.

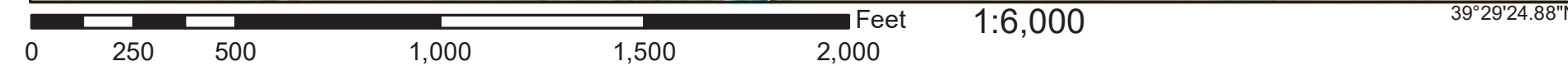
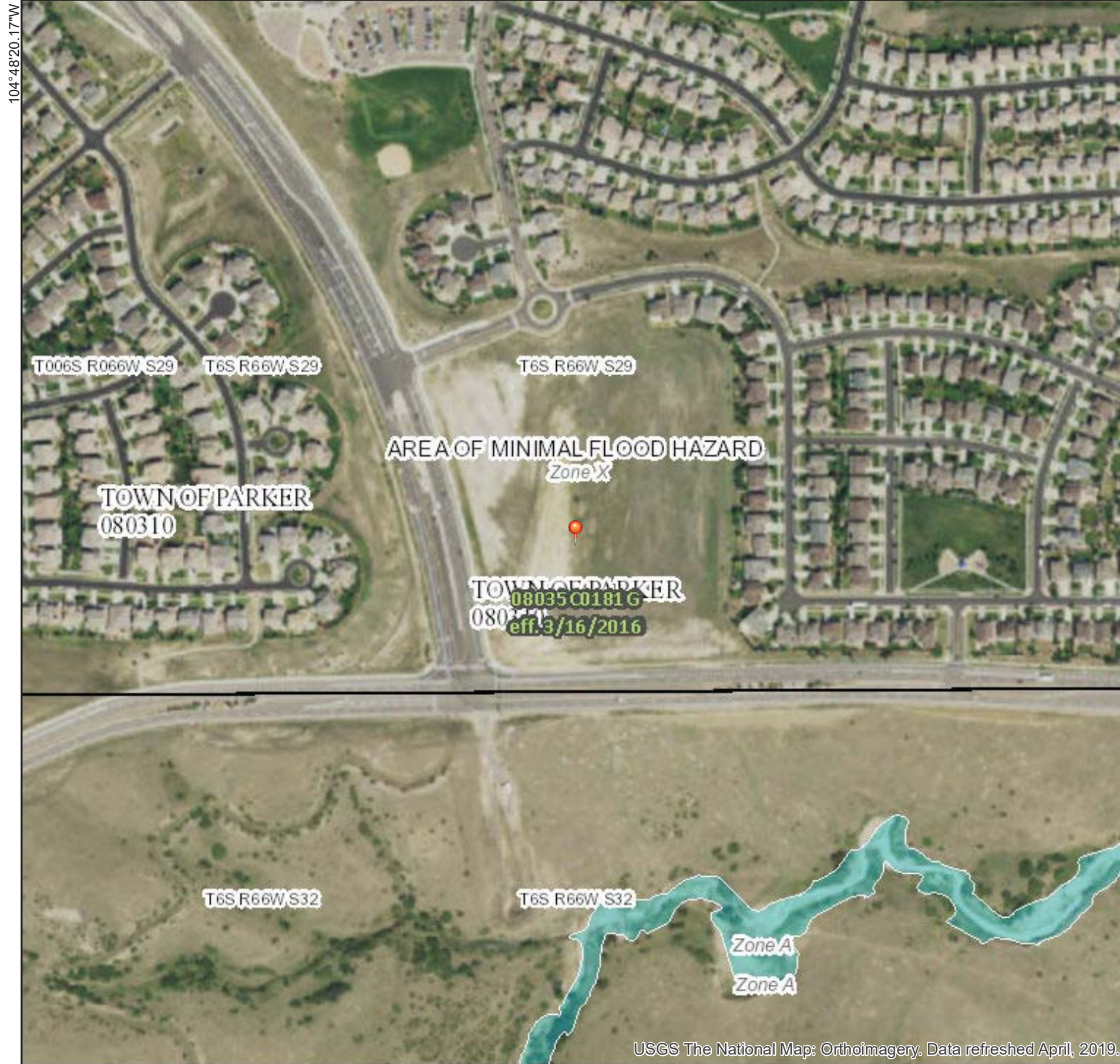
### **Attachments**

1. Table 5.1 One-Hour Point Rainfall
2. Table 6-3. Recommended percentage impervious values
3. Table 6-5. Runoff coefficients, c
4. Hydrologic Soil Map Information
5. Runoff Coefficients spreadsheet
6. Standard Form SF-1. Time of Concentration
7. Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)
8. Inlet in a Sump or Sag Location spreadsheet
9. Chambers and Hess Drainage Area Map
10. Proposed Drainage Area Map

# National Flood Hazard Layer FIRMette



39°29'52.64"N



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i>
		With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i>
		Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i>
		Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i>
		Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i>
		Area with Flood Risk due to Levee <i>Zone D</i>

