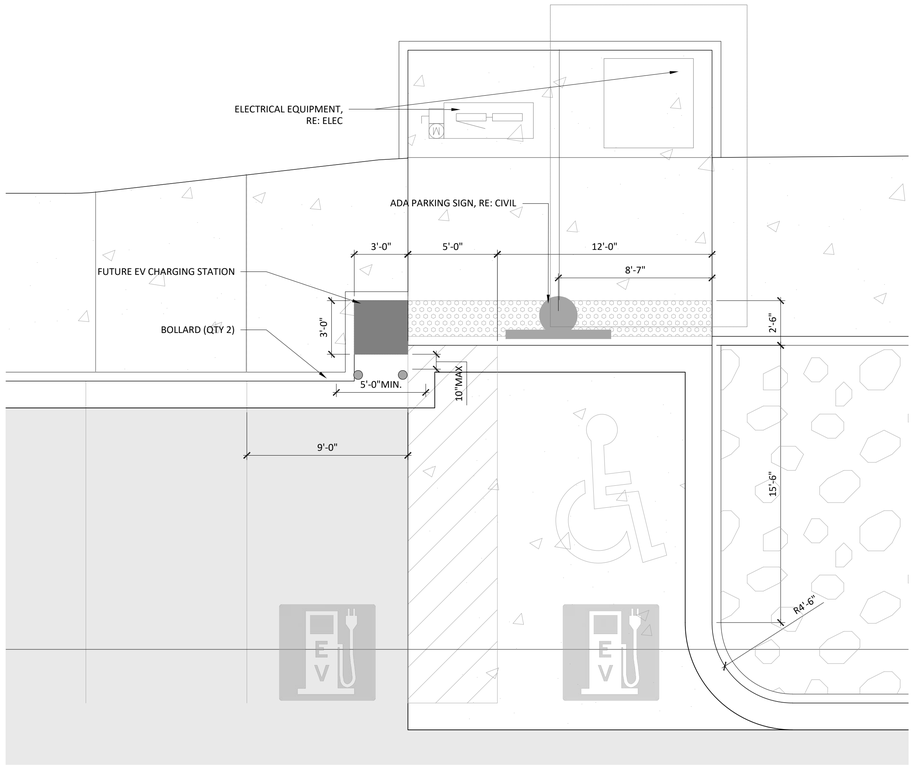
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THE SQUARES ABOVE ARE COLOR: WITH BLACK AND WHITE EITHER A TINT OR SHADE

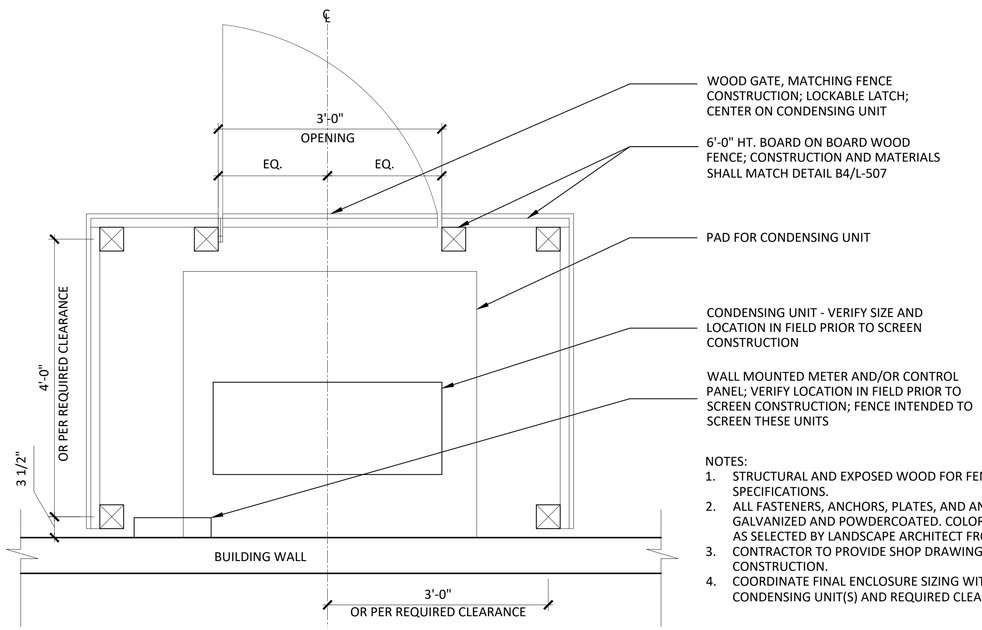


SCALE: 1/4" = 1'-0"

0' 2' 4' 8' 12'

B1 EV CHARGING STATION

1/4" = 1'-0"



NOTES:

1. STRUCTURAL AND EXPOSED WOOD FOR FENCE SHALL BE CEDAR. SEE SPECIFICATIONS.
2. ALL FASTENERS, ANCHORS, PLATES, AND ANGLES SHALL BE HOT DIPPED GALVANIZED AND POWDERCOATED. COLOR TO BE GRAPHITE BLACK RAL 9011 OR AS SELECTED BY LANDSCAPE ARCHITECT FROM FULL RANGE OF RAL COLORS.
3. CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR REVIEW PRIOR TO CONSTRUCTION.
4. COORDINATE FINAL ENCLOSURE SIZING WITH VERIFIED LOCATION OF CONDENSING UNIT(S) AND REQUIRED CLEARANCES.

A1 CONDENSING UNIT ENCLOSURE

1" = 1'-0"

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ARCHITECTURE
LANDSCAPE ARCHITECTURE
PLANNING
INTERIOR DESIGN

	DATE	DESCRIPTION

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Checked By:	RB, CB, DD

Key Map

Drawing

SITE DETAILS - 20

L-519

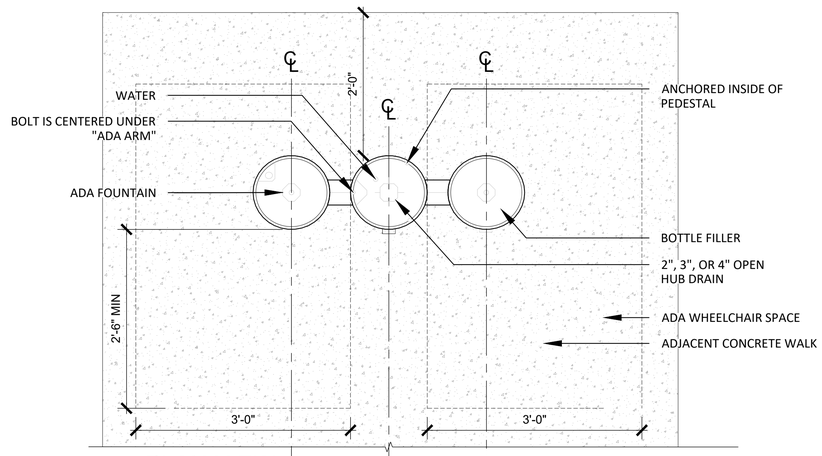
SITE PLAN SUBMITTAL
Page 203 of 311

CALL 811 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR MARKING OF UNDERGROUND MEMBER UTILITIES

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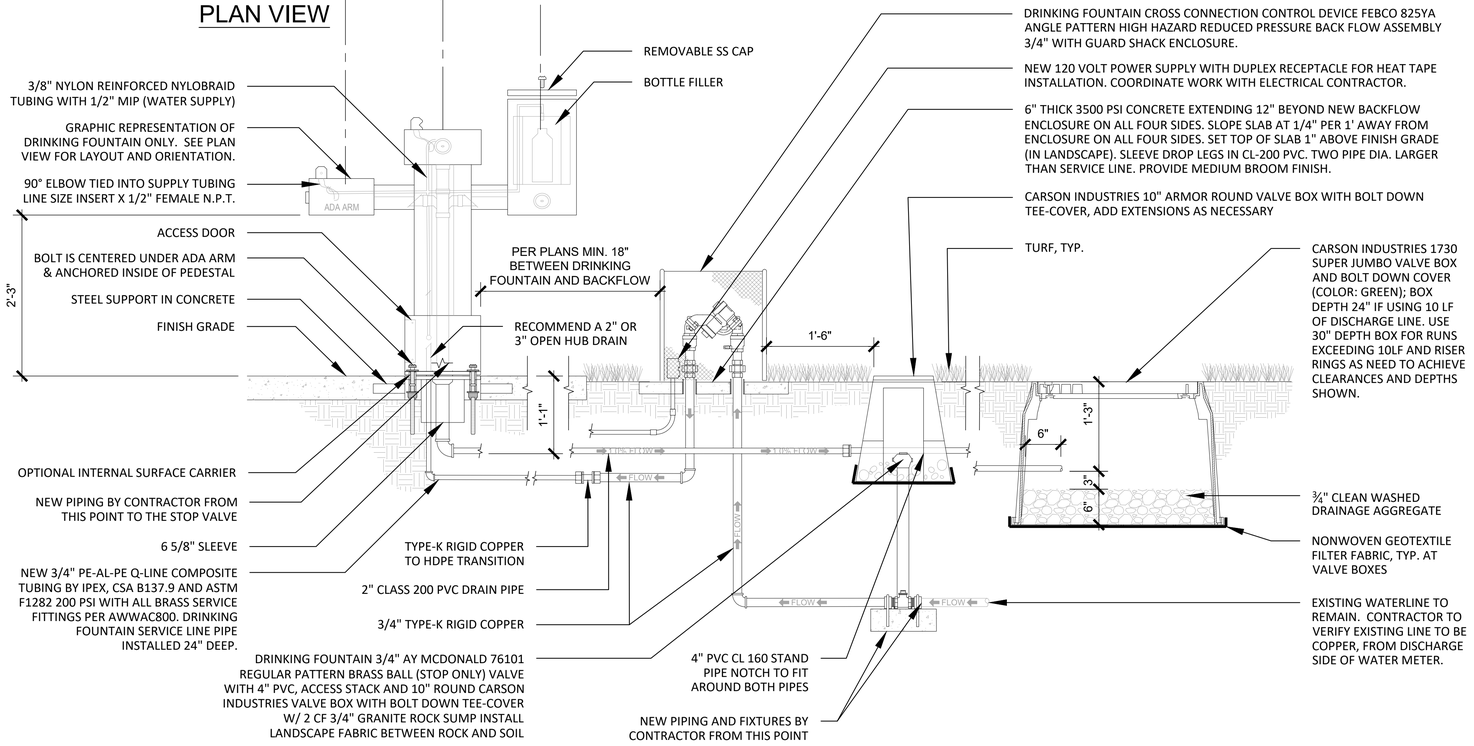
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THE SQUARES ABOVE ARE COLOR: WITH BLACK-AND-WHITE TOTAL AVAILABLE DENSITY



PLAN VIEW

NOTES:
1. MOST DEPENDABLE FOUNTAIN 2440 SMSS WITH BOTTLE FILLER AND TWO SEASON FOUNTAIN COVERS, COLOR TO BE BLACK. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.
2. PAD & DRINKING FOUNTAIN HEIGHT MUST MEET ADA STANDARDS.
3. ALL CONNECTIONS MUST BE DOUBLE CLAMPED.
4. COLOR TO BE DETERMINED BY OWNER.
5. REMOVE AND REPLACE EXISTING STOP AND WASTE VALVE AS NOTED.
6. 27" CLEAR MUST BE MAINTAINED FROM PAVING FINISH GRADE TO BOTTOM OF DRINKING BOWL. COORDINATE TOP OF CONCRETE PEDESTAL TO ACCOMMODATE 27" CLEARANCE.
7. FOR BACKFLOW ENCLOSURE AND SETTER SPECIFICATION: INSTALL ONE GUARD SHACK ENCLOSURE WITH SETTER AND FROST GUARD FOR DRINKING FOUNTAIN BACK FLOW ASSEMBLY. GUARD SHACK AND FROST GUARD COLORS TO BE GREEN.



A1 DRINKING FOUNTAIN, SERVICE LINE AND DISCHARGE SUMP

N.T.S.

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SITE DETAILS - 21

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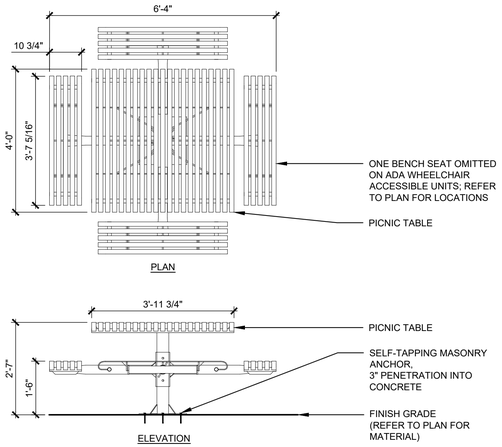
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THE SQUARES ABOVE ARE COLOR: WITH BLACK AND WHITE TESTING & PRINTING CORRECTLY



NOTES:

1. BASIS-OF-DESIGN PICNIC TABLE: VICTOR STANLEY (MFR). MODEL CRPR-4 48" SQUARE TOP (4-SEAT) AND MODEL CRPR-3 48" SQUARE TOP (3-SEAT; ADA WHEELCHAIR ACCESSIBLE). COLOR TO BE SELECTED BY LANDSCAPE ARCHITECT.
2. WHERE TABLE SITS ON UNIT PAVING OR CRUSHER FINES PAVING, MASONRY ANCHOR TO EXTEND THROUGH UNIT PAVING AND EMBED INTO C.I.P. CONCRETE SLAB OR FOOTING TO MIN. 3" DEPTH.

C1 PICNIC TABLE SQUARE

1/2" = 1'-0"

IMAGE SHOWN FOR REFERENCE ONLY
MANUFACTURER: VICTOR STANLEY
MODEL: RST-6 ACCESSIBLE 8' LONG
TABLE WITH (1) 6' BENCH AND (1)8' BENCH
COLOR: BLACK
MOUNTING: SURFACE MOUNT
QUANTITY: 3



C2 PICNIC TABLE RECTANGLE

NTS

TABLE:
MANUFACTURER: MAGLIN SITE FURNITURE
MODEL: FORO TABLE
CATALOG: MTB-1700-00770
SIZE: 36" SQUARE, 30" HEIGHT
COLOR: SILVER 14 GLOSS
MOUNT: FREESTANDING

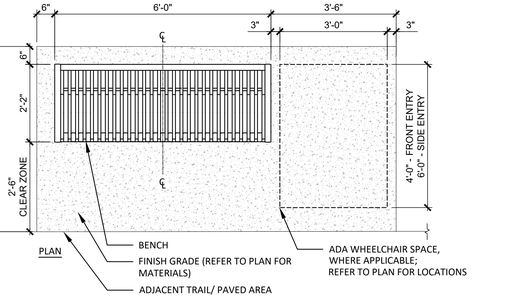
CHAIR:
MANUFACTURER: MAGLIN SITE FURNITURE
MODEL: KONTOUR CHAIR
CATALOG: FRC1700-MSF-M1-A
COLOR: EACH CHAIR TO BE SOLID COLOR.
OUT OF TOTAL, 25% TO BE TURQUOISE, 25% TO BE PASTEL BLUE, 25% TO BE VIOLET BLUE, 25% TO BE SAFRON YELLOW
MOUNT: FREESTANDING, STACKABLE





C4 MOVEABLE TABLE AND CHAIRS

N.T.S.

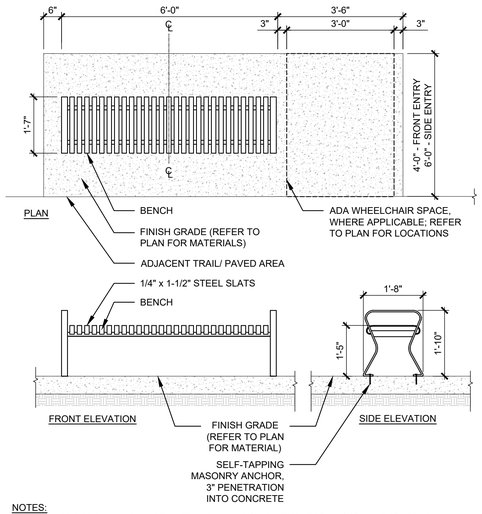


NOTES:

1. BASIS-OF-DESIGN BENCH: VICTOR STANLEY. RB-Z8 (BACKED), 6'-0"L x 2'-0"W x 1'-5"H BENCH, COLOR:STANDARD BLACK, SURFACE MOUNTED; MASONRY ANCHOR SIZE AS RECOMMENDED BY MFR.
2. WHERE BENCH SITS ON UNIT PAVING OR CRUSHER FINES PAVING, MASONRY ANCHOR TO EXTEND THROUGH UNIT PAVING AND EMBED INTO C.I.P. CONCRETE SLAB OR FOOTING TO MIN. 3" DEPTH.
3. PAD THICKNESS 4" PER PLANS AND FLATWORK DETAILS.

A1 BACKED BENCH - 6'

1/2" = 1'-0"

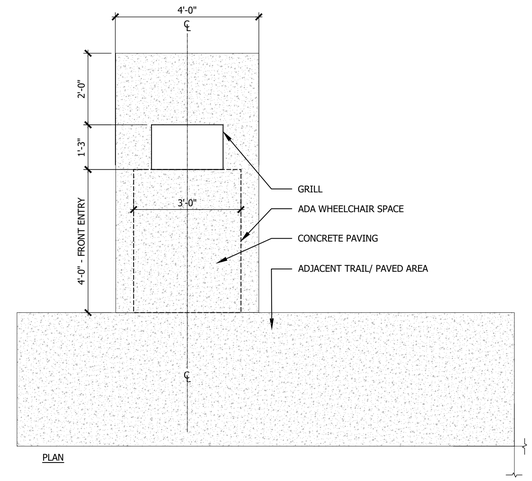


NOTES:

1. BASIS-OF-DESIGN BENCH: VICTOR STANLEY. RB-12 (BACKLESS), 6'-0"L x 1'-8"W x 1'-5"H BENCH. COLOR: STANDARD BLACK. SURFACE MOUNTED; MASONRY ANCHOR SIZE AS RECOMMENDED BY MFR.
2. WHERE BENCH SITS ON UNIT PAVING OR CRUSHER FINES PAVING, MASONRY ANCHOR TO EXTEND THROUGH UNIT PAVING AND EMBED INTO C.I.P. CONCRETE SLAB OR FOOTING TO MIN. 3" DEPTH.
3. PAD THICKNESS 4" PER PLANS AND FLATWORK DETAILS.

A2 BACKLESS BENCH - 6'

1/2" = 1'-0"



NOTES:

1. BASIS-OF-DESIGN GRILL: PILOT ROCK. ASW-24 B2.
2. COLOR: BLACK.
3. SIZE: 34" HEIGHT, 24" WIDE
4. SURFACE MOUNTED WITH ITEM B-18 SURFACE MOUNT POST; MASONRY ANCHOR SIZE AS RECOMMENDED BY MFR.
5. WHERE GRILL SITS ON UNIT PAVING OR CRUSHER FINES PAVING, MASONRY ANCHOR TO EXTEND THROUGH UNIT PAVING AND EMBED INTO C.I.P. CONCRETE SLAB OR FOOTING TO MIN. 3" DEPTH.
6. PAD THICKNESS 4" PER PLANS AND FLATWORK DETAILS.

A3 GRILL

1/2"=1'-0"

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Key Map

Drawing

SITE DETAILS - 22

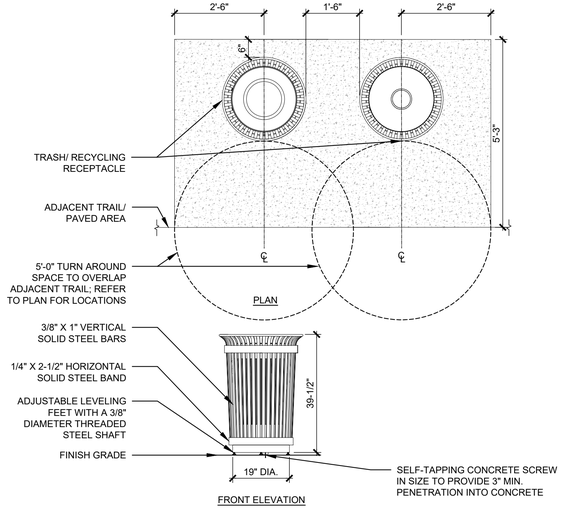
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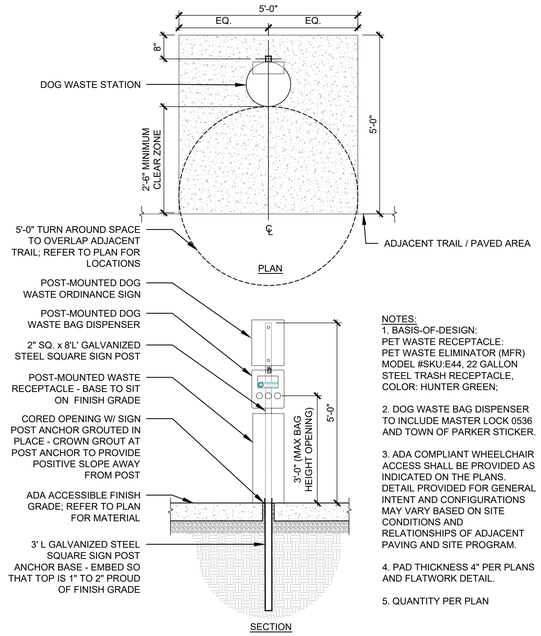
THE SQUARES ABOVE ARE COLOR, WITH BLACK AND WHITE TOTAL & PRINTS CORRECTLY

C M Y K



NOTES:
1. BASIS-OF-DESIGN: VICTOR STANLEY (MFR.) MODEL RB-45, 45 GAL., STANDARD TAPERED FORMED LID, COLOR: STANDARD BLACK; SURFACE MOUNT; PROVIDE W/ LINER.
2. WHERE RECEPTACLE SITS ON UNIT PAVING OR CRUSHER FINES PAVING, SELF-TAPPING CONCRETE SCREWS TO EXTEND THROUGH UNIT PAVING AND EMBED INTO C.I.P. CONCRETE SLAB OR FOOTING TO MIN. 3" DEPTH.
3. ADA COMPLIANT WHEELCHAIR ACCESS SHALL BE PROVIDED AS INDICATED ON THE PLANS. DETAIL PROVIDED FOR GENERAL INTENT AND CONFIGURATIONS MAY VARY BASED ON SITE CONDITIONS AND RELATIONSHIPS OF ADJACENT PAVING AND SITE PROGRAM.
4. RECYCLING RECEPTACLE TO HAVE A FLAT TOP "RECYCLING" LID.
5. PAD THICKNESS 4" PER PLANS AND FLATWORK DETAIL.
6. QUANTITY PER PLAN

C1 TRASH AND RECYCLING RECEPTACLE
1/2" = 1'-0"



NOTES:
1. BASIS-OF-DESIGN:
PET WASTE RECEPTACLE:
PET WASTE ELIMINATOR (MFR)
MODEL #SKU:E44, 22 GALLON
STEEL TRASH RECEPTACLE,
COLOR: HUNTER GREEN;

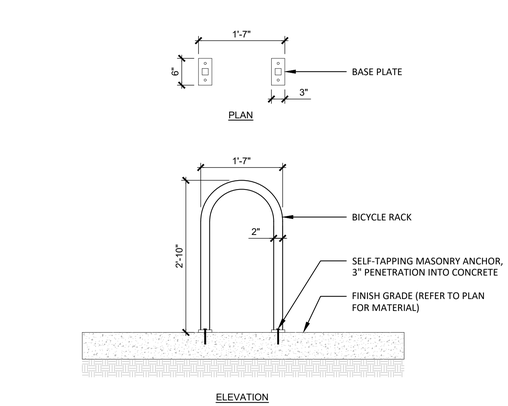
2. DOG WASTE BAG DISPENSER TO INCLUDE MASTER LOCK 0536 AND TOWN OF PARKER STICKER.