OTHER AREAS		Area of Minimal Flood Hazard <i>Zone X</i>
		Effective LOMRs
		Area of Undetermined Flood Hazard <i>Zone D</i>

GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall

OTHER FEATURES		Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
		Profile Baseline
	Hydrographic Feature	

MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped

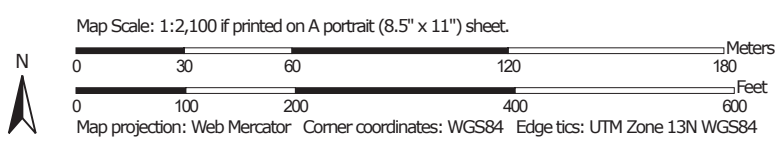
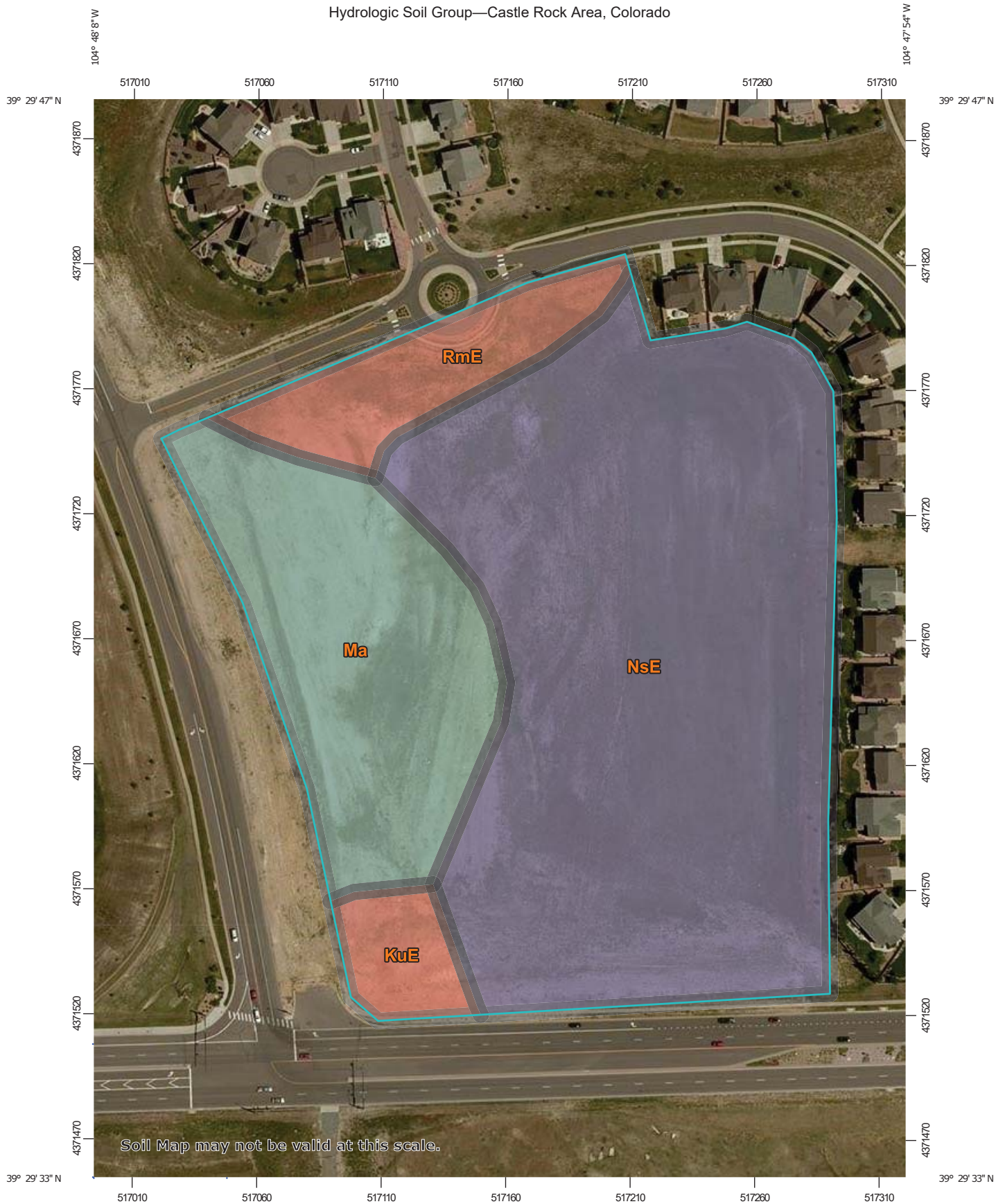
The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards









































































































The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **11/25/2019 at 3:59:17 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Hydrologic Soil Group—Castle Rock Area, Colorado



## MAP LEGEND

<b>Area of Interest (AOI)</b>	 Area of Interest (AOI)								
<b>Soils</b>	<table border="0"> <tr> <td> A</td> <td> A/D</td> <td> B</td> <td> B/D</td> <td> C</td> <td> C/D</td> <td> D</td> <td> Not rated or not available</td> </tr> </table>	 A	 A/D	 B	 B/D	 C	 C/D	 D	 Not rated or not available
 A	 A/D	 B	 B/D	 C	 C/D	 D	 Not rated or not available		
<b>Soil Rating Polygons</b>	<table border="0"> <tr> <td> A</td> <td> A/D</td> <td> B</td> <td> B/D</td> <td> C</td> <td> C/D</td> <td> D</td> <td> Not rated or not available</td> </tr> </table>	 A	 A/D	 B	 B/D	 C	 C/D	 D	 Not rated or not available
 A	 A/D	 B	 B/D	 C	 C/D	 D	 Not rated or not available		
<b>Soil Rating Lines</b>	<table border="0"> <tr> <td> A</td> <td> A/D</td> <td> B</td> <td> B/D</td> <td> C</td> <td> C/D</td> <td> D</td> <td> Not rated or not available</td> </tr> </table>	 A	 A/D	 B	 B/D	 C	 C/D	 D	 Not rated or not available
 A	 A/D	 B	 B/D	 C	 C/D	 D	 Not rated or not available		
<b>Water Features</b>	<table border="0"> <tr> <td> Streams and Canals</td> </tr> </table>	 Streams and Canals							
 Streams and Canals									
<b>Transportation</b>	<table border="0"> <tr> <td> Rails</td> <td> Interstate Highways</td> </tr> <tr> <td> US Routes</td> <td> Major Roads</td> </tr> <tr> <td> Local Roads</td> </tr> </table>	 Rails	 Interstate Highways	 US Routes	 Major Roads	 Local Roads			
 Rails	 Interstate Highways								
 US Routes	 Major Roads								
 Local Roads									
<b>Background</b>	 Aerial Photography								
<b>Soil Rating Points</b>	<table border="0"> <tr> <td> A</td> <td> A/D</td> <td> B</td> <td> B/D</td> </tr> </table>	 A	 A/D	 B	 B/D				
 A	 A/D	 B	 B/D						

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Castle Rock Area, Colorado  
 Survey Area Data: Version 12, Sep 13, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 10, 2014—Aug 21, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
KuE	Kutch clay loam, 8 to 20 percent slopes	D	0.6	3.8%
Ma	Manzanola clay loam	C	3.5	23.1%
NsE	Newlin-Satanta complex, 5 to 20 percent slopes	B	9.9	64.6%
RmE	Renohill-Buick complex, 5 to 25 percent slopes	D	1.3	8.5%
<b>Totals for Area of Interest</b>			<b>15.3</b>	<b>100.0%</b>

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

*Aggregation Method:* Dominant Condition

*Component Percent Cutoff:* None Specified

*Tie-break Rule:* Higher

**TABLE 5.1**  
**ONE-HOUR POINT RAINFALL**

Frequency of Design Event (yr)	One-hour Point Rainfall, $P_1$ (in)
2	0.99
5	1.39
10	1.64
25	1.98
50	2.31
100	2.60

### 5.3 FLOOD HYDROLOGY OVERVIEW

Various methods exist to determine appropriate flood peaks or hydrographs for storm drainage planning and design. Methods for determining flood peaks or hydrographs are the Rational Method, the Colorado Urban Hydrograph Procedure (CUHP), and Urban Drainage Stormwater Management (UDSWM) model. The Town of Parker discourages the use of computer models other than CUHP and UDSWM since these programs are preferred, if not required, by UDFCD for studies involving major drainageways where UDFCD approval is sought or where maintenance eligibility is requested.

The three methods are briefly described in this section, and a discussion of their applicability to the Town of Parker is discussed. UDSWM is mostly used to combine and route the hydrographs generated using CUHP.

In general, the Rational Method is the most widely used and accepted technique for determining peak flows in urban areas for small basins. Within the constraints outlined in the MANUAL, use of the Rational Method provides a relatively simple but effective way to analyze storm runoff.

CUHP is somewhat more complicated than the Rational Method. It allows a manual computation of a runoff hydrograph which may be used for further hydraulic routing through channels and/or detention ponds. Historically, CUHP is best used in urban areas for which runoff coefficients have been derived. However, recent improvements by UDFCD include consideration for different soil types, thus CUHP is now more applicable to rural areas. The reader is referred to UDFCD for the latest version of CUHP.