3. ADA COMPLIANT WHEELCHAIR ACCESS SHALL BE PROVIDED AS INDICATED ON THE PLANS. DETAIL PROVIDED FOR GENERAL INTENT AND CONFIGURATIONS MAY VARY BASED ON SITE CONDITIONS AND RELATIONSHIPS OF ADJACENT PAVING AND SITE PROGRAM.

4. PAD THICKNESS 4" PER PLANS AND FLATWORK DETAIL.

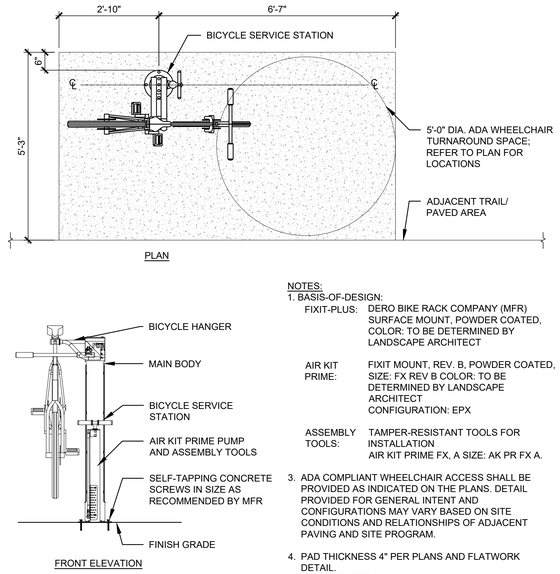
5. QUANTITY PER PLAN

C2 PET WASTE STATION
1/2" = 1'-0"



NOTES:
1. BASIS-OF-DESIGN: BIKE RACK (MADRAX U-200), SURFACE MOUNTED, COLOR: BLACK
2. FINAL LOCATION TO BE VERIFIED IN FIELD WITH OWNER AND DESIGN TEAM COORDINATE WITH OWNER ON EXACT LOCATION.
3. WHERE BICYCLE RACK SITS ON UNIT PAVING, MASONRY ANCHOR TO EXTEND THROUGH UNIT PAVING AND EMBED INTO C.I.P. CONCRETE FOOTING TO MIN. 3" DEPTH.
3. QUANTITY PER PLAN
4. COLOR: BLACK

A1 BIKE RACK
3/4" = 1'-0"



NOTES:
1. BASIS-OF-DESIGN:
FIXIT-PLUS: DERO BIKE RACK COMPANY (MFR) SURFACE MOUNT, POWDER COATED, COLOR: TO BE DETERMINED BY LANDSCAPE ARCHITECT

AIR KIT PRIME: FIXIT MOUNT, REV. B, POWDER COATED, SIZE: FX REV B COLOR: TO BE DETERMINED BY LANDSCAPE ARCHITECT CONFIGURATION: EPX

ASSEMBLY TOOLS: TAMPER-RESISTANT TOOLS FOR INSTALLATION AIR KIT PRIME FX, A SIZE: AK PR FX A.

3. ADA COMPLIANT WHEELCHAIR ACCESS SHALL BE PROVIDED AS INDICATED ON THE PLANS. DETAIL PROVIDED FOR GENERAL INTENT AND CONFIGURATIONS MAY VARY BASED ON SITE CONDITIONS AND RELATIONSHIPS OF ADJACENT PAVING AND SITE PROGRAM.

4. PAD THICKNESS 4" PER PLANS AND FLATWORK DETAIL.
5. QUANTITY PER PLAN
6. COLOR: BLACK

A2 BIKE SERVICE STATION
1/2" = 1'-0"

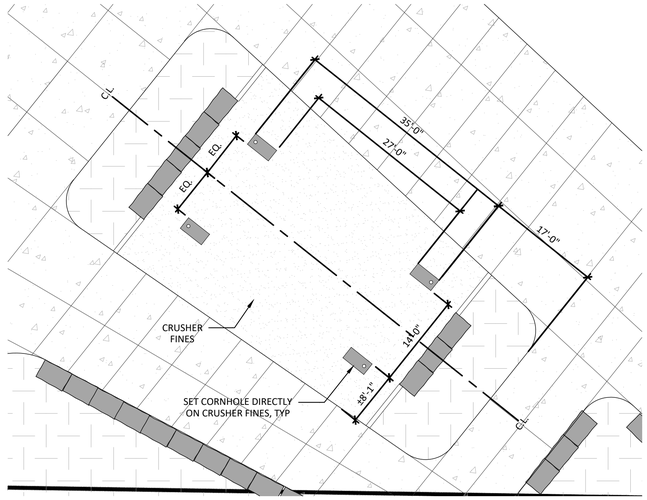


IMAGE SHOWN FOR REFERENCE ONLY
BASIS OF DESIGN:
MANUFACTURER: KAY PARK REC CORP.
MODEL: CONCRETE BAG TOSS
QUANTITY: 4



A4 CORNHOLE
NTS

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Key Map

Drawing
SITE DETAILS - 23

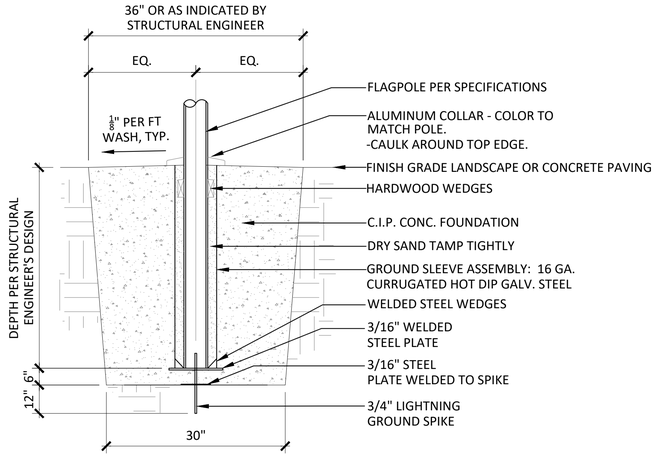
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SITE PLAN SUBMITTAL
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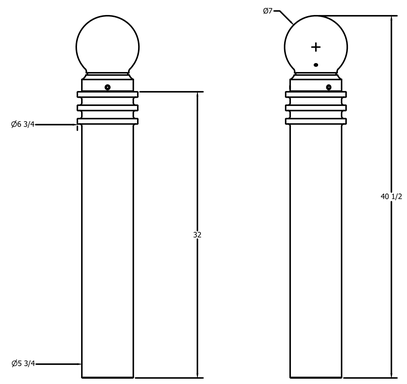
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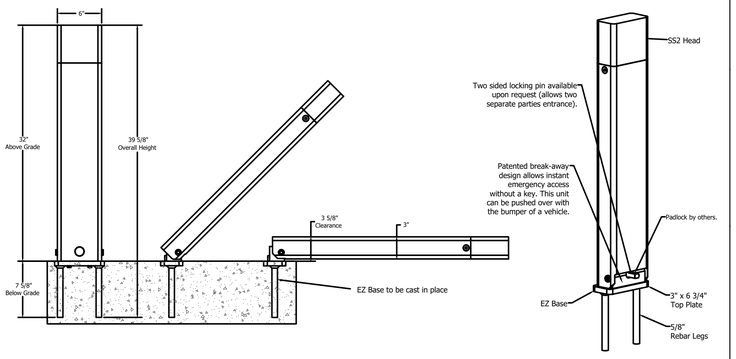
NOTE:
1. FLAGPOLE HEIGHT: AT BALLFIELDS TO BE 35'
2. FOUNDATION IS DELEGATED DESIGN. CONTRACTOR TO SUBMIT SHOP DRAWING OF DESIGN AND CALCULATIONS, SIGN/SEALED BY PROFESSIONAL ENGINEER IN JURISDICTION OF WORK BEING PERFORMED.
3. REFER TO MFR SPECIFICATIONS FOR ADDITIONAL INFORMATION.
4. LIGHTING REQUIRED AT ALL FLAGPOLES RE: ELECTRICAL
5. QUANTITY PER PLANS

C1 FLAGPOLE
1" = 1'-0"



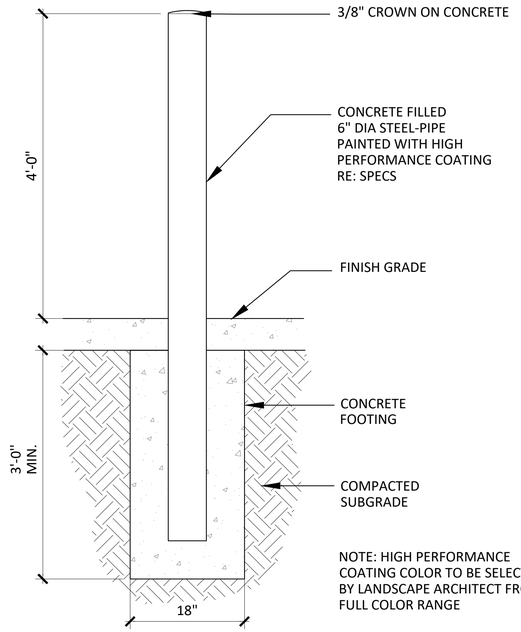
BASIS OF DESIGN:
R-7571 DECORATIVE BOLLARD BY RELIANCE FOUNDARY
COLOR: BLACK
MOUNT: EMBEDDED

C3 BOLLARD - DECORATIVE
1" = 1'-0"



PRODUCT INFORMATION:
MANUFACTURER: MAXIFORCE
MODEL: MCSP-SS2-S - COLLAPSIBLE, POWDER COATED WITH REFLECTIVE TAPE
OPERATION: PADLOCK
FINISH: ZINC RICH PRIMER OR GALVANIZED
COLOR: TO BE DETERMINED BY LANDSCAPE ARCHITECT

A1 REMOVABLE BOLLARD
NTS



NOTE: HIGH PERFORMANCE COATING COLOR TO BE SELECTED BY LANDSCAPE ARCHITECT FROM FULL COLOR RANGE

A3 BOLLARD
1/2" = 1'-0"

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SITE DETAILS - 24

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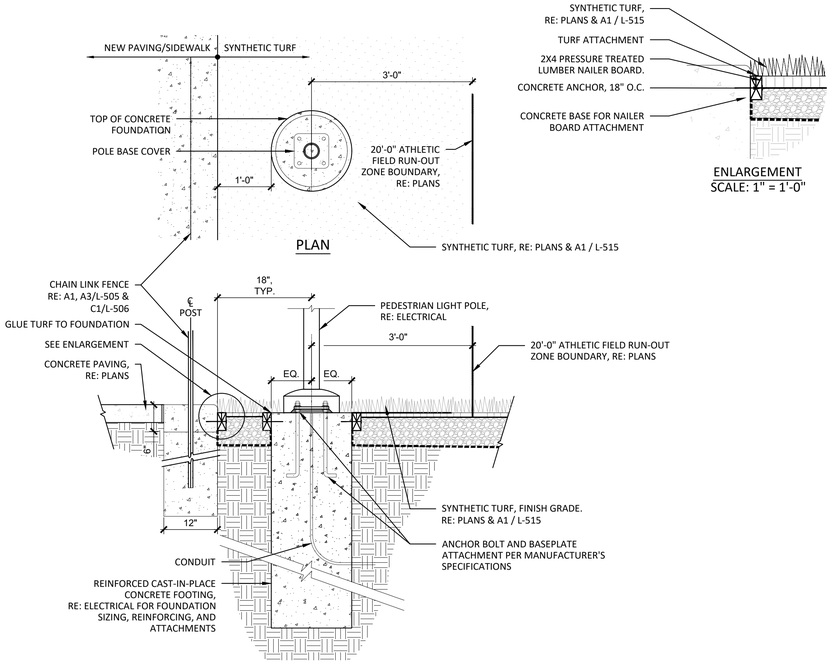
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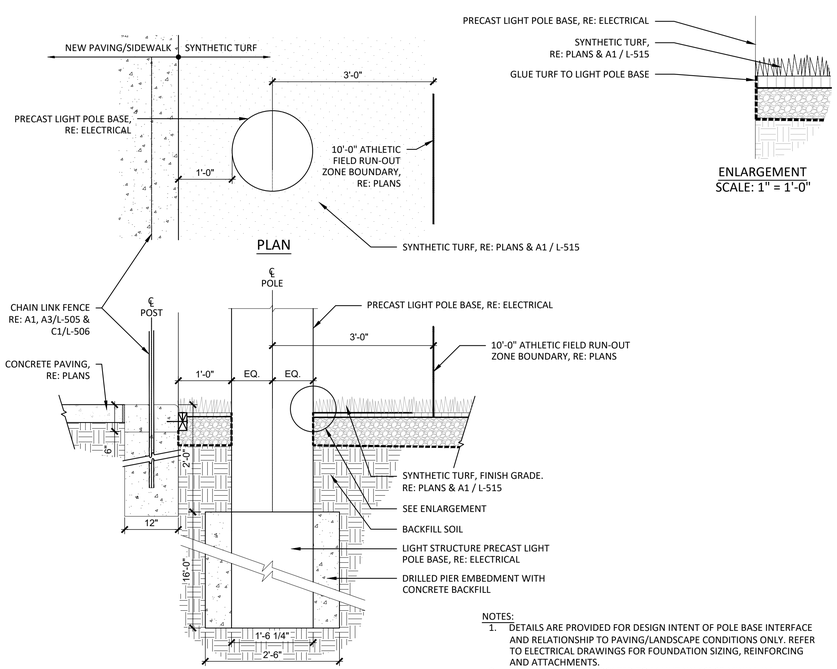
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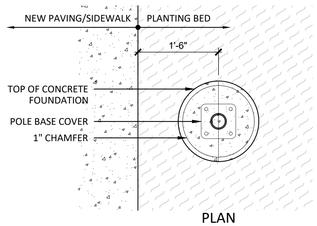


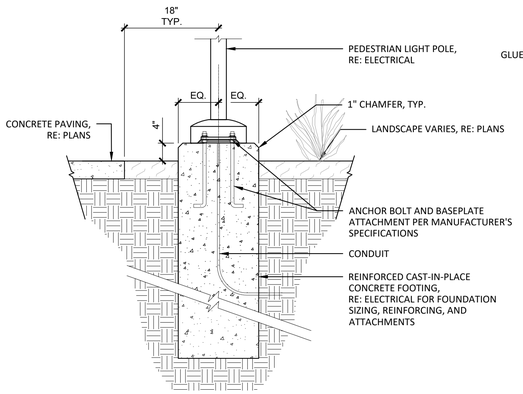
SECTION: PEDESTRIAN LIGHT POLE AT
SYNTHETIC TURF ADJACENT TO PAVEMENT



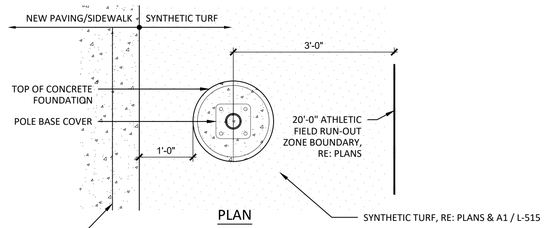
SECTION: MULTI-USE FIELDS LIGHT POLE AT
SYNTHETIC TURF ADJACENT TO PAVEMENT

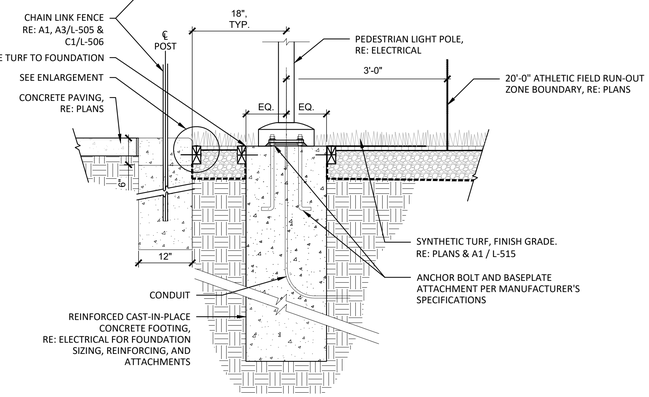
NOTES:
1. DETAILS ARE PROVIDED FOR DESIGN INTENT OF POLE BASE INTERFACE AND RELATIONSHIP TO PAVING/LANDSCAPE CONDITIONS ONLY. REFER TO ELECTRICAL DRAWINGS FOR FOUNDATION SIZING, REINFORCING AND ATTACHMENTS.
2. FIXTURES MUST BE LOCATED TO MAINTAIN 10'-0" MINIMUM HORIZONTAL CLEARANCE FROM ANY OBSTRUCTION. ENGINEER MUST BE NOTIFIED IF FOUNDATIONS ARE NEAR ANY RETAINING WALLS OR WITHIN / NEAR ANY SLOPES STEEPER THAN 3:1. POLES, FIXTURES, PRECAST BASES, ELECTRICAL ITEMS AND INSTALLATION PER MUSCO LIGHTING OR APPROVED EQUAL.



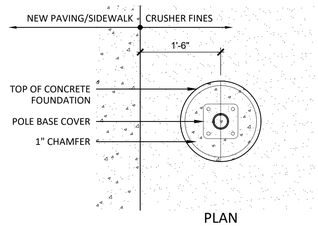


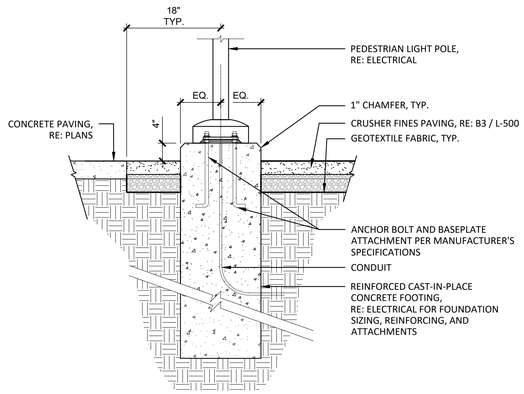
SECTION: PEDESTRIAN LIGHT POLE AT
PLANTING BED ADJACENT TO PAVEMENT





SECTION: PEDESTRIAN LIGHT POLE AT
SYNTHETIC TURF ADJACENT TO PAVEMENT





SECTION: PEDESTRIAN LIGHT POLE AT
CRUSHER FINES ADJACENT TO PAVEMENT

A4 LIGHTING FOOTER
1" = 1'-0"

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LANDSCAPE ARCHITECTURE

PLANNING

INTERIOR DESIGN

	DATE	DESCRIPTION

Project Number: 223072.00
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Checked By: RB, CB, DD

Key Map

Drawing

SITE DETAILS - 25

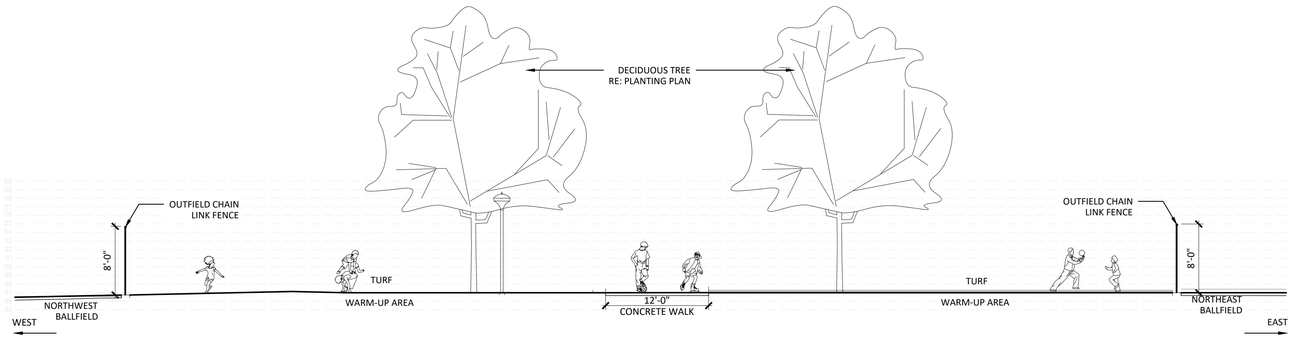
L-524

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Page 208 of 311

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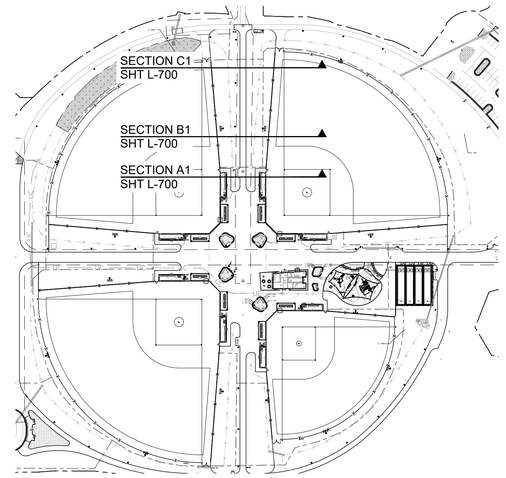
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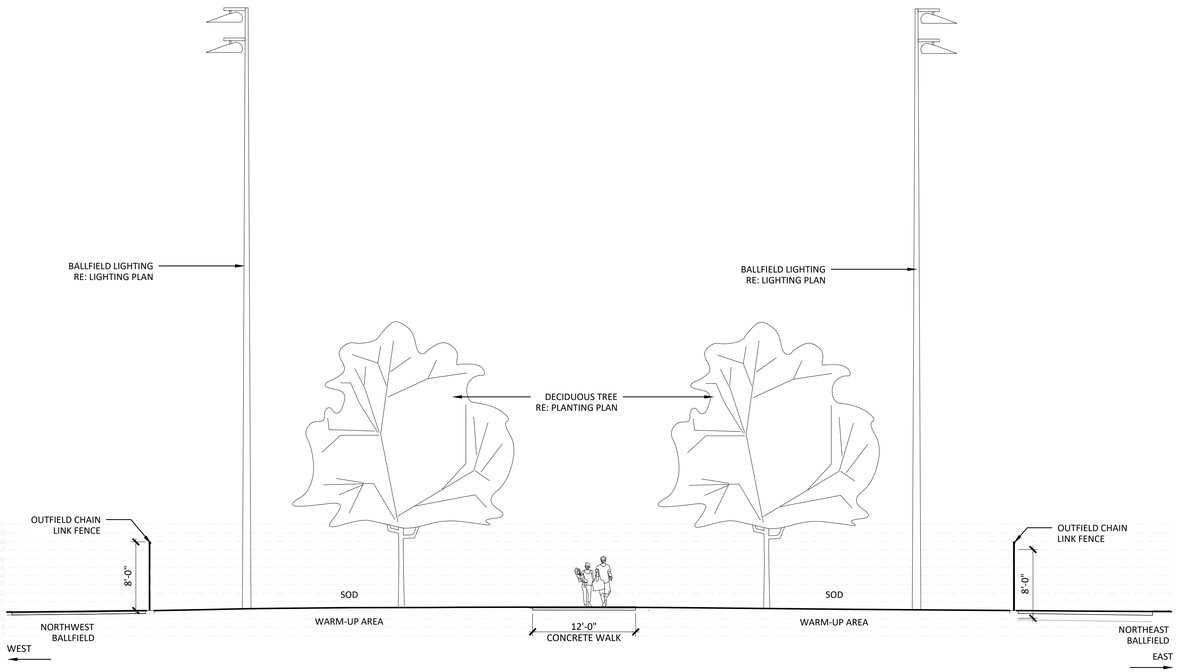
C1 BALLFIELD COMPLEX SECTION - C1