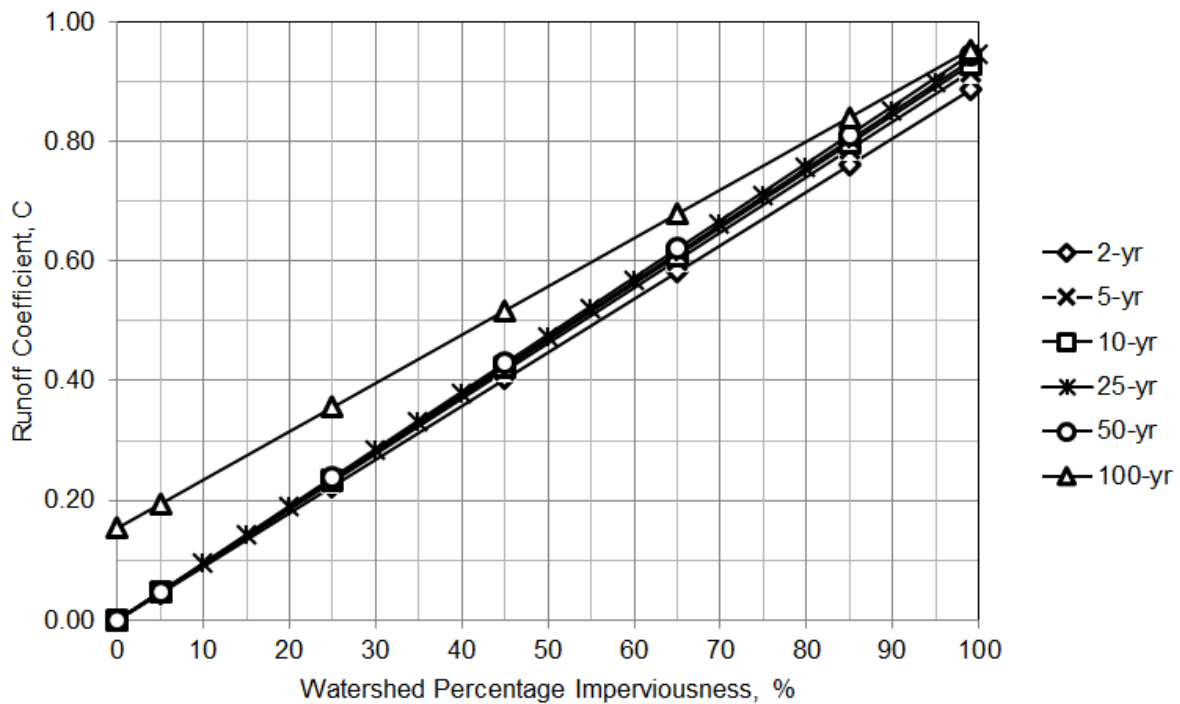
UDSWM is a computer model that generates runoff hydrographs and routes and combines these hydrographs. UDSWM is a modified version of the Runoff Block of the Environmental Protection Agency's Storm Water Management Model (SWMM). It has been modified to be used in conjunction with CUHP. Table 5.2 herein provides guidance on selecting the appropriate method for a given project.

**Table 6-3. Recommended percentage imperviousness values**

Land Use or Surface Characteristics	Percentage Imperviousness (%)
<b>Business:</b>	
Downtown Areas	95
Suburban Areas	75
<b>Residential lots (lot area only):</b>	
Single-family	
2.5 acres or larger	12
0.75 – 2.5 acres	20
0.25 – 0.75 acres	30
0.25 acres or less	45
Apartments	75
<b>Industrial:</b>	
Light areas	80
Heavy areas	90
<b>Parks, cemeteries</b>	10
<b>Playgrounds</b>	25
<b>Schools</b>	55
<b>Railroad yard areas</b>	50
<b>Undeveloped Areas:</b>	
Historic flow analysis	2
Greenbelts, agricultural	2
Off-site flow analysis (when land use not defined)	45
<b>Streets:</b>	
Paved	100
Gravel (packed)	40
Drive and walks	90
Roofs	90
Lawns, sandy soil	2
Lawns, clayey soil	2

**Table 6-5. Runoff coefficients, *c* (continued)**

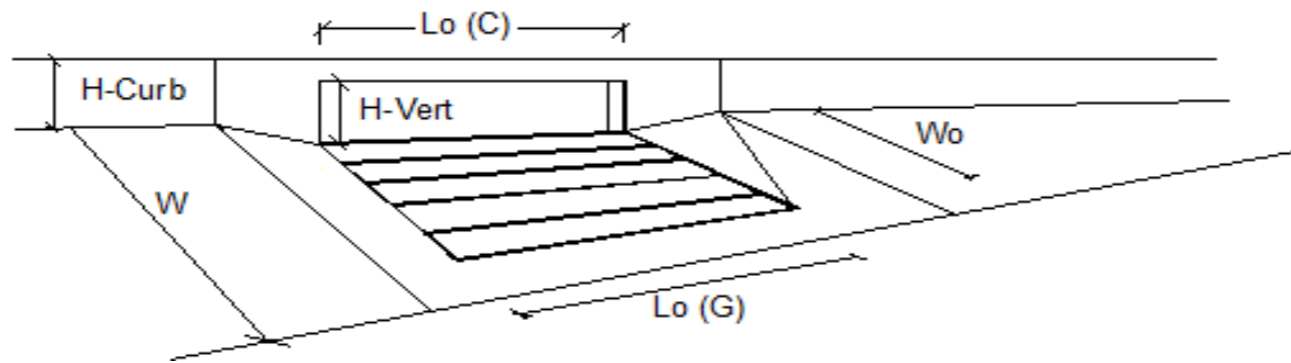
Total or Effective % Impervious	NRCS Hydrologic Soil Group C						
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	500-Year
2%	0.01	0.05	0.15	0.33	0.40	0.49	0.59
5%	0.03	0.08	0.17	0.35	0.42	0.5	0.6
10%	0.06	0.12	0.21	0.37	0.44	0.52	0.62
15%	0.1	0.16	0.24	0.4	0.47	0.55	0.64
20%	0.14	0.2	0.28	0.43	0.49	0.57	0.65
25%	0.18	0.24	0.32	0.46	0.52	0.59	0.67
30%	0.22	0.28	0.35	0.49	0.54	0.61	0.68
35%	0.26	0.32	0.39	0.51	0.57	0.63	0.7
40%	0.3	0.36	0.43	0.54	0.59	0.65	0.71
45%	0.34	0.4	0.46	0.57	0.62	0.67	0.73
50%	0.38	0.44	0.5	0.6	0.64	0.69	0.75
55%	0.43	0.48	0.54	0.63	0.66	0.71	0.76
60%	0.47	0.52	0.57	0.65	0.69	0.73	0.78
65%	0.51	0.56	0.61	0.68	0.71	0.75	0.79
70%	0.56	0.61	0.65	0.71	0.74	0.77	0.81
75%	0.6	0.65	0.68	0.74	0.76	0.79	0.82
80%	0.65	0.69	0.72	0.77	0.79	0.81	0.84
85%	0.7	0.73	0.76	0.79	0.81	0.83	0.86
90%	0.74	0.77	0.79	0.82	0.84	0.85	0.87
95%	0.79	0.81	0.83	0.85	0.86	0.87	0.89
100%	0.83	0.85	0.87	0.88	0.89	0.89	0.9



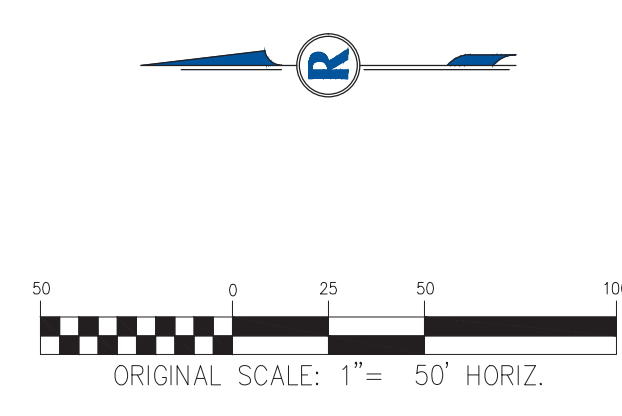
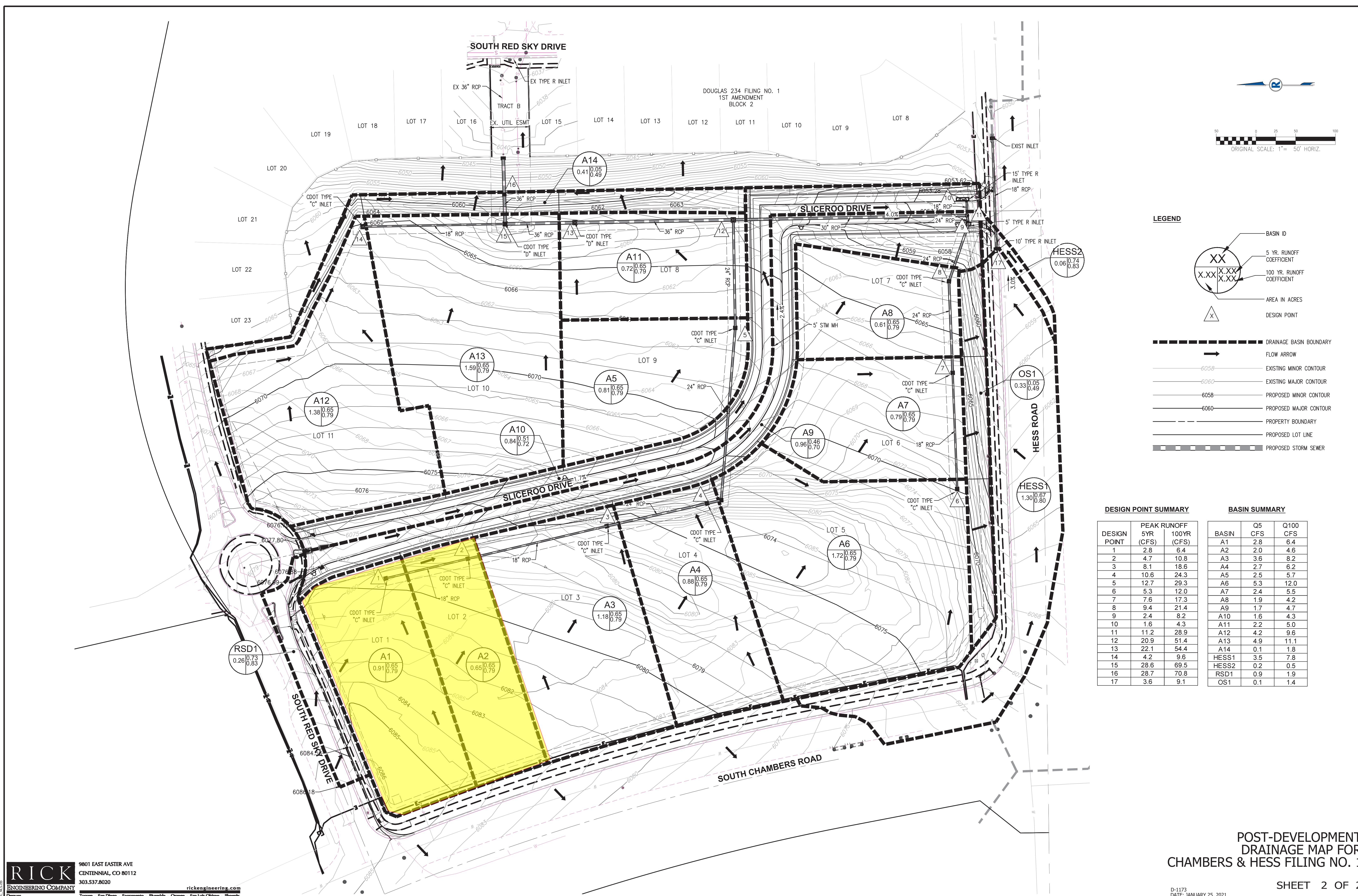
**Figure 6-1. Runoff coefficient vs. watershed imperviousness NRCS HSG A**