1/4" = 1'-0"



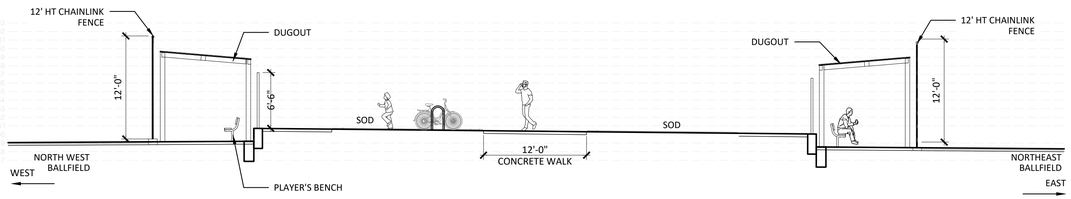
KEYMAP

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B1 BALLFIELD COMPLEX SECTION - B1

1/4" = 1'-0"



A1 BALLFIELD COMPLEX SECTION - A1

1/4" = 1'-0"

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Denver, CO 80202
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Wheat Ridge, CO 80215
p. 303.278.7297

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Key Map

Drawing

SITE SECTIONS - BALLFIELD COMPLEX

L-700

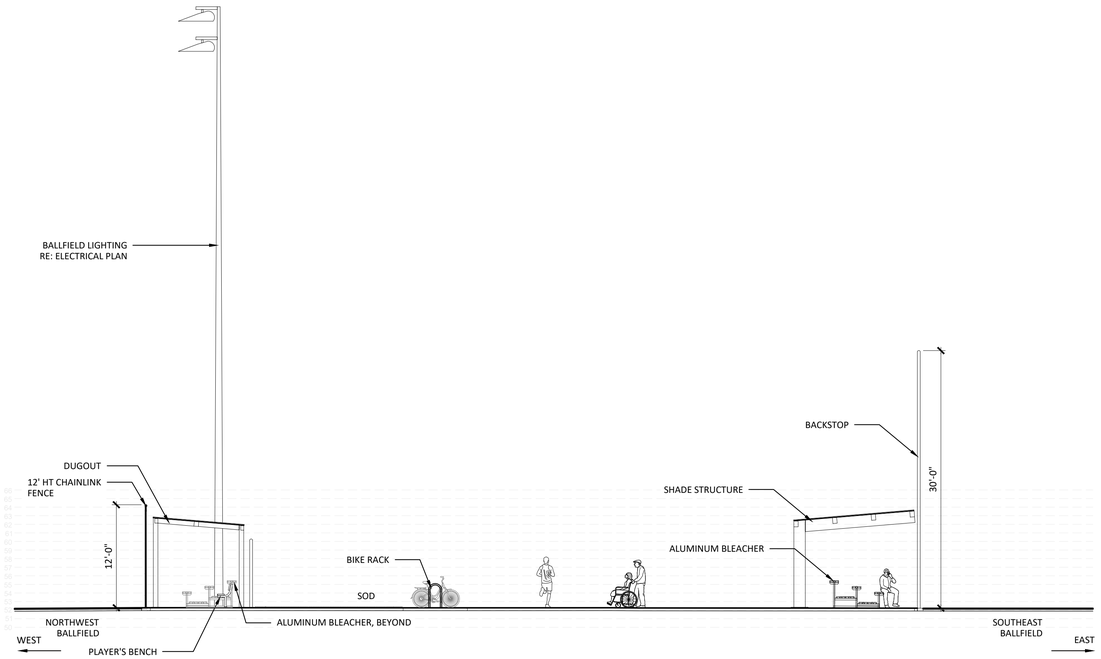
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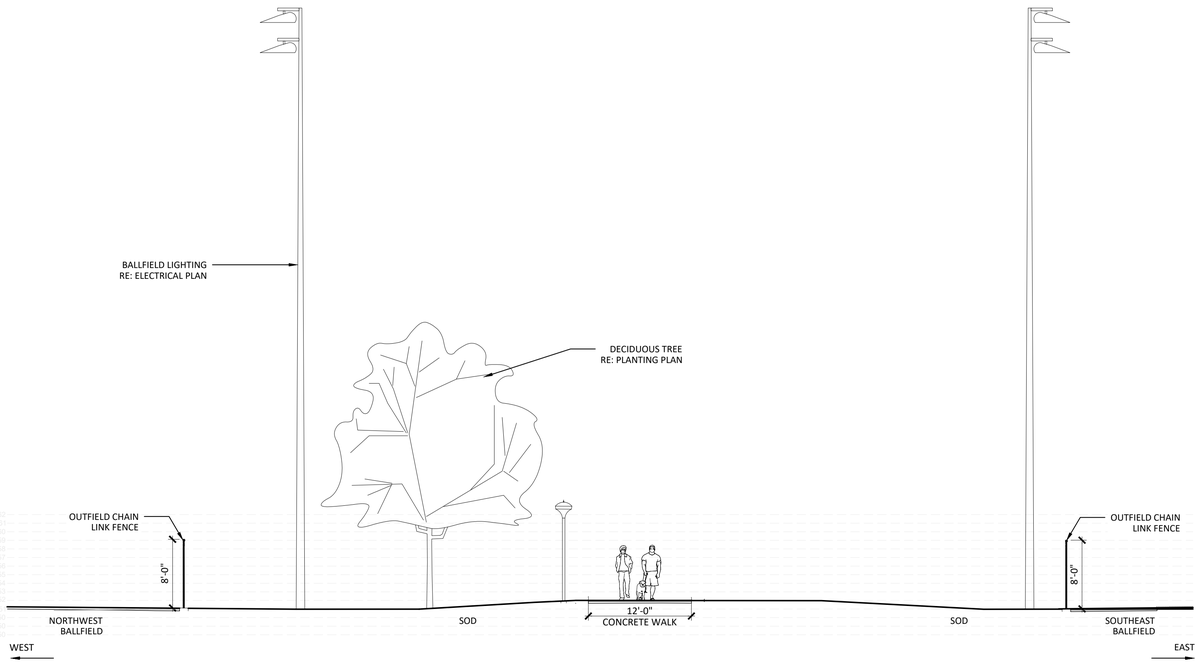
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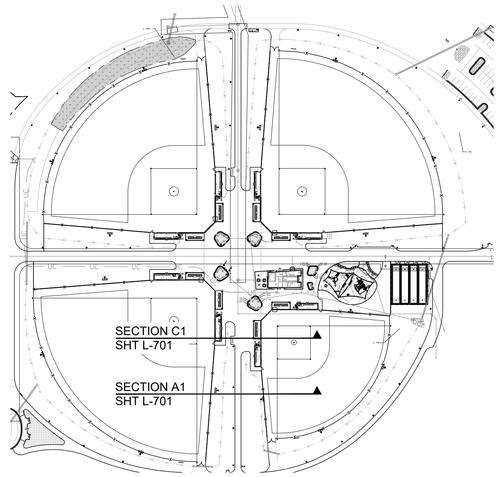
C1 BALLFIELD COMPLEX SECTION - C1

1/4" = 1'-0"



A1 BALLFIELD COMPLEX SECTION - A1

1/4" = 1'-0"



KEYMAP

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p. 303.986.2175

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Key Map

Drawing

SITE SECTIONS - BALLFIELD COMPLEX

L-701

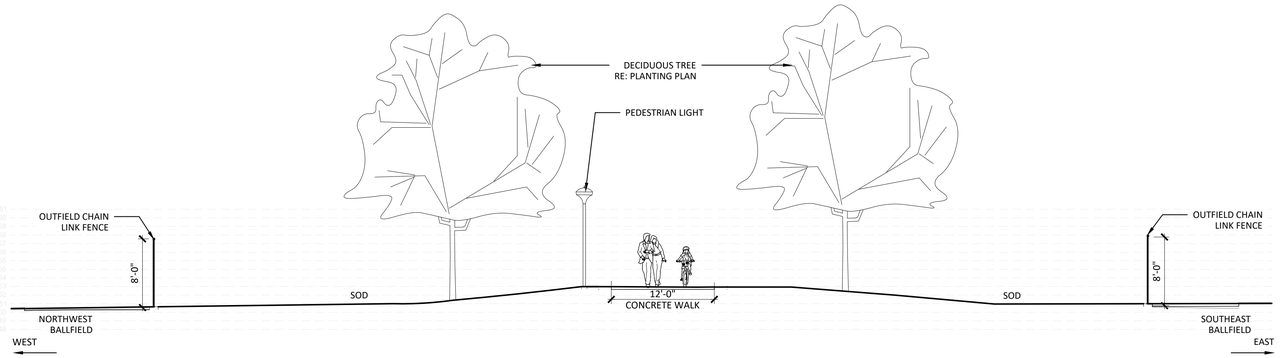
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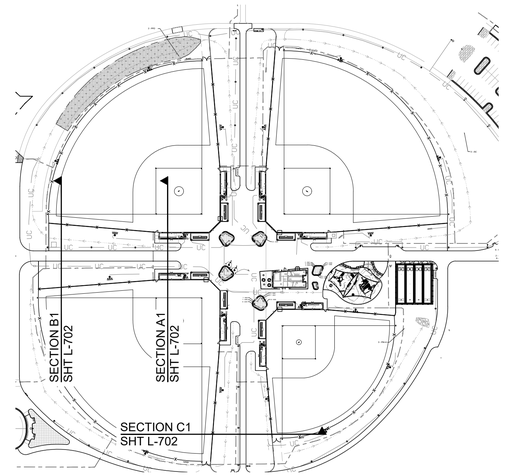
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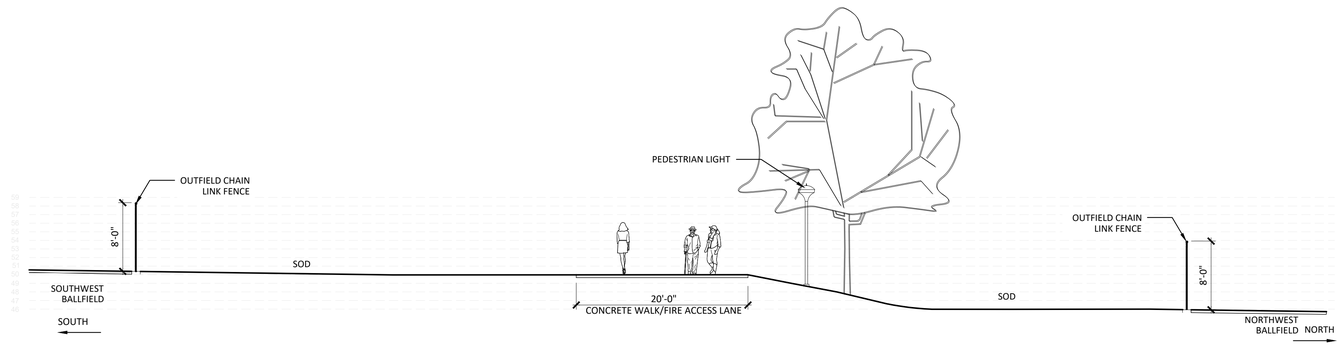
C1 BALLFIELD COMPLEX SECTION - C1

1/4" = 1'-0"



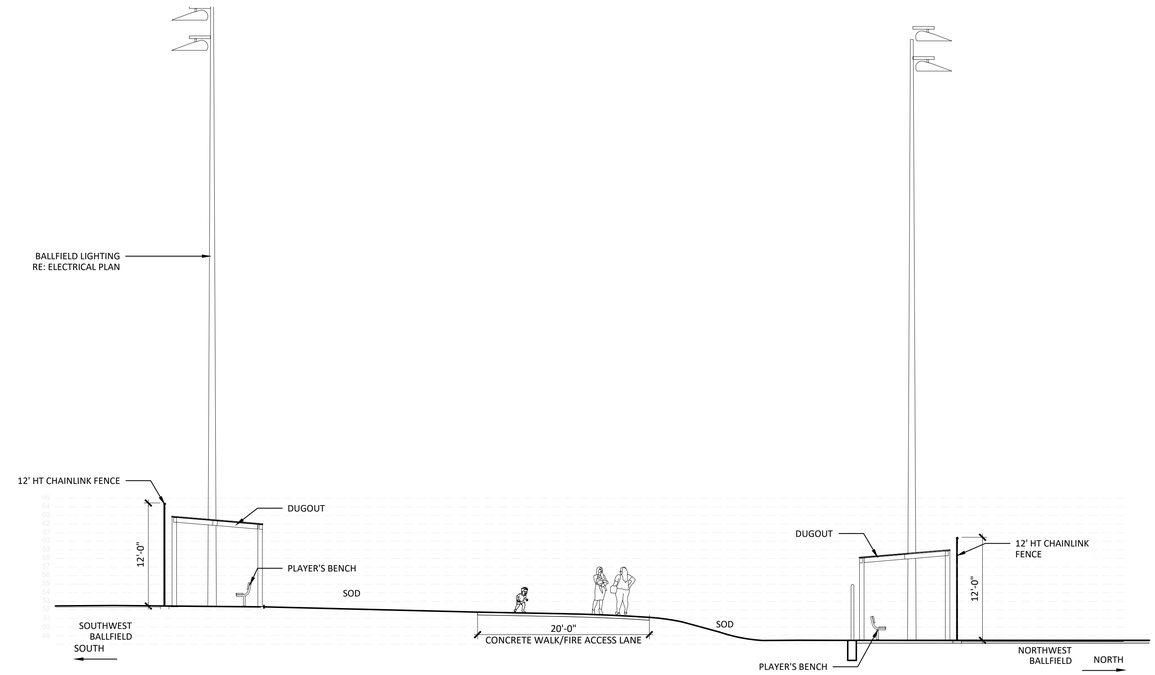
KEYMAP

NTS



B1 BALLFIELD COMPLEX SECTION - B1

1/4" = 1'-0"



A1 BALLFIELD COMPLEX SECTION - A1

1/4" = 1'-0"

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p. 303.986.2175

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PLANNING
INTERIOR DESIGN

	DATE	DESCRIPTION

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Key Map

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SITE SECTIONS - BALLFIELD COMPLEX

L-702

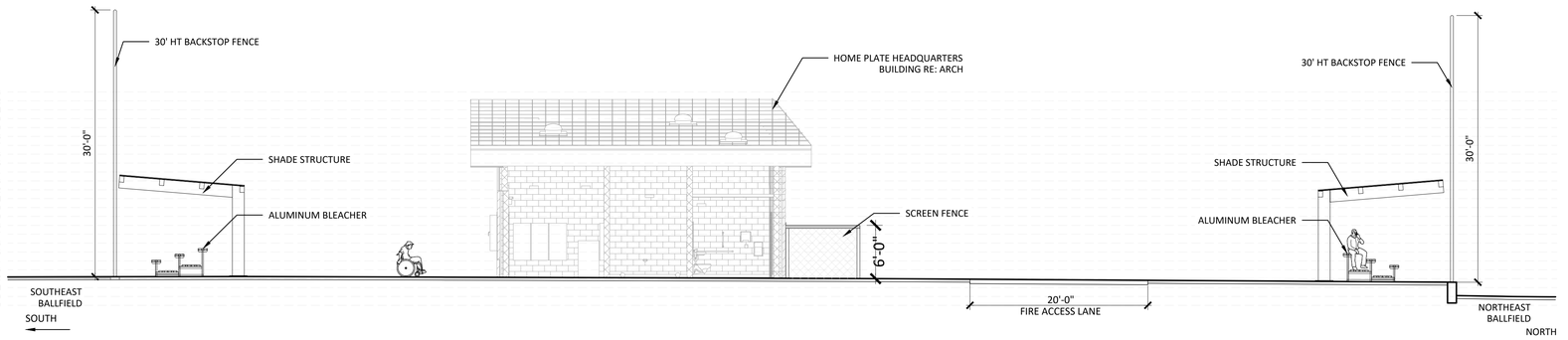
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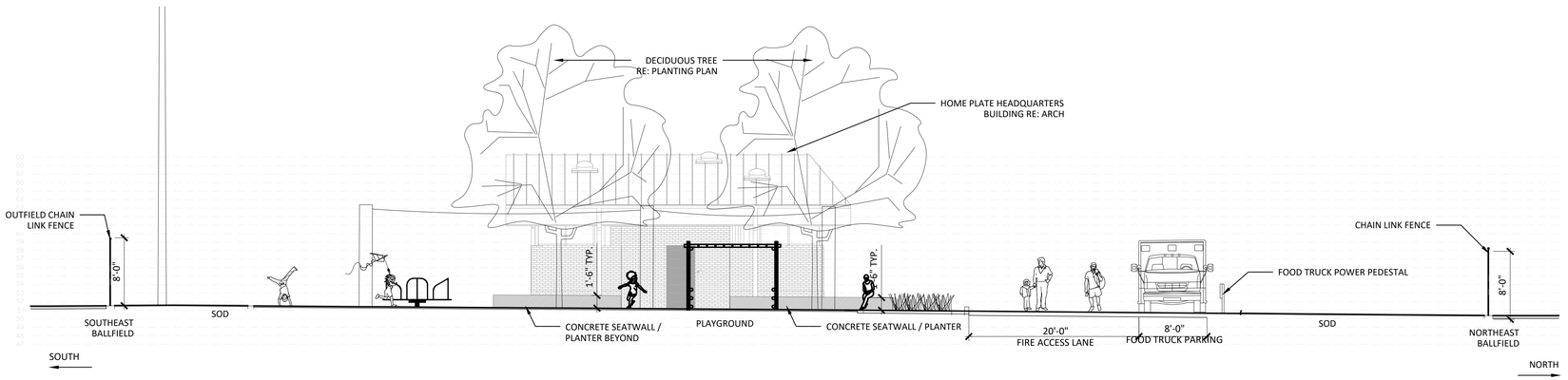
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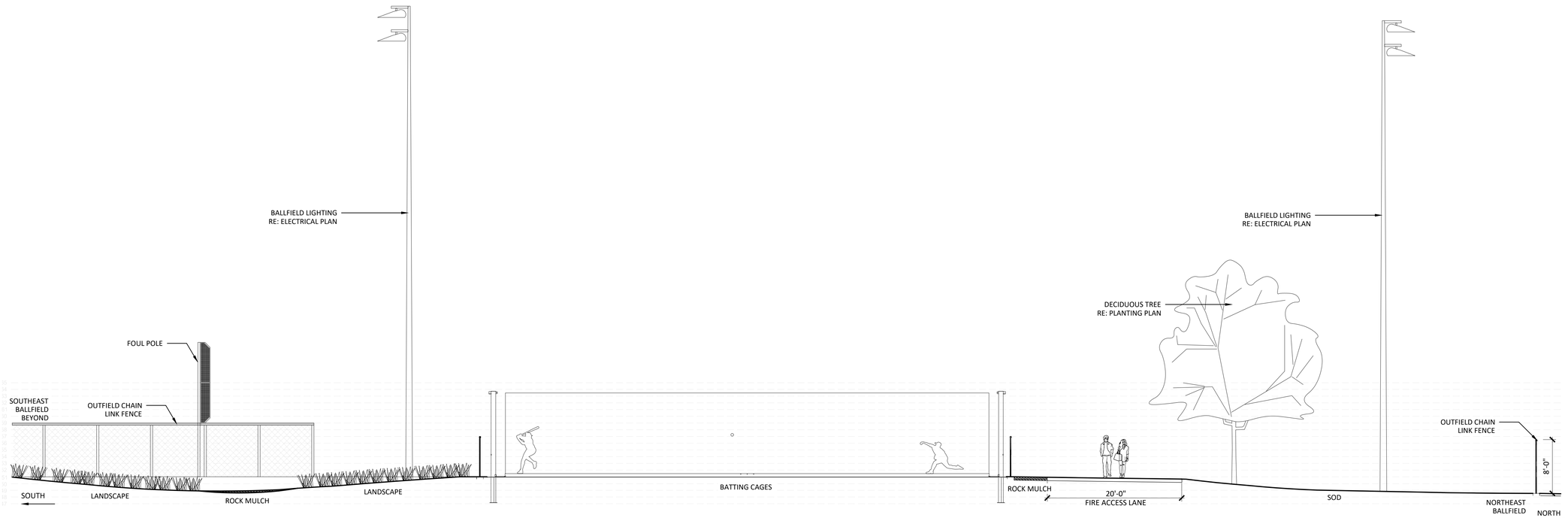
C1 BALLFIELD COMPLEX SECTION - C1

1/8" = 1'-0"



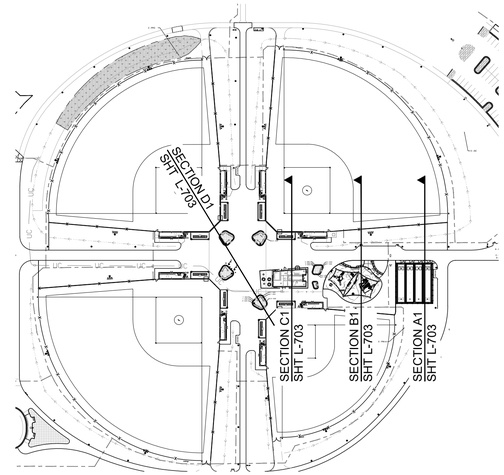
B1 BALLFIELD COMPLEX SECTION - B1 (ADD ALTERNATE NO. 03)

1/8" = 1'-0"



A1 BALLFIELD COMPLEX SECTION - A1 (ADD ALTERNATE NO. 04)

1/8" = 1'-0"



KEYMAP

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Denver, CO 80202
p. 303.444.1951

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Wheat Ridge, CO 80215
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ARCHITECTURE
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Key Map