## INLET ON A CONTINUOUS GRADE

Version 4.06 Released August 2018



Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
<b>Street Hydraulics: OK - Q &lt; Allowable Street Capacity'</b>			
<b>Design Discharge for Half of Street (from Sheet Inlet Management)</b>			
Water Spread Width	1.2	2.7	cfs
Water Depth at Flowline (outside of local depression)	4.1	5.8	ft
Water Depth at Street Crown (or at $T_{MAX}$ )	1.9	2.4	inches
Ratio of Gutter Flow to Design Flow	0.0	0.0	inches
Discharge outside the Gutter Section W, carried in Section $T_x$	0.646	0.485	
Discharge within the Gutter Section W	0.4	1.4	cfs
Discharge Behind the Curb Face	0.8	1.3	cfs
Flow Area within the Gutter Section W	0.0	0.0	cfs
Velocity within the Gutter Section W	0.12	0.16	sq ft
Water Depth for Design Condition	6.6	8.1	fps
	4.9	5.4	inches
<b>Grate Analysis (Calculated)</b>			
Total Length of Inlet Grate Opening	N/A	N/A	ft
Ratio of Grate Flow to Design Flow	N/A	N/A	
<b>Under No-Clogging Condition</b>			
Minimum Velocity Where Grate Splash-Over Begins	N/A	N/A	fps
Interception Rate of Frontal Flow	N/A	N/A	
Interception Rate of Side Flow	N/A	N/A	
Interception Capacity	N/A	N/A	cfs
<b>Under Clogging Condition</b>			
Clogging Coefficient for Multiple-unit Grate Inlet	N/A	N/A	
Clogging Factor for Multiple-unit Grate Inlet	N/A	N/A	
Effective (unclogged) Length of Multiple-unit Grate Inlet	N/A	N/A	ft
Minimum Velocity Where Grate Splash-Over Begins	N/A	N/A	fps
Interception Rate of Frontal Flow	N/A	N/A	
Interception Rate of Side Flow	N/A	N/A	
<b>Actual Interception Capacity</b>	N/A	N/A	cfs
<b>Carry-Over Flow = <math>Q_o - Q_a</math></b> (to be applied to curb opening or next d/s inlet)	N/A	N/A	cfs
<b>Curb or Slotted Inlet Opening Analysis (Calculated)</b>			
Equivalent Slope $S_e$ (based on grate carry-over)	0.224	0.174	ft/ft
Required Length $L_T$ to Have 100% Interception	5.22	8.72	ft
<b>Under No-Clogging Condition</b>			
Effective Length of Curb Opening or Slotted Inlet (minimum of $L$ , $L_T$ )	5.22	8.72	ft
Interception Capacity	1.2	2.7	cfs
<b>Under Clogging Condition</b>			
Clogging Coefficient	1.25	1.25	
Clogging Factor for Multiple-unit Curb Opening or Slotted Inlet	0.06	0.06	
Effective (Unclogged) Length	8.75	8.75	ft
<b>Actual Interception Capacity</b>	1.2	2.7	cfs
<b>Carry-Over Flow = <math>Q_{b(GRATE)} - Q_a</math></b>	0.0	0.0	cfs
<b>Summary</b>			
<b>Total Inlet Interception Capacity</b>	1.2	2.7	cfs
<b>Total Inlet Carry-Over Flow (flow bypassing inlet)</b>	0.0	0.0	cfs
<b>Capture Percentage = <math>Q_a/Q_o =</math></b>	100	100	%