Drawing

SITE SECTIONS - BALLFIELD COMPLEX

L-703

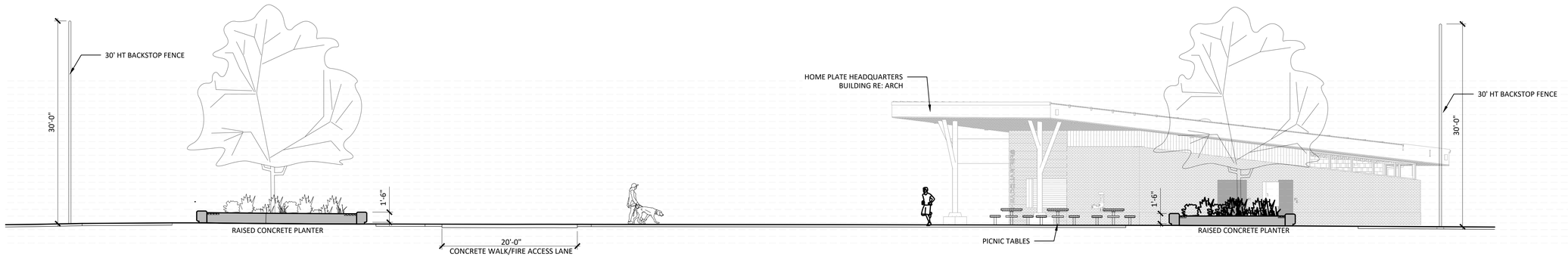
SITE PLAN SUBMITTAL

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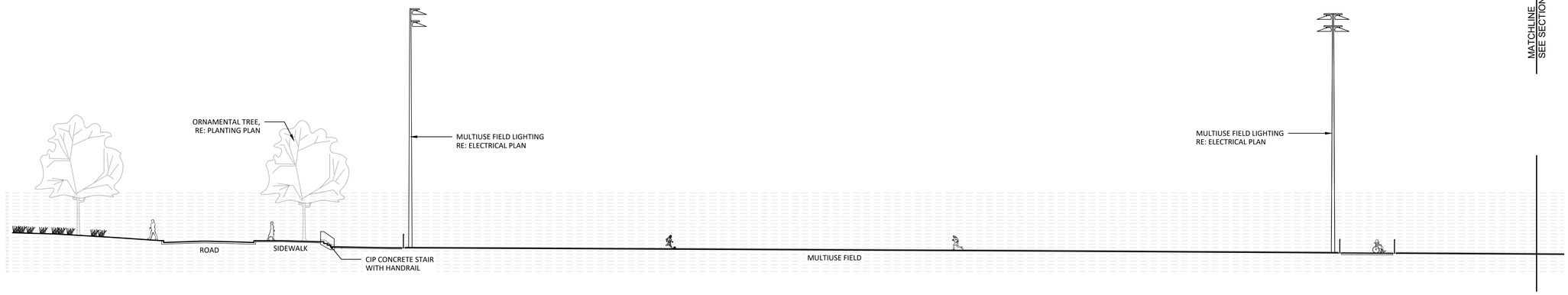
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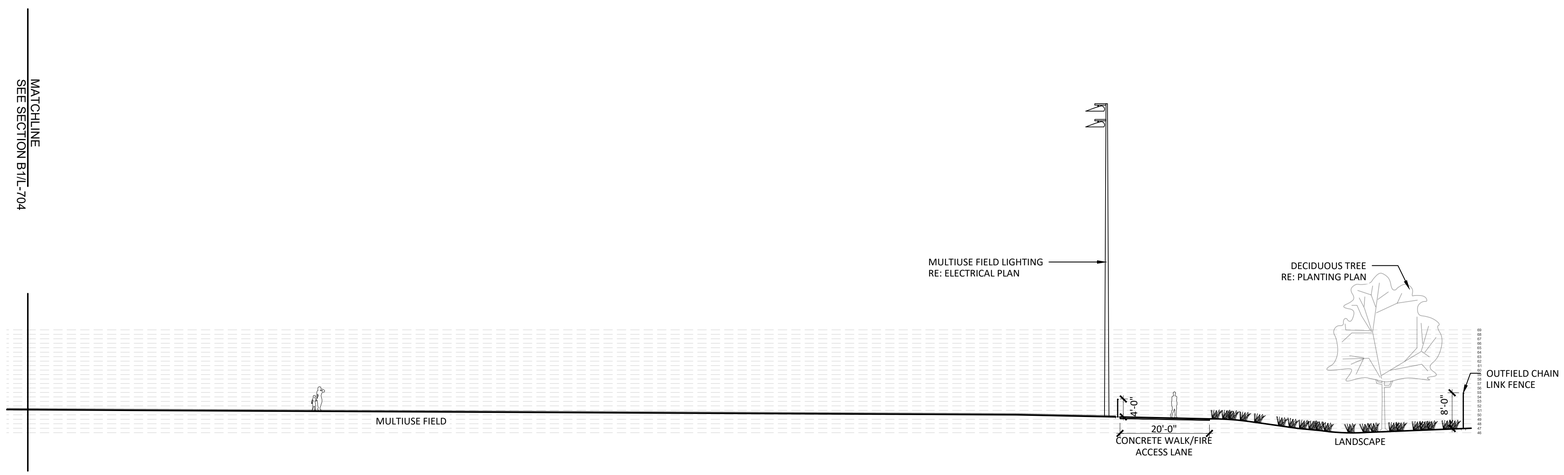
D1 BALLFIELD COMPLEX SECTION - D1

1/8" = 1'-0"



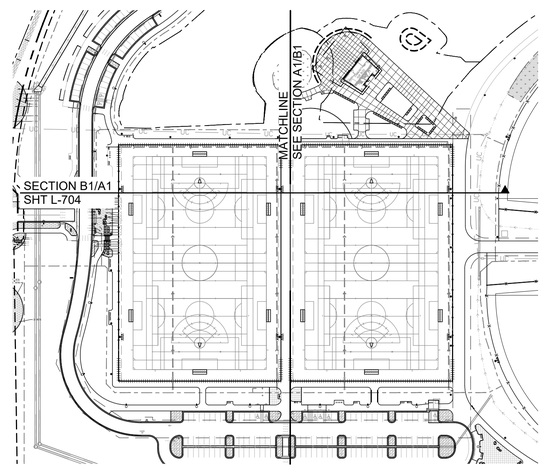
B1 MULTIPURPOSE FIELD SECTION - B1

1/16" = 1'-0"



A1 MULTIPURPOSE FIELD SECTION - A1

1/16" = 1'-0"



KEYMAP

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p. 303.607.0977

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1675 Larimer Street, #500
Denver, CO 80202
p. 303.444.1951

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ARCHITECTURE
LANDSCAPE ARCHITECTURE
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Key Map

Drawing

SITE SECTIONS - BALLFIELD COMPLEX & MULTI-USE FIELDS

L-704

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Salisbury Park - North - August , 2024

FIGURES SHOWN BELOW ARE FOR ALL IRRIGATED AREAS AT 100% BUILD-OUT OF PARK

ParkerWater & Sanitation District

LANDSCAPE/IRRIGATION WORKSHEET

Tap sizing for dedicated irrigation taps

Yellow fields require user input

Irrigation Water Requirement = (ETo x PF x LA)/IE

Define Landscape & Irrigation System:		
High Water Use Plant Material		
Cool season turfgrass (Kentucky bluegrass)		
Plant Factor (PF, %)	High	0.9
Landscape Area (LA, sq ft)	365100	
Irrigation Efficiency (IE, %)	Overhead	0.65
High-Moderate Water Use Plant Material		
Hybrid grass types (Texas hybrids, fescue, etc.)		
Plant Factor (PF, %)	High-Med	0.8
Landscape Area (LA, sq ft)		
Irrigation Efficiency (IE, %)	Overhead	0.65
Moderate Water Use Plant Material		
Traditional trees/shrubs/perennials, alternative grass types (buffalograss, blue grama, etc.)		
Plant Factor (PF, %)	Med	0.5
Landscape Area (LA, sq ft)	0	
Irrigation Efficiency (IE, %)	Drip	0.9
Low Water Use Plant Material		
Xeric trees/shrubs/perennials, native grass mixes		
Plant Factor (PF, %)	Low	0.25
Landscape Area (LA, sq ft)	592310	
Irrigation Efficiency (IE, %)	Overhead	0.65

*PWSD holds final discretion for determining appropriate plant factor designations based on landscape plan submission

Visit Appendix B of the SMWSA Model Ordinance for Plant Factor Guidelines

Site Summary:	
Total Irrigated Area (sq ft)	957410
Avg Plant Factor (%)	0.50
Avg Irrigation Efficiency (%)	0.65

Peak monthly ET, Parker, CO:		
Peak monthly ET (July):	7.56	in

Irrigation Water Requirement (IWR):		
Peak monthly IWR:	3437286	gal
Maximum flow requirement	359.3	GPM

* Max. flow requirements are based on peak monthly irrigation demands and follow PWSD's watering guidelines

Hydraulic Requirements:	
Tap Size (in)	**Max GPM v = 7.5 fps**
3/4"	10
1"	18
1 1/2"	40
2"	71
3"	189
4"	380

* 2" and smaller assumes Seamless K Copper Tube
3" and larger assumes Ductile Iron Pipe Class 350

Tap Size Requirement	4"

System Design Pressure: PSI

☐ Design pressure has been field tested & verified

☐ This form has been completed in compliance with PWSD's Engineering Standards & Specifications Manual regarding irrigation design specifications

Irrigation Construction Notes

1. ALL BASE INFORMATION HAS BEEN TAKEN FROM DRAWINGS PREPARED BY HORD COPLAN MACHT.
2. REFER TO TECHNICAL SPECIFICATIONS AND CONSTRUCTION DETAILS FOR INSTALLATION PROCEDURES.
3. CONTRACTOR SHALL FIELD VERIFY PRESSURE AT BACKFLOW PREVENTER LOCATION FOR EACH TAP PRIOR TO ORDERING MATERIALS OR STARTING ANY IRRIGATION INSTALLATION AND NOTIFY CONSULTANT OF ANY DIFFERENCES FROM STATED PRESSURE. IF CONTRACTOR FAILS TO NOTIFY CONSULTANT HE ASSUMES FULL RESPONSIBILITY FOR ANY SYSTEM ALTERATIONS. SYSTEM HAS BEEN DESIGNED FOR A STATIC PRESSURE OF XX PSI (PER PARKER WATER AND SANITATION DISTRICT).

TAP LOCATION/NUMBER	AVAILABLE PRESSURE	REQUIRED PRESSURE
1	110 PSI	90 PSI

4. CONTRACTOR SHALL COORDINATE INSTALLATION OF SLEEVING WITH INSTALLATION OF CONCRETE FLATWORK AND PAVING. ALL SLEEVING IS BY CONTRACTOR UNLESS OTHERWISE NOTED. UNLESS NOTED OTHERWISE ON IRRIGATION PLANS INSTALL SLEEVING BASED ON SLEEVE SIZING GUIDE BELOW:

PIPE SIZE OR WIRE QUANTITY	REQUIRED SLEEVE
3/4" - 1" PIPING	1-2" PVC SLEEVE
1-1/2" - 2" PIPING	1-4" PVC SLEEVE
2-1/2" - 3" PIPING	1-6" PVC SLEEVE
4" PIPING	1-8" PVC SLEEVE
6" PIPING	1-12" PVC SLEEVE
TWO-WIRE DECODER CABLE	1-2" PVC SLEEVE

NOTE: EACH LENGTH OF SLEEVED PIPE SHOWN SHALL BE ROUTED THROUGH SEPARATE SLEEVE. IRRIGATION TWO-WIRE CABLE SHALL BE ROUTED IN SEPARATE SLEEVE/CONDUIT WITHOUT IRRIGATION PIPING

5. WHERE NOT NOTED ON IRRIGATION PLANS CONTRACTOR TO INSTALL PLASTIC 15 SERIES NOZZLES ON POP-UP SPRAY HEADS SPACED GREATER THAN 12 FEET, INSTALL 12 SERIES NOZZLES ON POP-UP SPRAY HEADS SPACED 10-12 FEET, INSTALL 10 SERIES NOZZLES ON ALL POP-UP SPRAY HEADS SPACED 8-10 FEET. INSTALL 8 SERIES NOZZLES ON POP-UP SPRAY HEADS SPACED 8 FEET AND LESS.
6. REFER TO PLANTING PLAN FOR EXACT TREE LOCATIONS AND QUANTITIES, TREES SHOWN ON IRRIGATION PLANS ARE APPROXIMATE.
7. CONTRACTOR SHALL REPAIR OF REPLACE ANY EXISTING IRRIGATION EQUIPMENT, TURF, PLANT MATERIAL OR SITE FEATURES DAMAGED DURING NEW INSTALLATION. REPLACEMENT OR REPAIR OF DAMAGED EQUIPMENT OR MATERIAL SHALL BE DETERMINED BY THE OWNER AND THE CONSULTANT.
8. VALVE BOXES SHALL BE LOCATED 36" MINIMUM FROM CENTERLINE OF ALL SWALES, 24" MINIMUM FROM EDGES OF ALL WALKS, CURBS, DRIVES AND OTHER HARD SURFACE AREAS.
9. REFER TO IRRIGATION TECHNICAL SPECIFICATIONS FOR PLANTING AND IRRIGATION LAY-OUT REQUIREMENTS, COORDINATION AND PRIORITIES.
10. LATERAL PIPING DIAMETERS SHALL INCLUDE 1", 1 1/2", 2" AND 2 1/2". 3/4" AND 1 1/4" DIAMETER LATERAL PIPE IS NOT ACCEPTABLE.
11. ALL VALVE BOXES AND LIDS TO BE PURPLE.
12. TWO-WIRE CABLE SHALL BE PAIGE ELECTRIC P7072-D 14 GA. CABLE. USE 3M CO. DBR/Y-6 CONNECTORS FOR ALL CABLE SPLICES (RE: SPECS.).

Irrigation Equipment and Materials Schedule

SYMBOL	MANUFACTURER	MODEL NO.	DESCRIPTION	DETAIL NO.	COMMENTS
●,	RAIN BIRD	RD06-S-P30-F W/ SPEC'D. NOZZLE	POP-UP SPRAY HEAD	2	PURPLE CAP
■, ■, ■, ■	RAIN BIRD	RD12-S-P30-F W/ SPEC'D. NOZZLE	POP-UP SPRAY HEAD	2	
12-25, 12-25, 12-25, 12-30, 12-30, 12-30	HUNTER	I-20-12 W/ MPR-25 SERIES NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
	HUNTER	I-20-12 W/ MPR-30 SERIES NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-2.0LA	HUNTER	I-20-12 W/ 2.0LA NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-2.5LA	HUNTER	I-20-12 W/ 2.5LA NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-3.5LA	HUNTER	I-20-12 W/ 3.5LA NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-2.5	HUNTER	I-20-12 W/ 2.5 NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-3.0	HUNTER	I-20-12 W/ 3.0 NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-4.0	HUNTER	I-20-12 W/ 4.0 NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
12-5.0	HUNTER	I-20-12 W/ 5.0 NOZZLE	HI-POP GEAR DRIVEN ROTOR	1	
● 4.0	HUNTER	I-20-06-22 W/ 4.0 NOZZLE	GEAR DRIVEN ROTOR	1	
● 6.0	HUNTER	I-20-06-SS W/ 6.0 NOZZLE	GEAR DRIVEN ROTOR	1	
● 07	HUNTER	I-25-06-SS W/ 07 NOZZLE	GEAR DRIVEN ROTOR	1	
● 08	HUNTER	I-25-06-SS W/ 08 NOZZLE	GEAR DRIVEN ROTOR	1	
● 10	HUNTER	I-25-06-SS W/ 10 NOZZLE	GEAR DRIVEN ROTOR	1	
● 13	HUNTER	I-25-06-SS W/ 13 NOZZLE	GEAR DRIVEN ROTOR	1	
● 15	HUNTER	I-25-06-SS W/ 15 NOZZLE	GEAR DRIVEN ROTOR	1	
◕	RAIN BIRD	RWS-M-B-C-1402 W/ RWS-SOCK	ROOT WATERING SYSTEM	2	
⬡	WEATHERTRAK	WTOXR-C-H2O200+SWM+SPT AND ANTENNA	ELECTRIC TWO-WIRE CONTROLLER W/ PEDESTAL & FLIP-TOP COVER	10 & 11	TWO-WIRE SYSTEM OPTI-FLOW
	FEBCO	LF860-NRS - 4" DIA.	BACKFLOW PREVENTER	24 & 25	
METER		4" DIA.	WATER METER		
⊕	RAIN BIRD	1" DIA.	DRIP VALVE ASSEMBLY	13	W/ DECODER
		CLASS 200 SOLVENT WELD - 1" DIA. UNLESS NOTED OTHERWISE	PVC LATERAL	8	
		CLASS 200 SOLVENT WELD (1-1/2" THRU 2-1/2" DIA.)	PVC MAINLINE	8	
		CLASS 200 GASKETED (3" DIA. & LARGER)	PVC MAINLINE	8, 9 & 27	
		CLASS 200 SOLVENT WELD	NEW PVC SLEEVING		
	NETAFIM	TLCV-9-12	IN-LINE DRIP TUBING	17, 18, 19 & 20	SHRUB BEDS
		80 PSI NSF - 3/4" DIA.	POLY DRIP TUBING	-	
		EXISTING	PVC SLEEVING		
S	WEATHERTRACK	WT2W-H20-SA	SURGE PROTECTOR/GROUND ROD	26	
R	IRRITROL	RFS-1000	WIRELESS RAIN/FREEZE SENSOR	12	
F	WEATHERTRAK	WTF3-400-RS-NC - 4" DIA.	HYDROMETER	25	REED SWITCH NORMALLY CLOSED FLANGED
▼	RAIN BIRD	1402	TREE BUBBLER	16	NATIVE SEED AREAS
	MATCO-NORCA	10 RT (GASKETED) - 3" DIA. & LARGER	GATE VALVE	23	
	MATCO-NORCA	10 RS (FIPTxFIPT) - 2" & 2-1/2" DIA.	GATE VALVE	22	
▽		LINE DIA.	DRIP LINE BLOW-OUT STUB	14 & 15	
▼	RAIN BIRD	44 LRC	QUICK COUPLING VALVE	3	
	RAIN BIRD	EFB-CP-PRS-D SERIES	ELECTRIC CONTROL VALVE	4	W/ DECODER
H		2" DIA.	FIRE HYDRANT CONNECTION	31	6
♦	A.Y. McDONALD	76101 - 3/4" DIA.	MANUAL DRAIN VALVE	5	
●	HUNTER	I-20-04 W/ 4.0 NOZZLE	ON-GRADE GEAR DRIVEN ROTOR	29	
		CLASS 200 SOLVENT WELD - 1" DIA. UNLESS NOTED OTHERWISE	ON-GRADE PVC LATERAL	29	
		CLASS 200 SOLVENT WELD	ON-GRADE PVC MAINLINE		
CPW		CONTROLLER & STATION NO.			
		CONTROL VALVE SIZE			

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Drawn By: DZ
Checked By: DZ

Key Map

Drawing

IRRIGATION COVER SHEET

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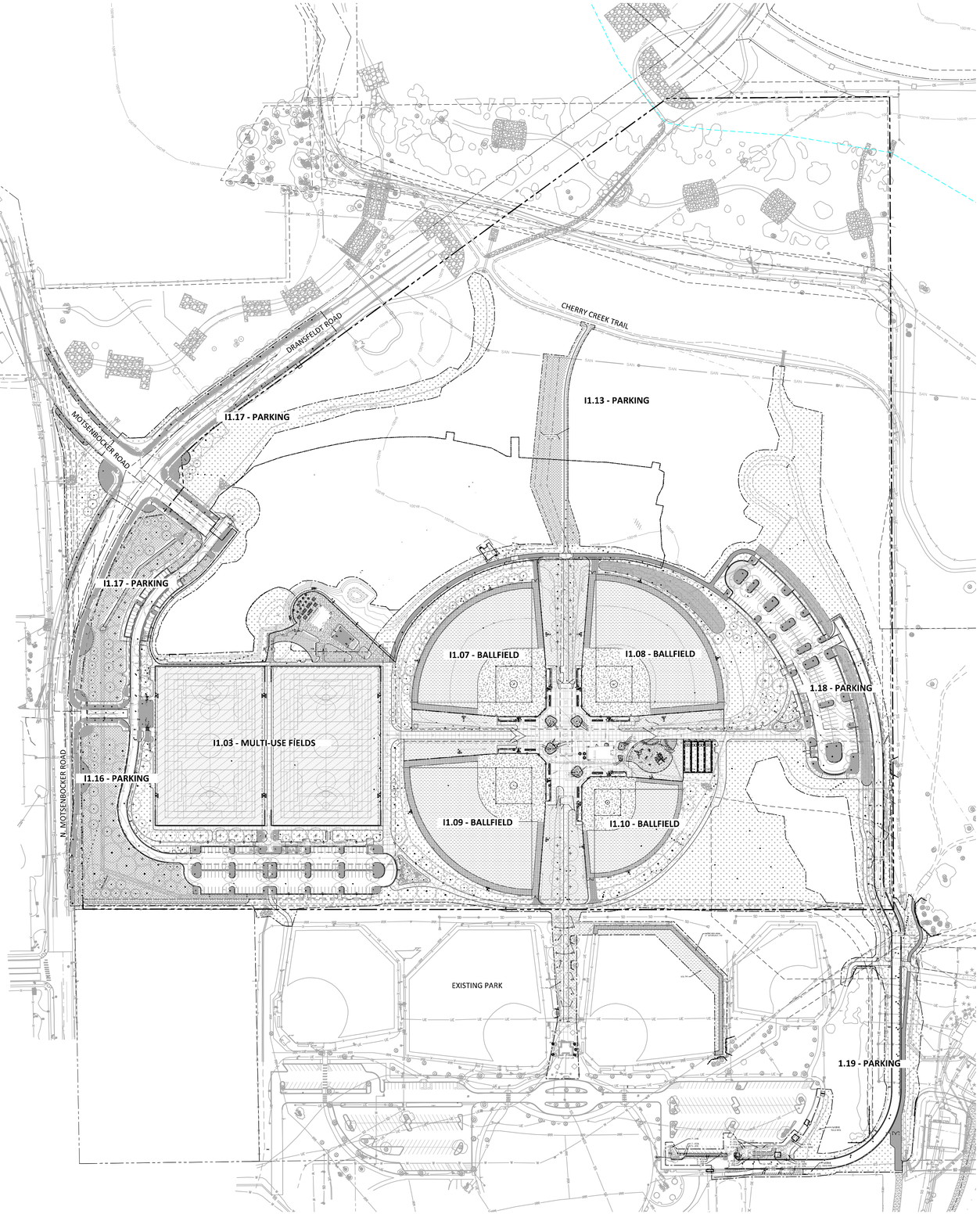
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A1 OVERALL IRRIGATION - BASE BID

1" = 140'-0"

SCALE: 1" = 140'-0"

0' 70' 140' 280' 420'

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OVERALL IRRIGATION PLAN- BASE BID