**LEGEND**

- XX BASIN ID
- X.XX 5 YR. RUNOFF COEFFICIENT
- X.XX 100 YR. RUNOFF COEFFICIENT
- X.XX AREA IN ACRES
- X DESIGN POINT
- DRAINAGE BASIN BOUNDARY
- FLOW ARROW
- EXISTING MINOR CONTOUR
- EXISTING MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPERTY BOUNDARY
- PROPOSED LOT LINE
- PROPOSED STORM SEWER

**DESIGN POINT SUMMARY**

DESIGN POINT	PEAK RUNOFF 5YR (CFS)	100YR (CFS)
1	2.8	6.4
2	4.7	10.8
3	8.1	18.6
4	10.6	24.3
5	12.7	29.3
6	5.3	12.0
7	7.6	17.3
8	9.4	21.4
9	2.4	8.2
10	1.6	4.3
11	11.2	28.9
12	20.9	51.4
13	22.1	54.4
14	4.2	9.6
15	28.6	69.5
16	28.7	70.8
17	3.6	9.1

**BASIN SUMMARY**

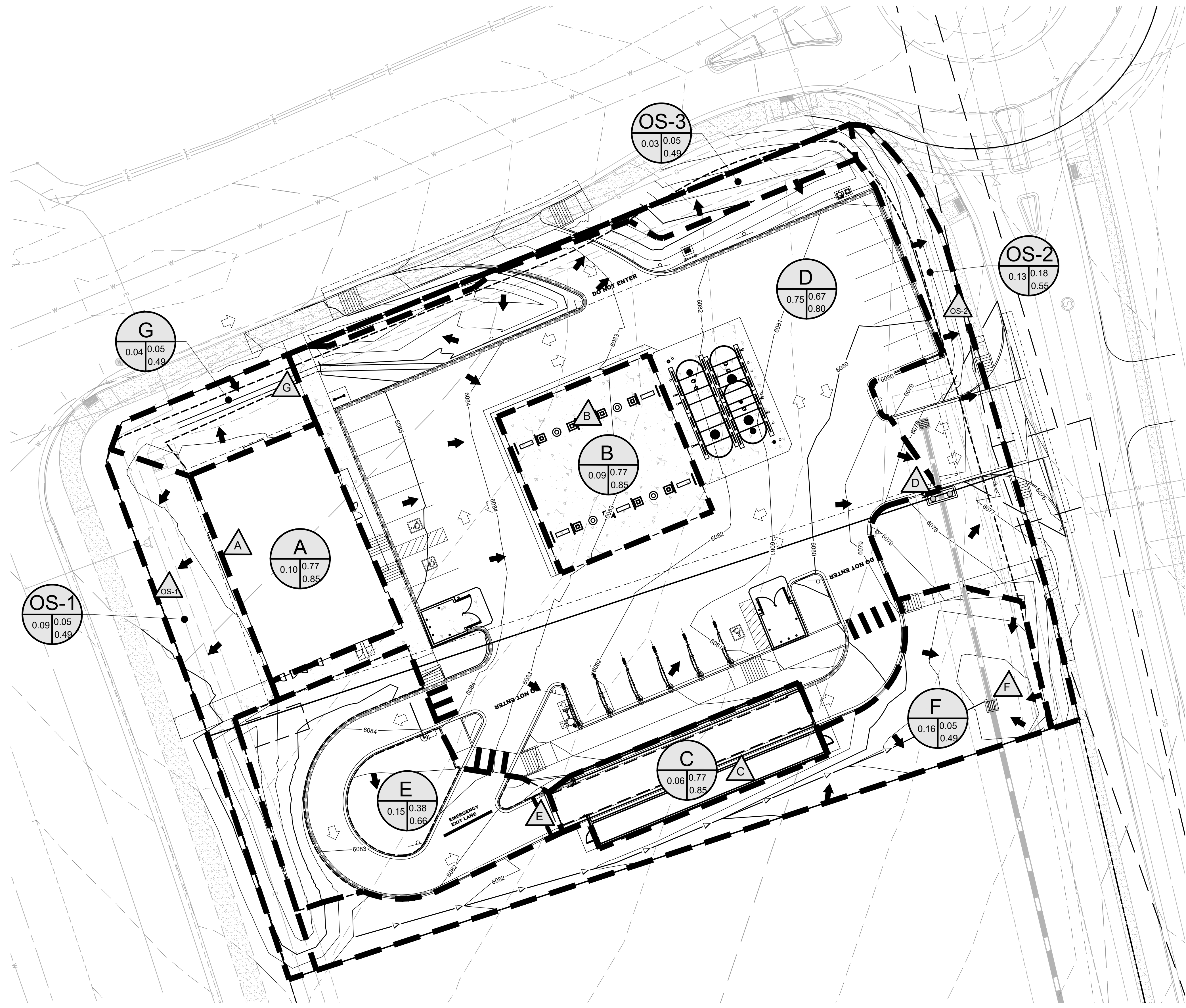
BASIN	Q5 CFS	Q100 CFS
A1	2.8	6.4
A2	2.0	4.6
A3	3.6	8.2
A4	2.7	6.2
A5	2.5	5.7
A6	5.3	12.0
A7	2.4	5.5
A8	1.9	4.2
A9	1.7	4.7
A10	1.6	4.3
A11	2.2	5.0
A12	4.2	9.6
A13	4.9	11.1
A14	0.1	1.8
HESS1	3.5	7.8
HESS2	0.2	0.5
RSD1	0.9	1.9
OS1	0.1	1.4

**RICK** ENGINEERING COMPANY  
 9801 EAST EASTER AVE  
 CENTENNIAL, CO 80112  
 303.537.8020  
 rickengineering.com

D-1173  
 DATE: JANUARY 25, 2021

SHEET 2 OF 2

NOT FOR CONSTRUCTION – EXHIBIT FOR DRAINAGE STUDY REPORT ONLY

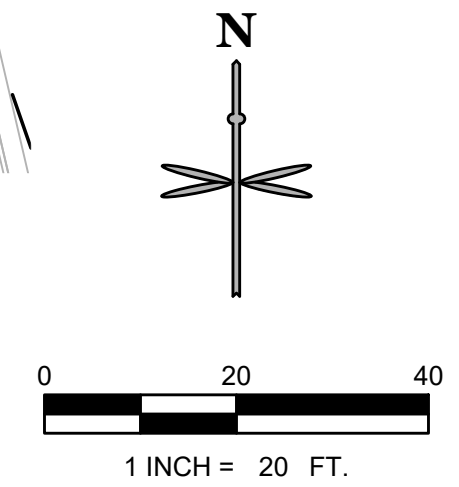


**LEGEND**

- ◀ DS PROPOSED DOWN SPOUT
- ▬ BASIN BOUNDARY
- - - SUB-BASIN BOUNDARY
- ▬▬▬ PROPERTY LINE
- FLOW ARROW
- △ 1 DESIGN POINT

BASIN SUMMARY RUNOFF TABLE

BASIN	DESIGN POINT	CONTRIBUTING BASIN ACREAGE	5-YR C-VALUE	100-YR C-VALUE	5-YR RUNOFF (CFS)	100-YR RUNOFF (CFS)
A	A	0.10	0.77	0.85	0.35	0.72
B	B	0.09	0.77	0.85	0.31	0.64
C	C	0.06	0.77	0.85	0.23	0.48
D	D	0.75	0.67	0.80	2.37	5.30
E	E	0.15	0.38	0.66	0.27	0.88
F	F	0.16	0.05	0.49	0.04	0.69
G	G	0.04	0.05	0.49	0.01	0.17
OS-1	OS-1	0.09	0.05	0.49	0.02	0.33
OS-2	OS-2	0.12	0.18	0.55	0.11	0.62
OS-3	OS-3	0.03	0.05	0.49	0.01	0.13
TOTAL		1.59	0.49	0.70	3.72	9.96



- NOTES**
1. WATER QUALITY AND DETENTION FOR THIS SITE IS PROVIDED IN A REGIONAL FACILITY LOCATED EAST OF THE SITE KNOWN AS DETENTION POND "POND A".

**811**  
 Know what's below.  
 Call before you dig.

CALL UTILITY NOTIFICATION  
 CENTER OF COLORADO  
 1-800-922-1987 or 811

CALL 3-BUSINESS DAYS (NOT INCLUDING INITIAL DAY OF CONTACT) IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

No.	REVISION	BY	DATE

**EES**  
 ENGINEERING AND  
 ENVIRONMENTAL  
 SOLUTIONS, INC.

501 S Cherry St, Suite 300  
 Glendale, CO 80246  
 303-572-7997 www.ees.us.com

**CONSTRUCTION DOCUMENTS**

**CONVENIENCE STORE AND CARWASH**

CHAMBERS ROAD AND RED SKY DRIVE PARKER, CO 80134

**DRAINAGE MAP**

PROJECT NO: \_\_\_\_\_  
 DESIGNED BY: LCG  
 DRAWN BY: JLG  
 DATE: 2021-05-27

**D1.0**