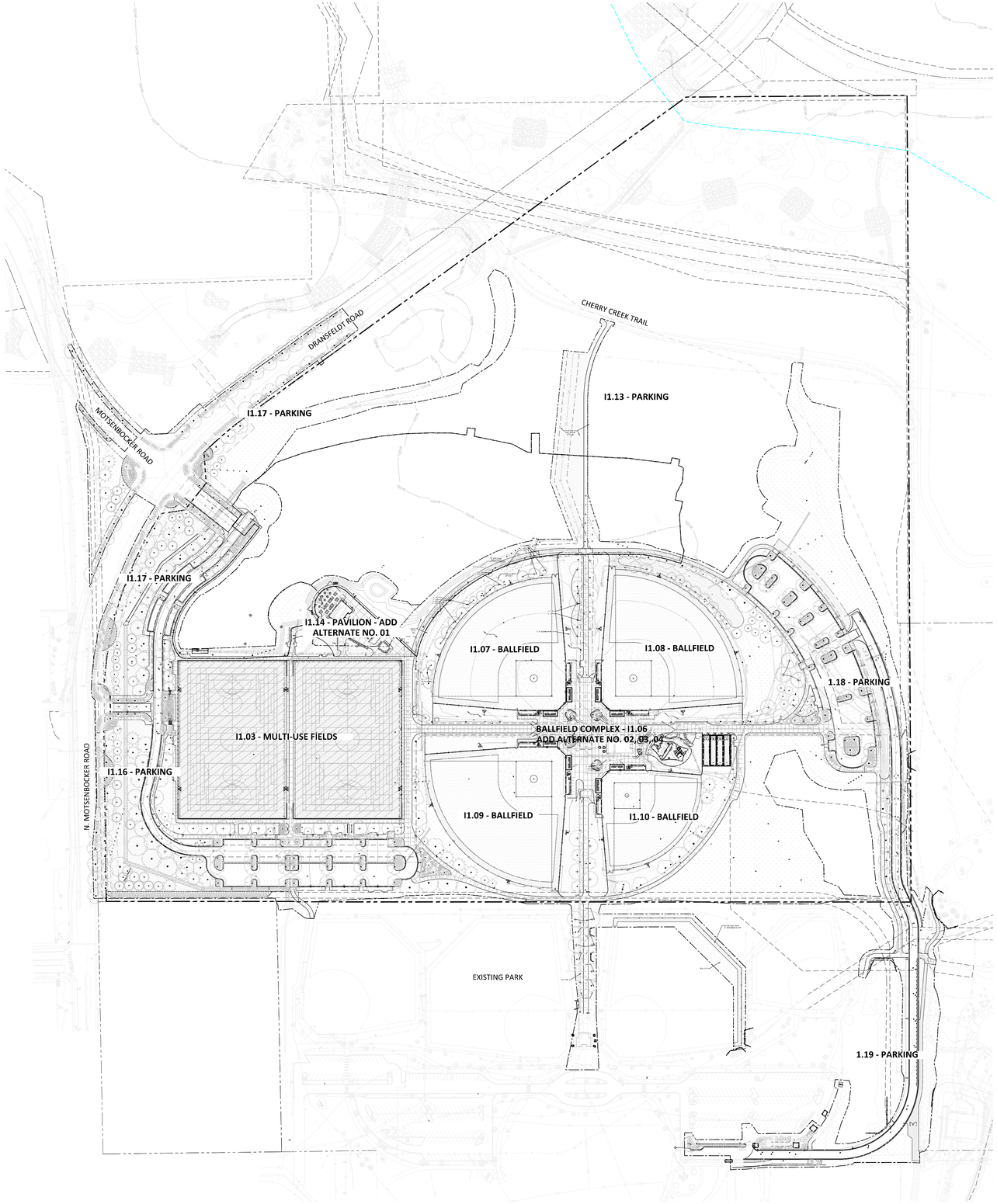
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A1 OVERALL IRRIGATION PLAN - ADD ALTERNATES

SCALE: 1" = 140'-0"

0' 70' 140' 280' 420'

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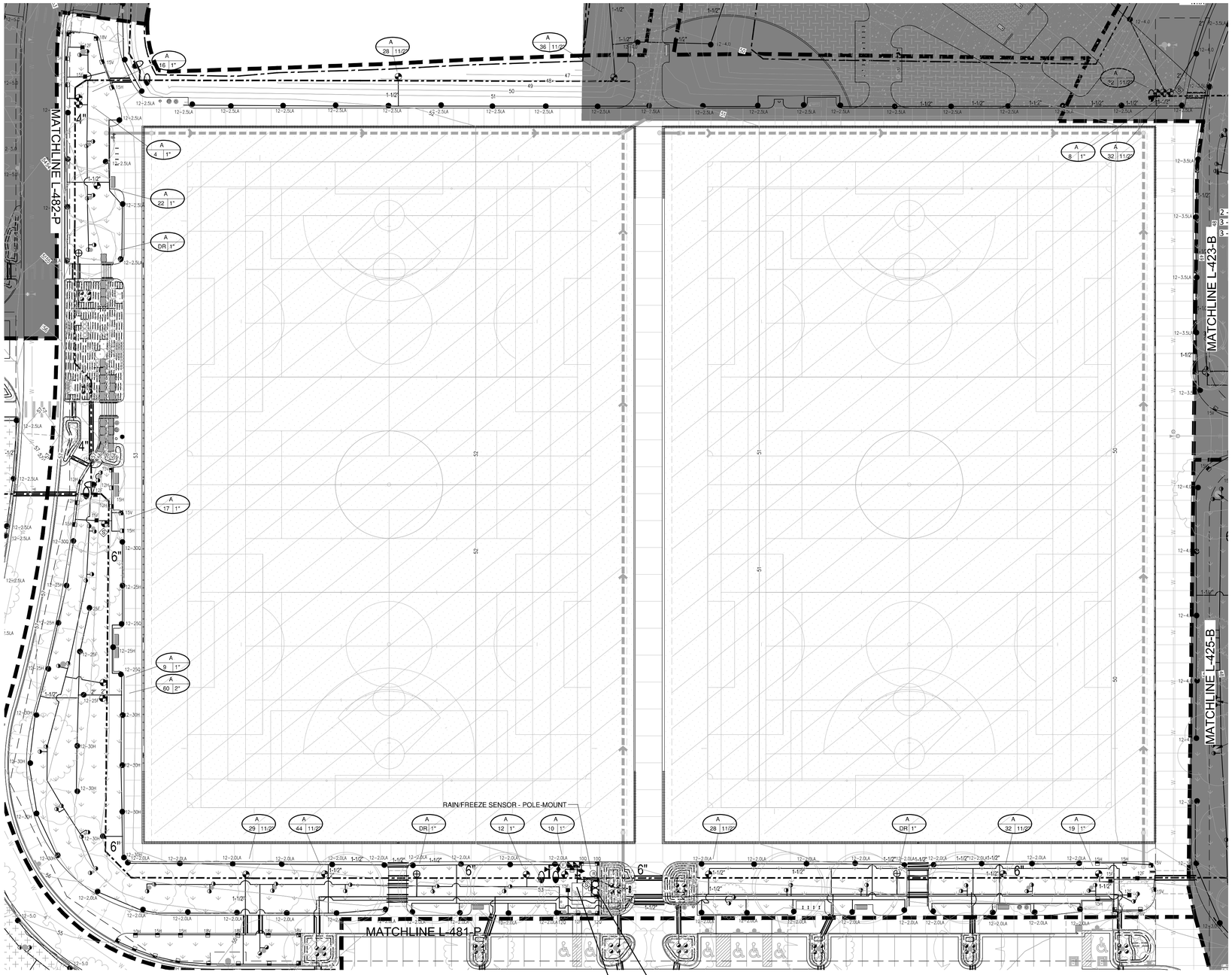
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OVERALL IRRIGATION PLAN- ALTERNATE

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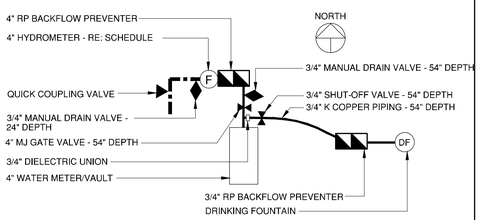


CONTROLLER LOCATION "A"

PEDESTAL MOUNT TWO WEATHERTRAK OPTI-FLOW SERIES 200 STATION TWO-WIRE CONTROLLER AT INDICATED LOCATION. 120 VOLT ELECTRICAL POWER IS AVAILABLE WITHIN 20 L.F. OF CONTROLLER LOCATION. ELECTRICAL POWER AND CONNECTION TO CONTROLLER IS BY CONTRACTOR WITH WORK CONFORMING TO LOCAL CODE. FEES AND PERMITS ASSOCIATED WITH WORK ARE TO BE OBTAINED AND PAID BY CONTRACTOR. FINAL LOCATION OF CONTROLLER SHALL BE REVIEWED AND APPROVED BY OWNER'S REPRESENTATIVE PRIOR TO INSTALLATION.

POINT OF CONNECTION - 4"

TIE ONTO 4" IRRIGATION WATER METER AT 54" DEPTH AT INDICATED LOCATION. EXTEND 4" CLASS 51 DUCTILE IRON PIPING AT 54" DEPTH TO BACKFLOW PREVENTER LOCATION. INSTALL 4"x4"x1" SIDE-GATED TEE (DRINKING FTN. CONNECTION), ONE 4" MJ GATE VAVE, ONE 3/4" MANUAL DRAIN VALVE, ONE 4" REDUCED PRESSURE BACKFLOW PREVENTER, ENCLOSURE, 4" HYDROMETER, ONE 3/4" MANUAL DRAIN VALVE, ONE QUICK COUPLING VALVE AND EXTEND CLASS 200 PVC MAINLINE AS SHOWN. WORK SHALL CONFORM TO LOCAL CODE. FEES AND PERMITS ASSOCIATED WITH WORK ARE TO BE OBTAINED AND PAID BY CONTRACTOR. FINAL LOCATION OF BACKFLOW PREVENTER SHALL BE REVIEWED AND APPROVED BY OWNER'S REPRESENTATIVE PRIOR TO START OF WORK.



REFER TO DETAIL, SHEET L-515, FOR DRINKING FOUNTAIN INSTALLATION

PT. OF CONNECTION ENLARGEMENT

NOT TO SCALE

A1 MULTI-USE FIELDS PLANTING ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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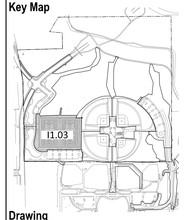
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Key Map



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MULTI-USE FIELDS IRRIGATION ENLARGEMENT

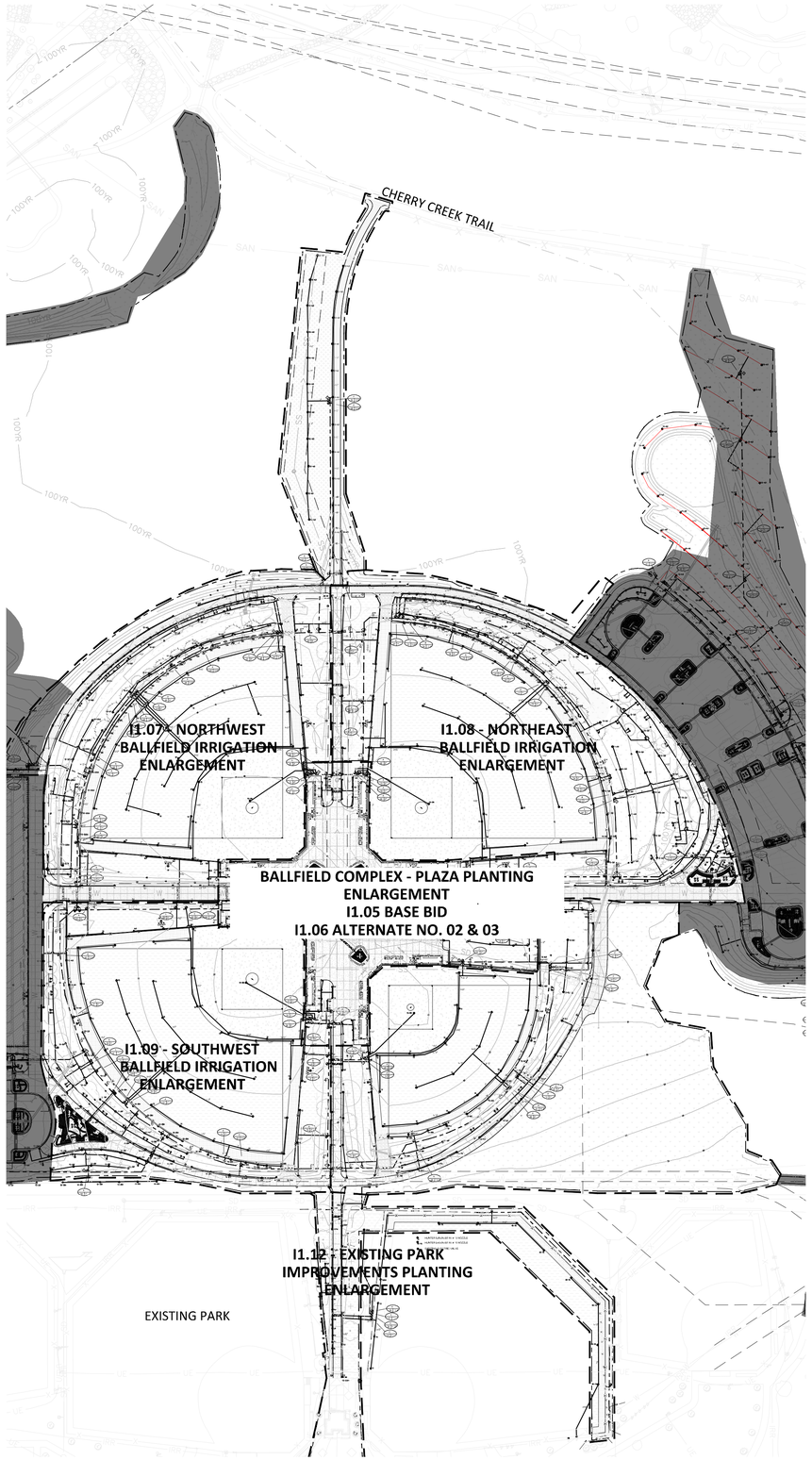
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A1 OVERALL BALLFIELD COMPLEX - IRRIGATION KEY PLAN

1"=100'-0"

SCALE: 1" = 100'-0"

0' 50' 100' 200' 300'

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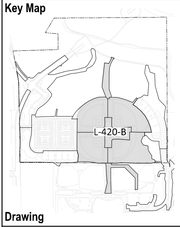
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OVERALL BALLFIELD COMPLEX - IRRIGATION KEY PLAN

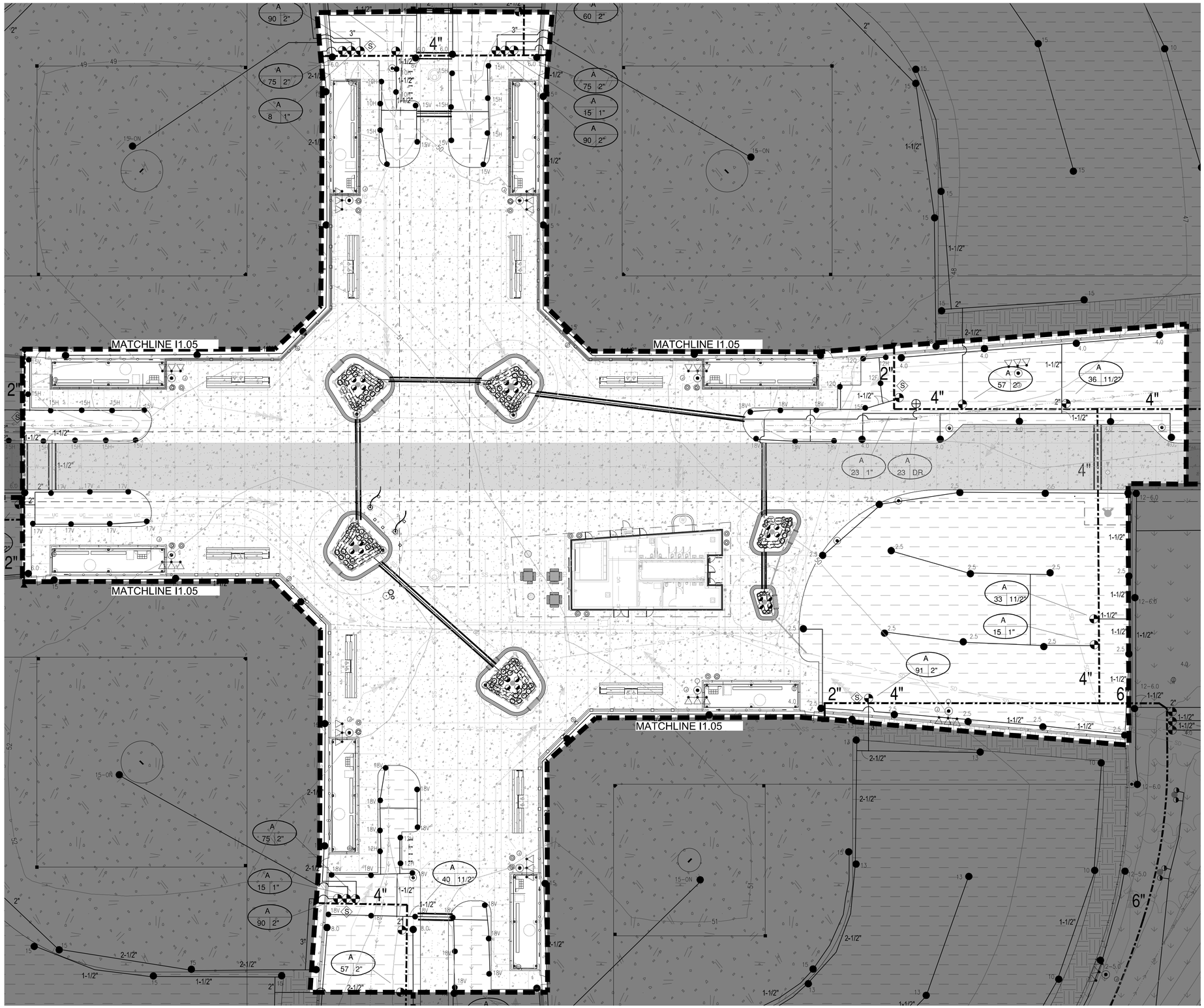
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A1 PLAZA IRRIGATION ENLARGEMENT - BASE BID

1" = 20'-0"

SCALE: 1" = 20'-0"

0' 10' 20' 40' 60'

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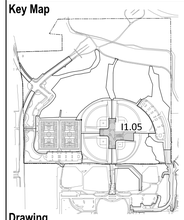
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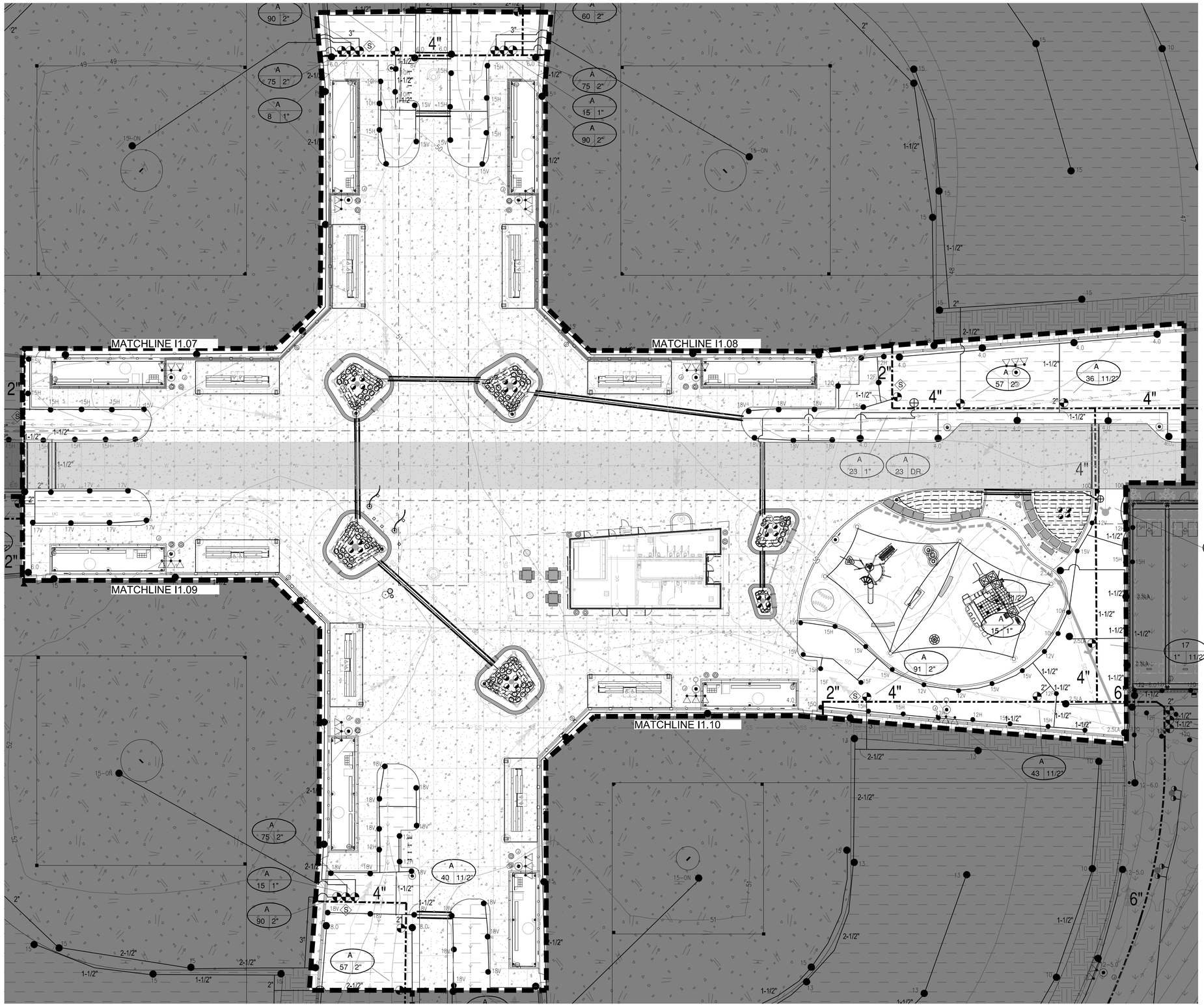
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PLAZA IRRIGATION ENLARGEMENT- BASE BID

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SITE PLAN SUBMITTAL
Page 219 of 311

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A1 **PLAZA IRRIGATION ENLARGEMENT - ADD ALTERNATES NO. 02 and 03**
1" = 20'-0"

SCALE: 1" = 20'-0"
0' 10' 20' 40' 60'

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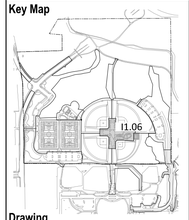
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Key Map



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PLAZA IRRIGATION ENLARGEMENT- ADD ALTERNATE NO. 02 & 03

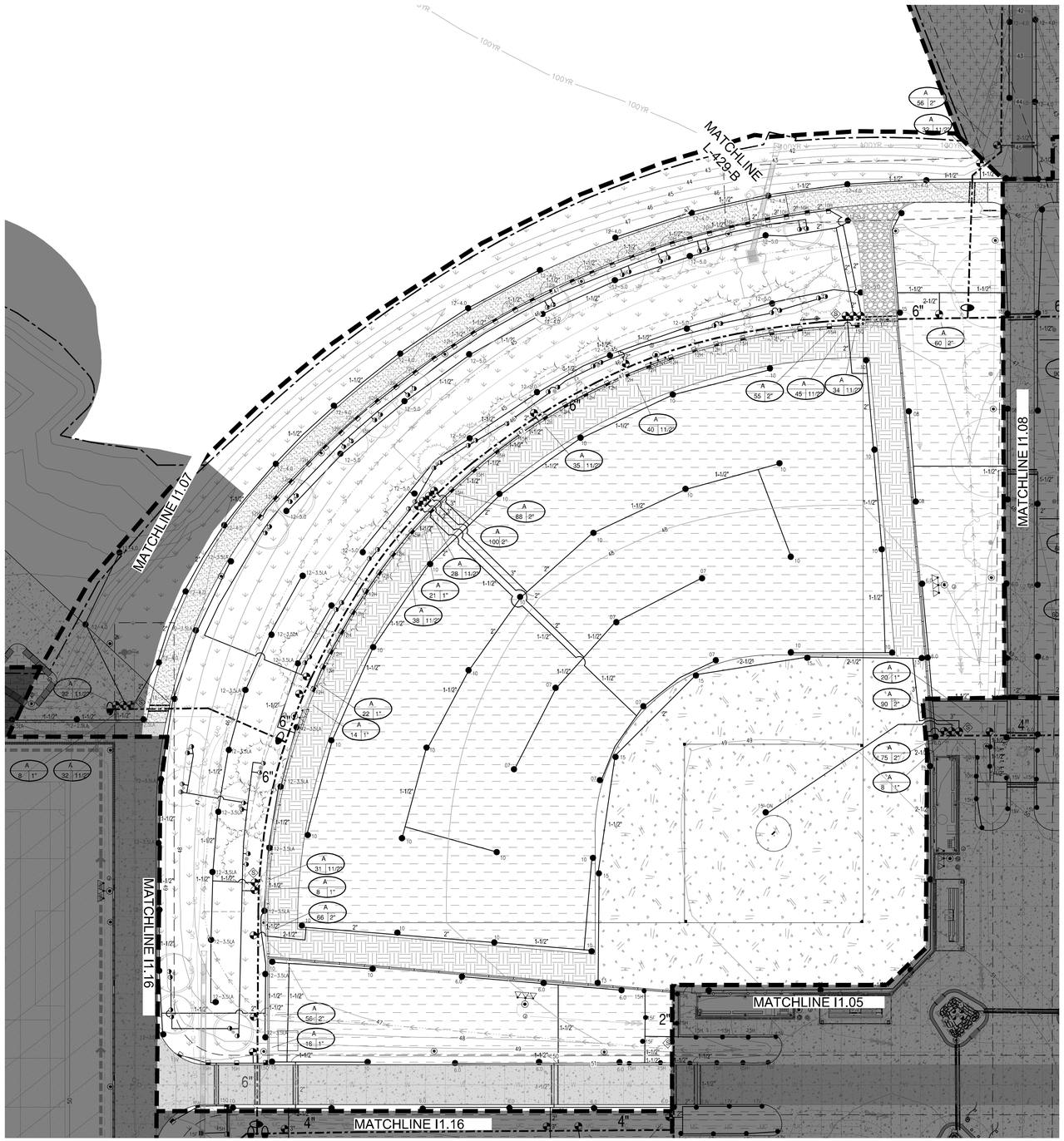
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A1 NORTHWEST BALLFIELD IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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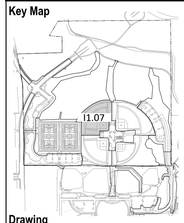
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Key Map



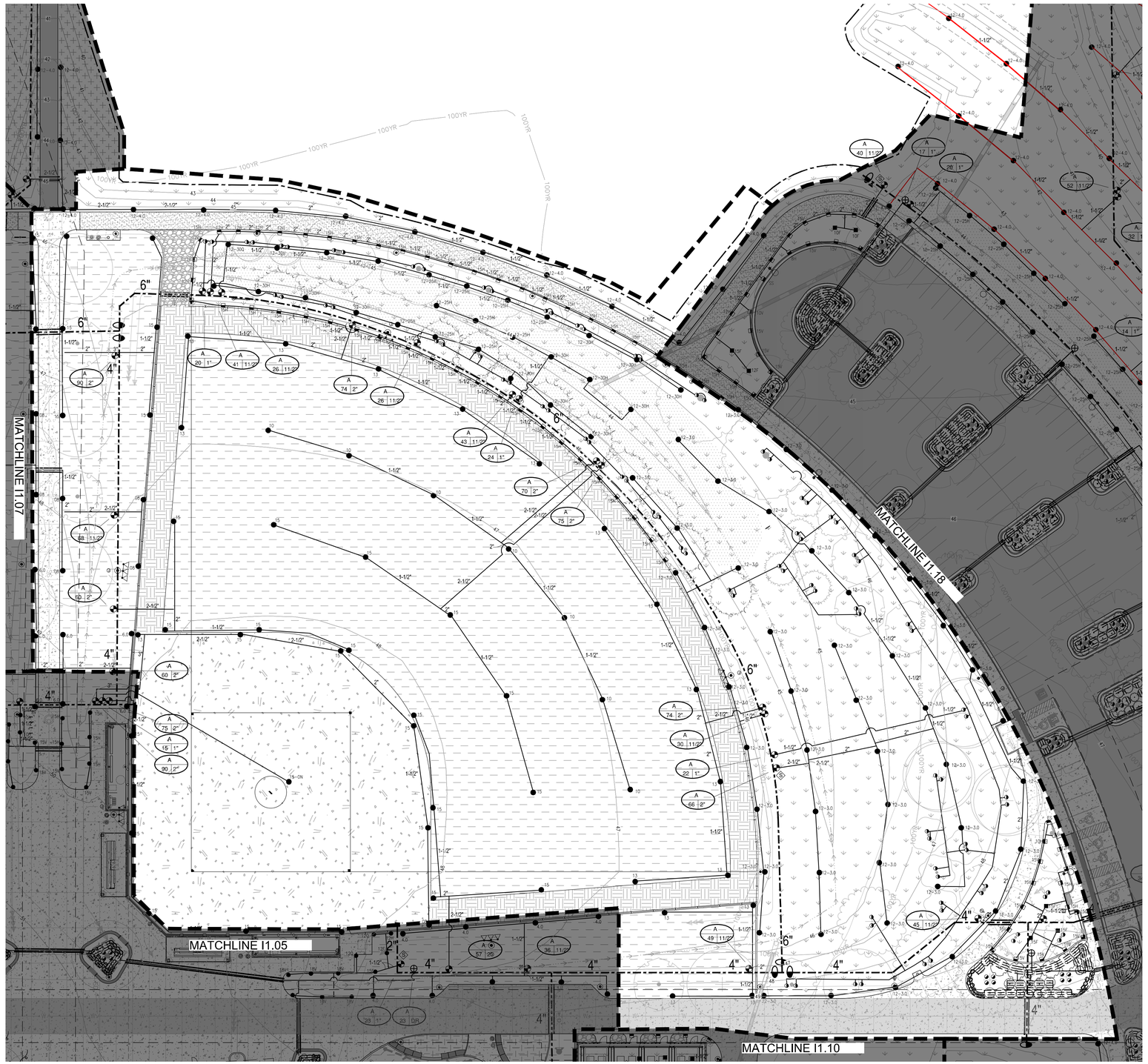
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NORTHWEST BALLFIELD IRRIGATION ENLARGEMENT

I1.07

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Page 221 of 311

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A1 NORTHEAST BALLFIELD IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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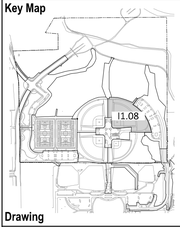
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Key Map



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NORTHEAST BALLFIELD IRRIGATION ENLARGEMENT

I1.08

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Page 222 of 311

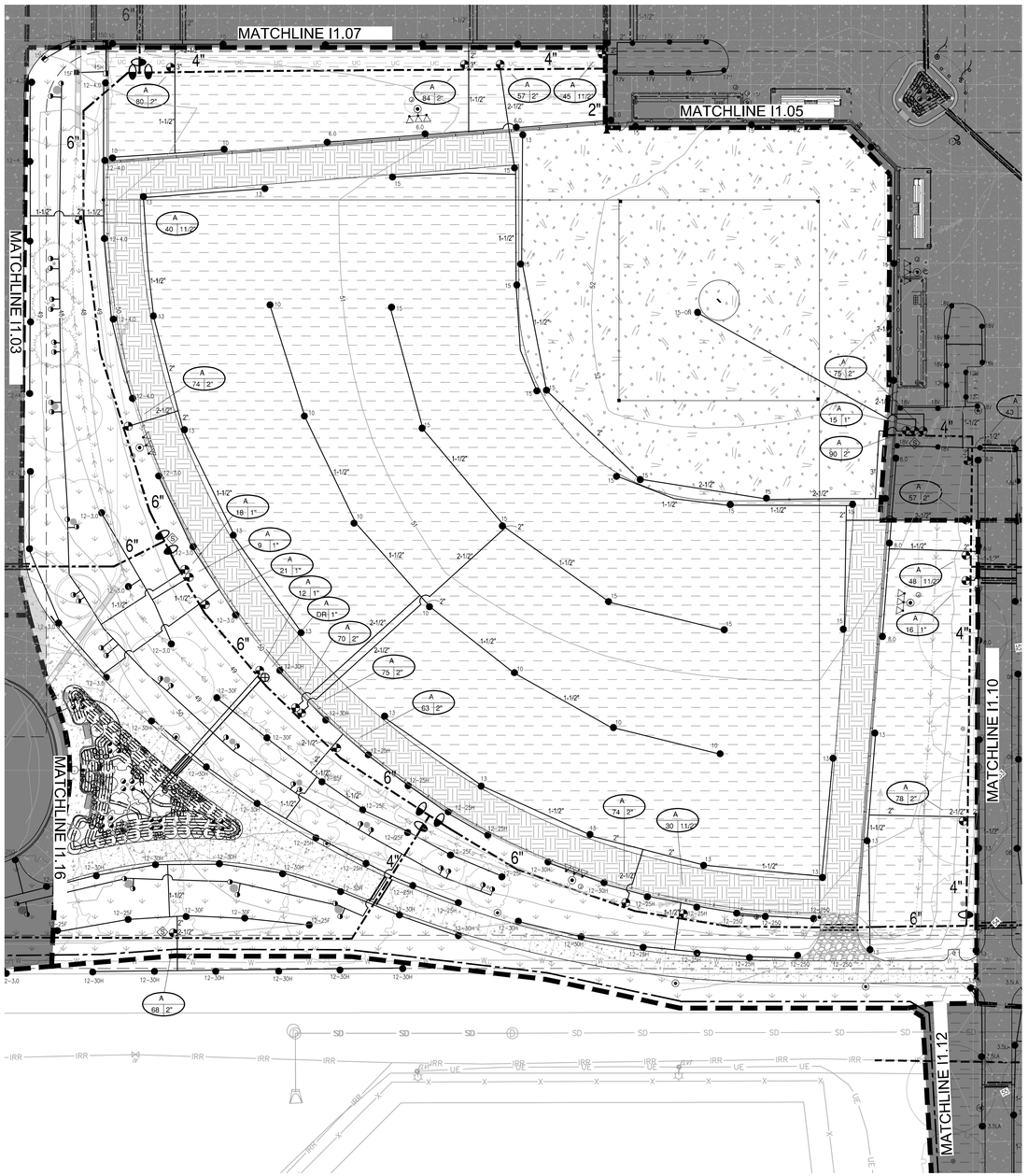
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A1 SOUTHWEST BALLFIELD IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

COLORADO 811 UTILITY NOTIFICATION CENTER

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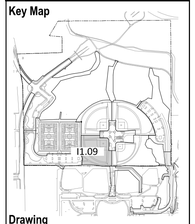
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Key Map



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SOUTHWEST BALLFIELD IRRIGATION ENLARGEMENT

I1.09

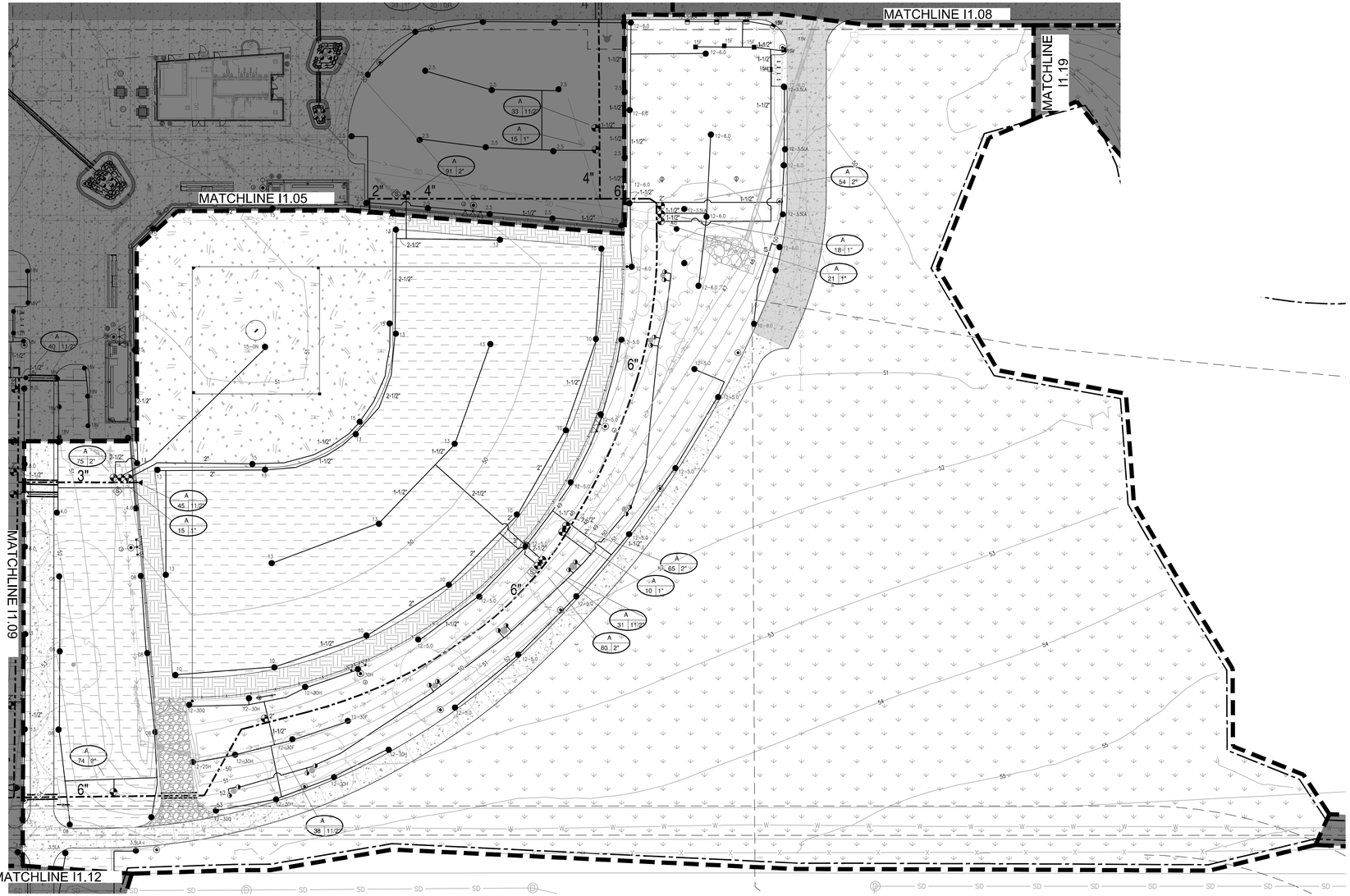
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A1 SOUTHEAST BALLFIELD IRRIGATION ENLARGEMENT - BASE BID

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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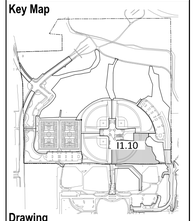
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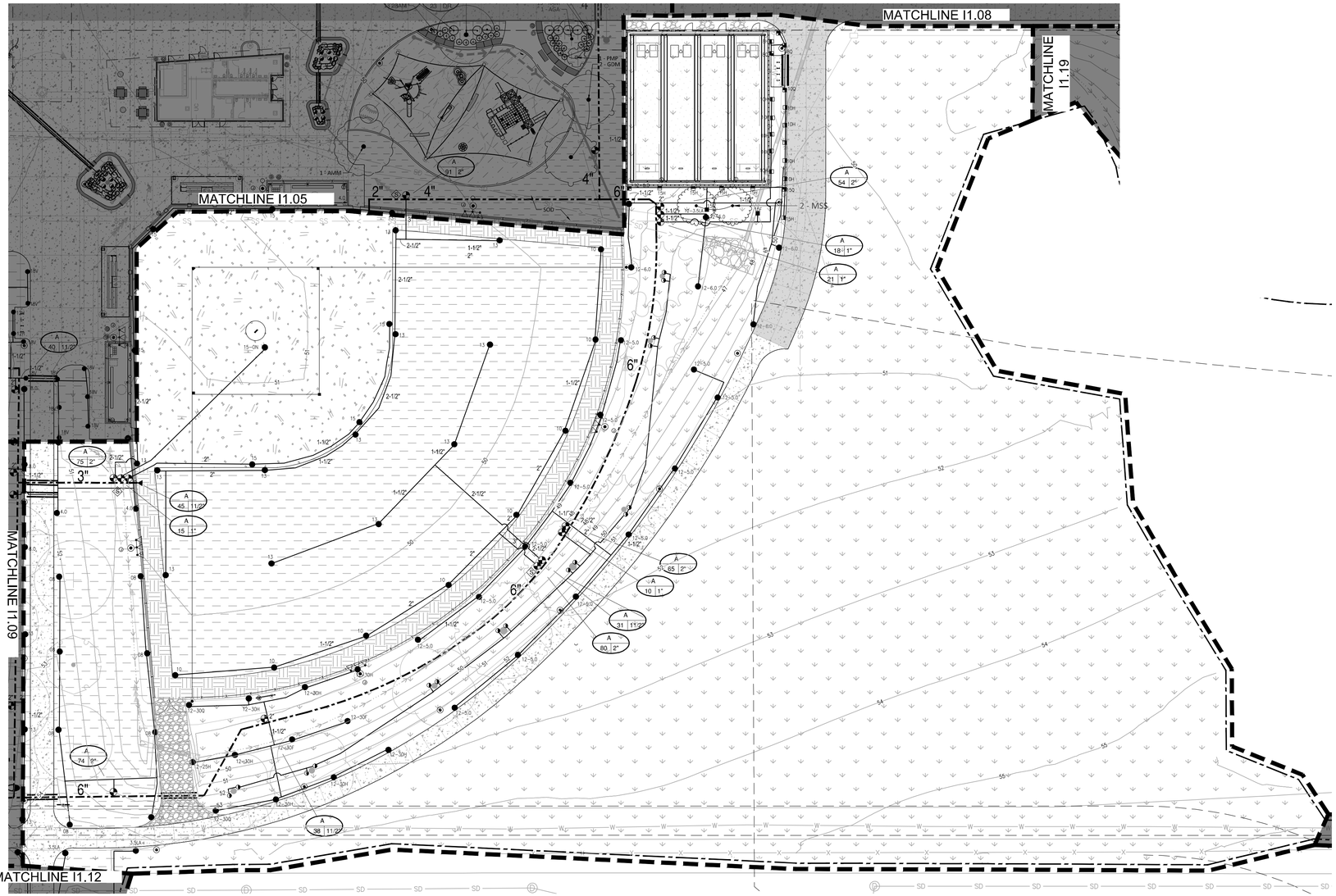
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SOUTHEAST BALLFIELD IRRIGATION ENLARGEMENT- BASE BID

I1.10

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A1 SOUTHEAST BALLFIELD IRRIGATION ENLARGEMENT - ADD ALTERNATE NO. 04

1" = 30'-0"

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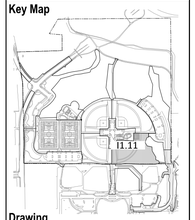
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SOUTHEAST BALLFIELD IRRIGATION ENLARGEMENT- ADD ALTERNATE NO. 04

I1.11

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Page 225 of 311

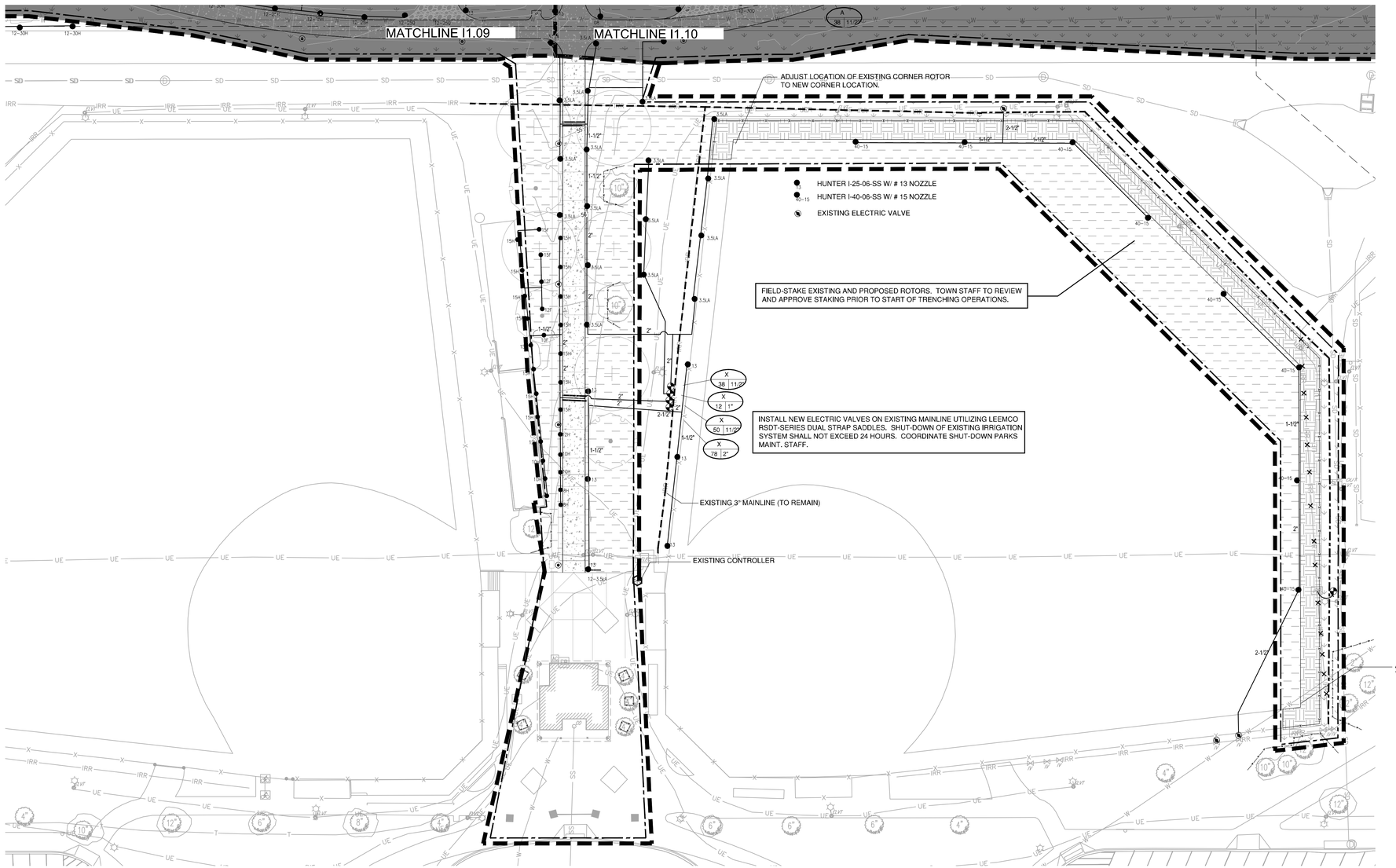
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A1 EXISTING PARK IMPROVEMENTS IRRIGATION ENLARGEMENT

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SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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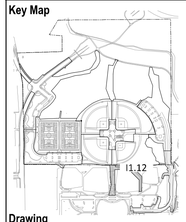
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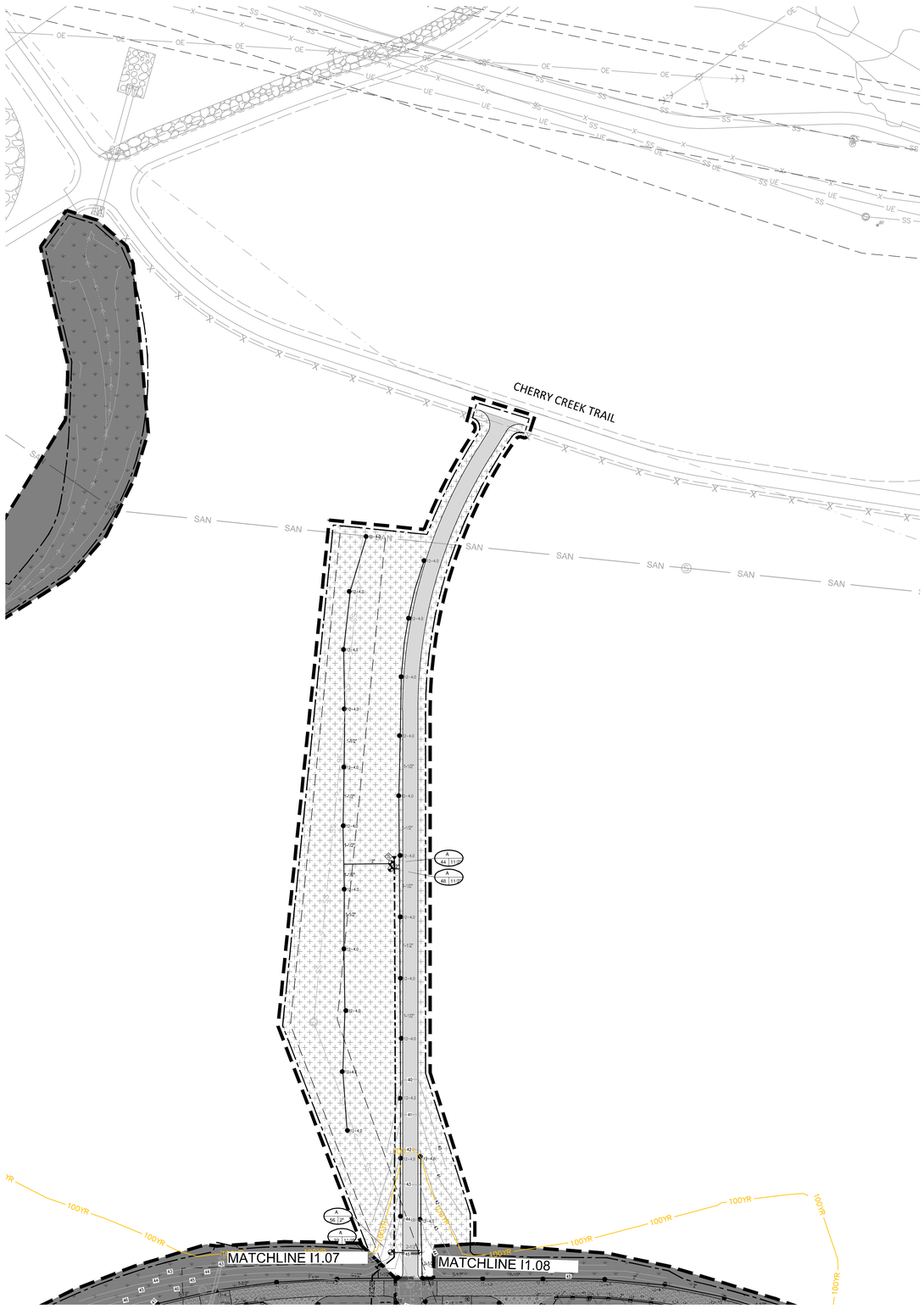
Drawing

EXISTING PARK IMPROVEMENTS IRRIGATION ENLARGEMENT

I1.12

SITE PLAN SUBMITTAL

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A1 CHERRY CREEK CONNECTION IRRIGATION ENLARGEMENT

1" = 40'-0"

SCALE: 1" = 40'-0"

0' 20' 40' 80' 120'

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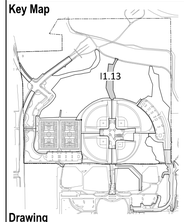
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Key Map



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CHERRY CREEK CONNECTION IRRIGATION ENLARGEMENT

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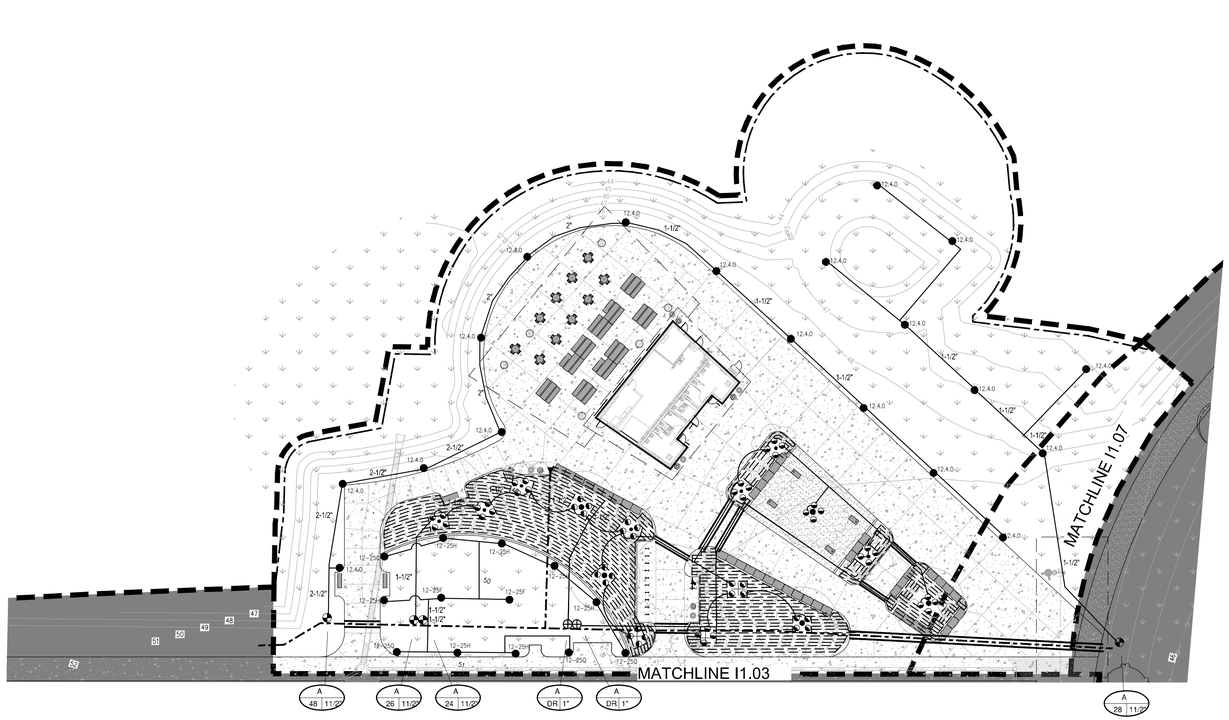
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A1 COMMUNITY HUB IRRIGATION ENLARGEMENT - ADD ALTERNATE NO. 01

1" = 30-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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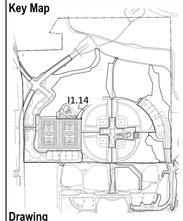
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COMMUNITY HUB IRRIGATION ENLARGEMENT- ADD ALTERNATE NO. 01

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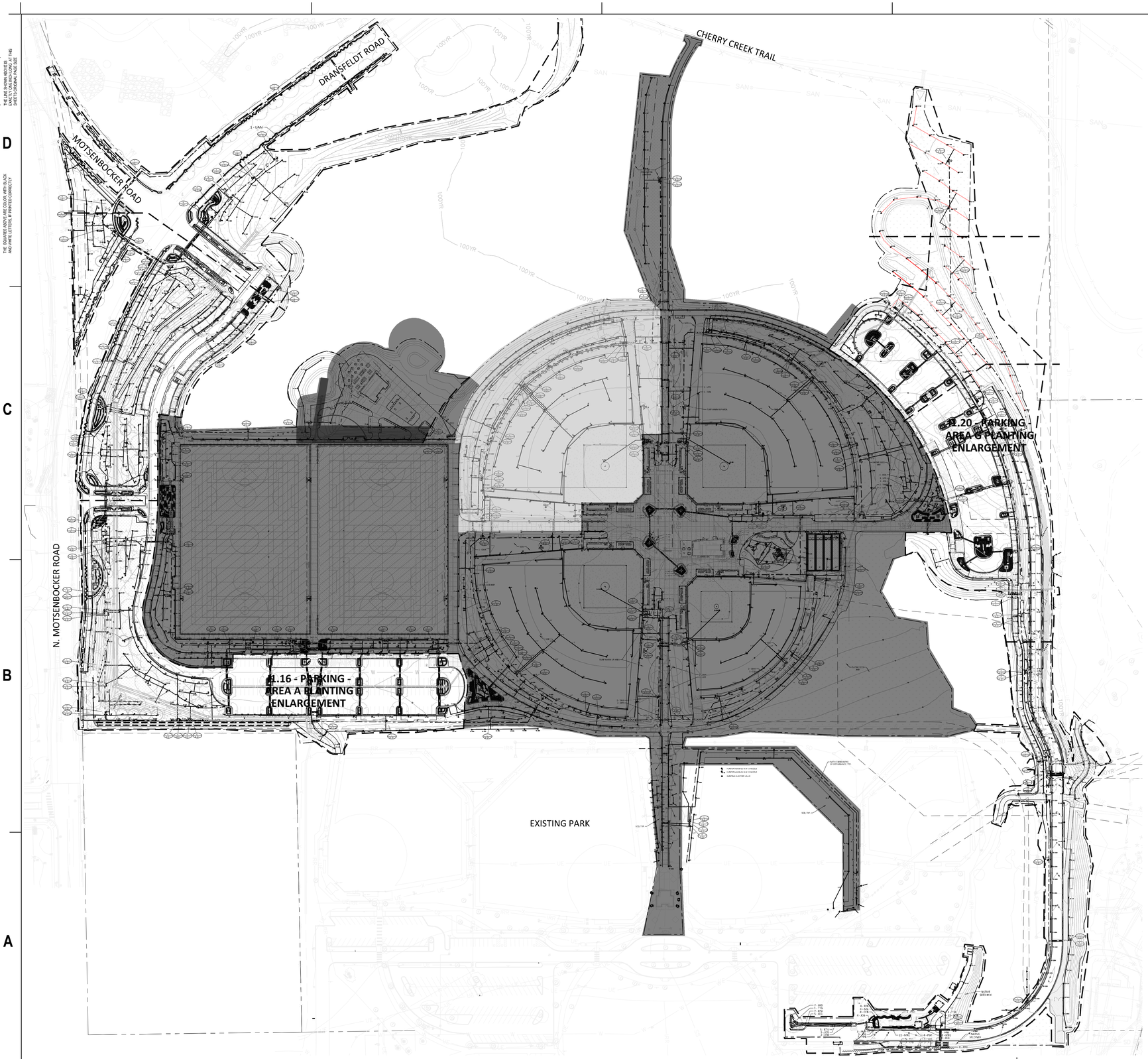
D

C

B

A

1 2 3 4 5



A1 OVERALL PARKING - IRRIGATION KEY PLAN

1" = 100'-0"

SCALE: 1" = 100'-0"

0' 60' 120' 240' 480'

COLORADO 811 — CALL 811 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR MARKING OF UNDERGROUND MEMBER UTILITIES

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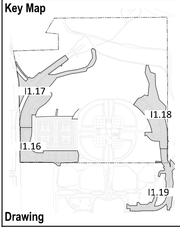
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Key Map



Drawing

OVERALL PARKING - IRRIGATION KEY PLAN

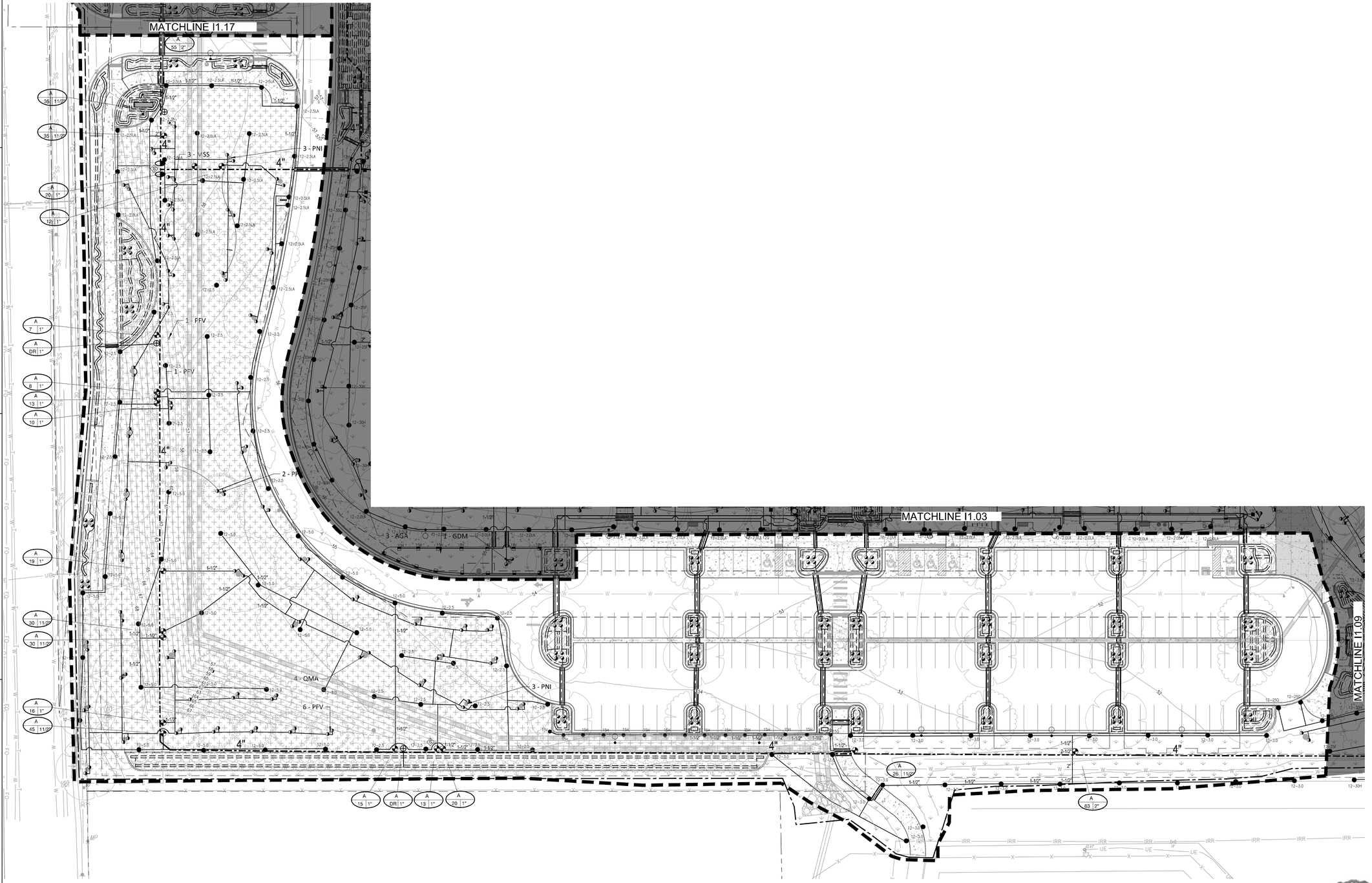
I1.15

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A1 PARKING - AREA A IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

COLORADO 811

CALL 811 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR MARKING OF UNDERGROUND MEMBER UTILITIES

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p. 303.607.0977

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Denver, CO 80202
p. 303.444.1951

ELECTRICAL ENGINEER
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3000 Youngfield Street, #264
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p. 303.278.7297

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ARCHITECTURE

LANDSCAPE ARCHITECTURE

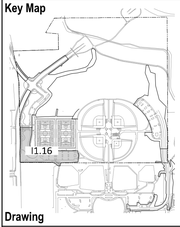
PLANNING

INTERIOR DESIGN

	DATE	DESCRIPTION

Project Number:	223072.00
Sheet Issue Date:	2024-11-22
Drawn By:	DZ
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Key Map



Drawing

PARKING - AREA A IRRIGATION ENLARGEMENT

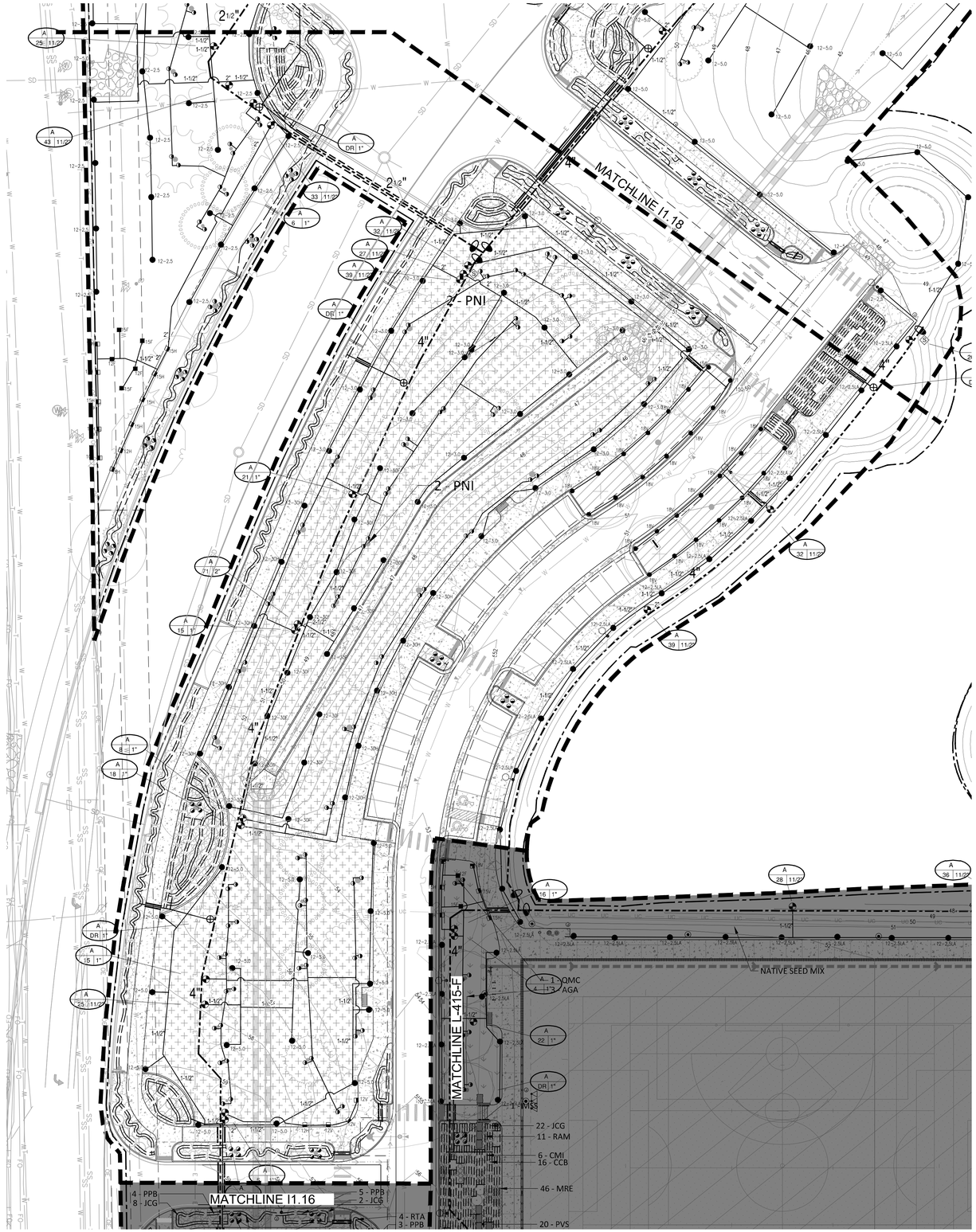
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Page 230 of 311

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A1 PARKING - AREA B IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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ARCHITECTURE

LANDSCAPE ARCHITECTURE

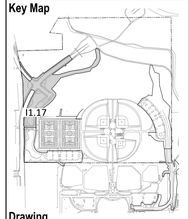
PLANNING

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Key Map



Drawing

PARKING - AREA B IRRIGATION ENLARGEMENT

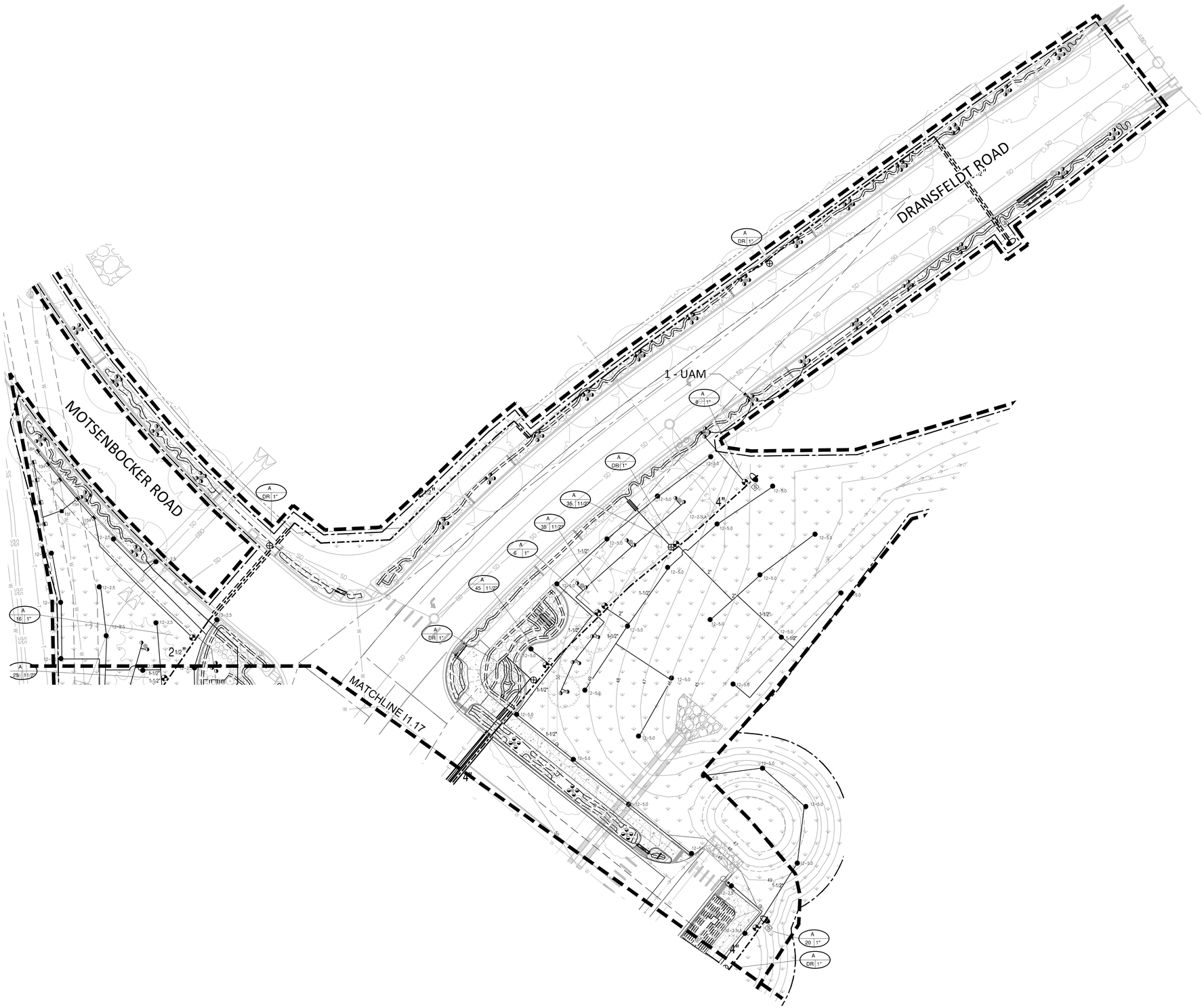
I1.17

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A1 PARKING - AREA B IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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LANDSCAPE ARCHITECTURE

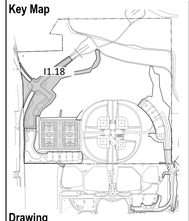
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Key Map



Drawing

PARKING - AREA B IRRIGATION ENLARGEMENT

I1.18

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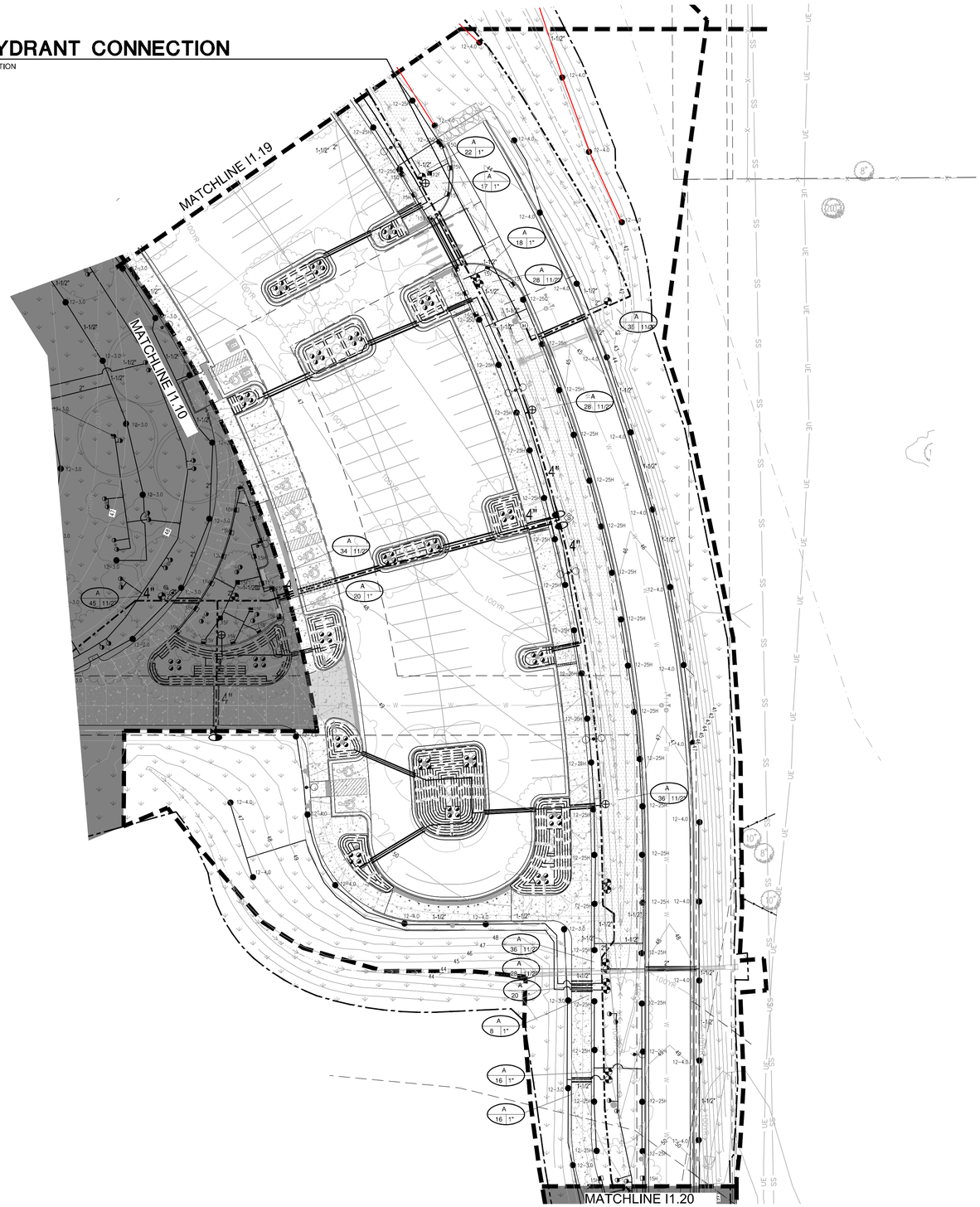
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C M Y K

FIRE HYDRANT CONNECTION

TEMPORARY IRRIGATION



A1 PARKING - AREA C IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'

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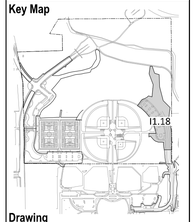
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		SCALE: 1" = 30'-0"

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Key Map



Drawing

PARKING - AREA C IRRIGATION ENLARGEMENT

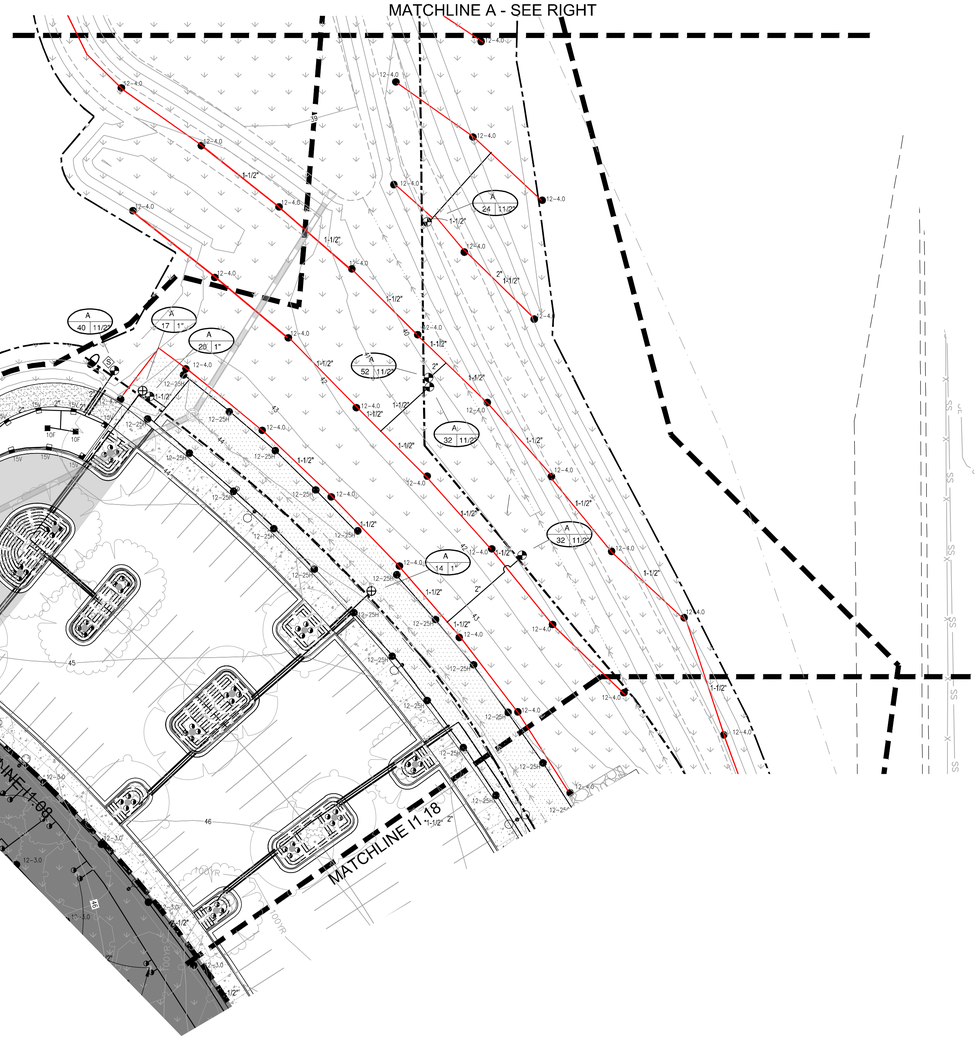
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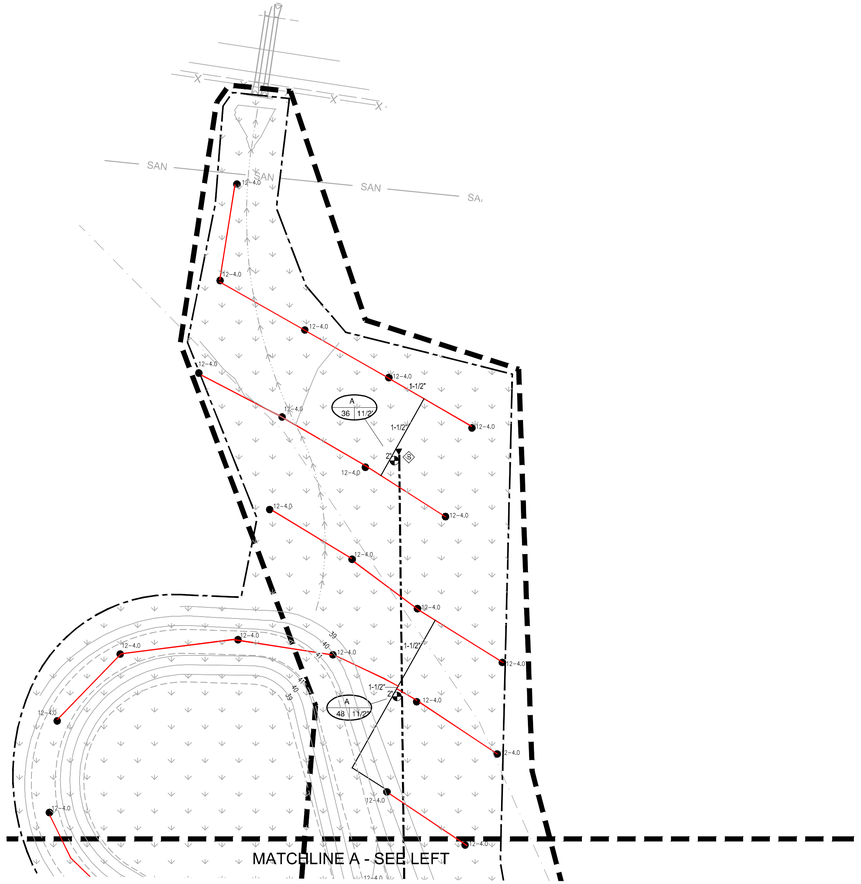


A1 PARKING - AREA C IRRIGATION ENLARGEMENT

1" = 30'-0"

SCALE: 1" = 30'-0"

0' 15' 30' 60' 90'



A2 PARKING - AREA C IRRIGATION ENLARGEMENT

1" = 30'-0"

0' 15' 30' 60' 90'

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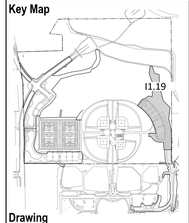
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LANDSCAPE ARCHITECTURE
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PARKING - AREA C IRRIGATION ENLARGEMENT

I1.20

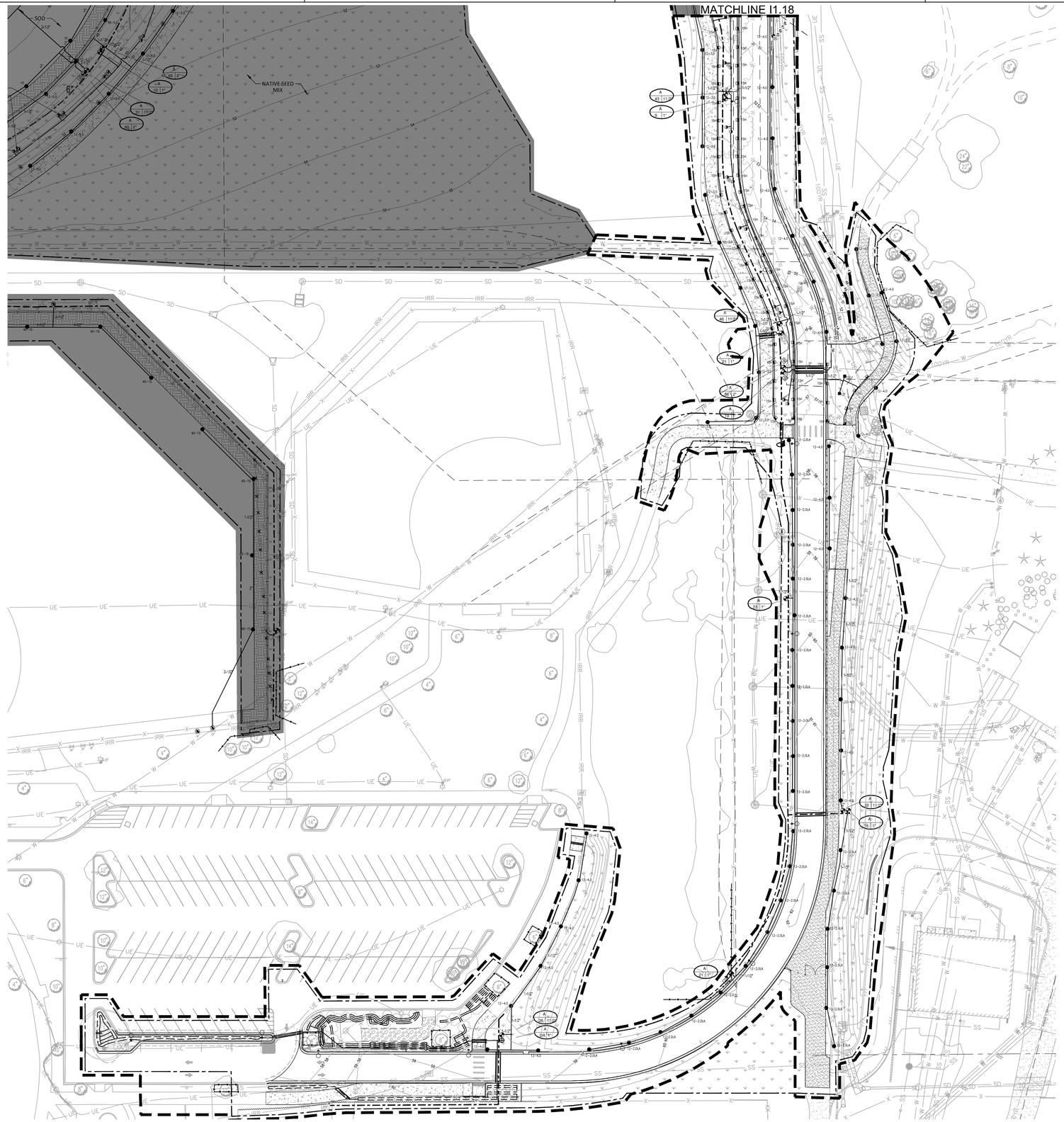
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MATCHLINE I1.18



A1 PARKING - AREA G IRRIGATION ENLARGEMENT

1" = 40'-0"

SCALE: 1" = 40'-0"

0' 20' 40' 80' 120'

CALL 811 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR MARKING OF UNDERGROUND MEMBER UTILITIES

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LANDSCAPE ARCHITECTURE

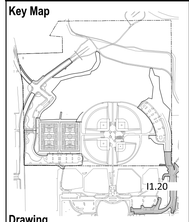
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Key Map



Drawing

PARKING - AREA G IRRIGATION ENLARGEMENT

I1.21

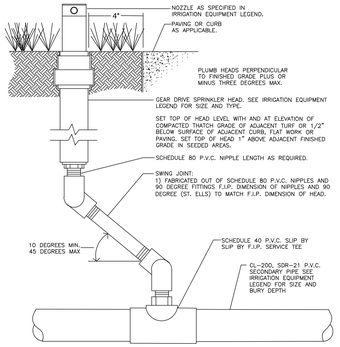
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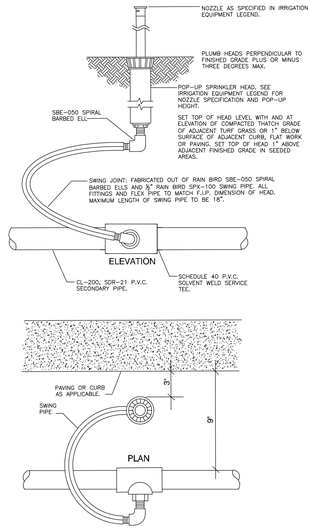
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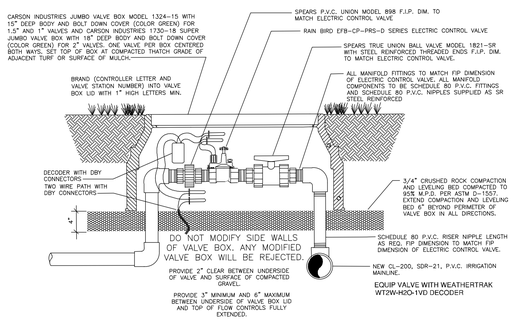
GEAR DRIVEN ROTOR

1



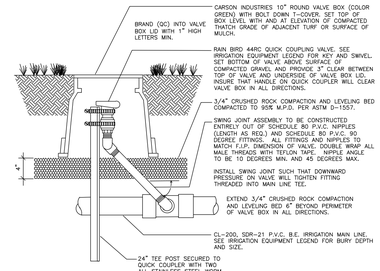
POP-UP SPRAY HEAD

2



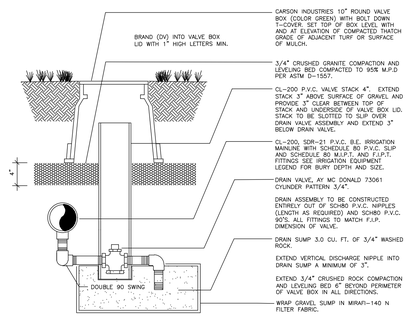
ELECTRIC CONTROL VALVE

3



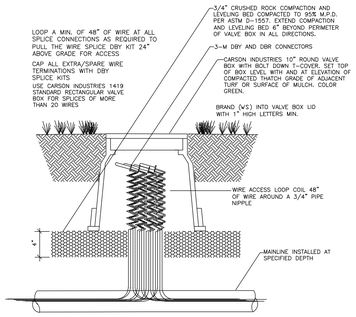
QUICK COUPLING VALVE

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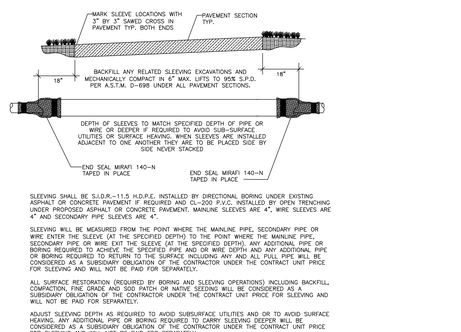
MANUAL DRAIN VALVE

5



RAIN/FREEZE SENSOR

6



SLEEVING SHALL BE S.I.D.R.-11.5 H.D.P.E. INSTALLED BY DIRECTIONAL BORING UNDER EXISTING ASPHALT OR CONCRETE PAVEMENT. IF REQUIRED AND CL-200 P.V.C. INSTALLED BY OPEN TRENCHING UNDER PROPOSED ASPHALT OR CONCRETE PAVEMENT. MAINLINE SLEEVES ARE 4", WIRE SLEEVES ARE 4" AND SECONDARY PIPE SLEEVES ARE 4".

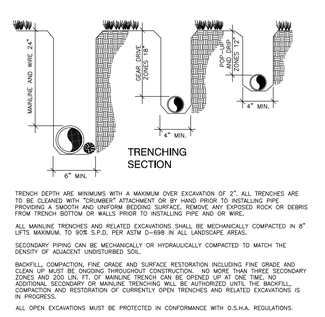
SLEEVING WILL BE MEASURED FROM THE POINT WHERE THE MAINLINE PIPE, SECONDARY PIPE OR WIRE ENTER THE SLEEVE (AT THE SPECIFIED DEPTH) TO THE POINT WHERE THE MAINLINE PIPE, SECONDARY PIPE OR WIRE EXIT THE SLEEVE (AT THE SPECIFIED DEPTH). ANY ADDITIONAL PIPE OR BORING REQUIRED TO ACHIEVE THE SPECIFIED PIPE AND OR WIRE DEPTH AND ANY ADDITIONAL PIPE OR BORING REQUIRED TO RETURN TO THE SURFACE INCLUDING ANY AND ALL PULL PIPE WILL BE CONSIDERED AS A SUBSIDIARY OBLIGATION OF THE CONTRACTOR UNDER THE CONTRACT UNIT PRICE FOR SLEEVING AND WILL NOT BE PAID FOR SEPARATELY.

ALL SURFACE RESTORATION (REQUIRED BY BORING AND SLEEVING OPERATIONS) INCLUDING BACKFILL, COMPACTION, FINE GRADE AND SOD PATCH OR NATIVE SEEDING WILL BE CONSIDERED AS A SUBSIDIARY OBLIGATION OF THE CONTRACTOR UNDER THE CONTRACT UNIT PRICE FOR SLEEVING AND WILL NOT BE PAID FOR SEPARATELY.

ADJUST SLEEVING DEPTH AS REQUIRED TO AVOID SUBSURFACE UTILITIES AND OR TO AVOID SURFACE HEAVING. ANY ADDITIONAL PIPE OR BORING REQUIRED TO CARRY SLEEVING DEEPER WILL BE CONSIDERED AS A SUBSIDIARY OBLIGATION OF THE CONTRACTOR UNDER THE CONTRACT UNIT PRICE FOR SLEEVING AND WILL NOT BE PAID FOR SEPARATELY.

IRRIGATION SLEEVING

7



TRENCHING SECTION

TRENCH DEPTH ARE MINIMUMS WITH A MAXIMUM OVER EXCAVATION OF 2". ALL TRENCHES ARE TO BE CLEANED WITH "CRUMBER" ATTACHMENT OR BY HAND PRIOR TO INSTALLING PIPE PROVIDING A SMOOTH AND UNIFORM BEDDING SURFACE. REMOVE ANY EXPOSED ROCK OR DEBRIS FROM TRENCH BOTTOM OR WALLS PRIOR TO INSTALLING PIPE AND OR WIRE.

ALL MAINLINE TRENCHES AND RELATED EXCAVATIONS SHALL BE MECHANICALLY COMPACTED IN 8" LIFTS MAXIMUM. TO 90% S.P.D. PER ASTM D-698 IN ALL LANDSCAPE AREAS.

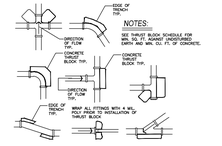
SECONDARY PIPING CAN BE MECHANICALLY OR HYDRAULICALLY COMPACTED TO MATCH THE DENSITY OF ADJACENT UNDISTURBED SOIL.

BACKFILL, COMPACTION, FINE GRADE AND SURFACE RESTORATION INCLUDING FINE GRADE AND CLEAN UP MUST BE ONGOING THROUGHOUT CONSTRUCTION. NO MORE THAN THREE SECONDARY ZONES AND 200 LIN. FT. OF MAINLINE TRENCH CAN BE OPENED UP AT ONE TIME. NO ADDITIONAL SECONDARY OR MAINLINE TRENCHING WILL BE AUTHORIZED UNTIL THE BACKFILL, COMPACTION, AND RESTORATION OF CURRENTLY OPEN TRENCHES AND RELATED EXCAVATIONS IS IN PROGRESS.

ALL OPEN EXCAVATIONS MUST BE PROTECTED IN CONFORMANCE WITH O.S.H.A. REGULATIONS.

TRENCHES

8



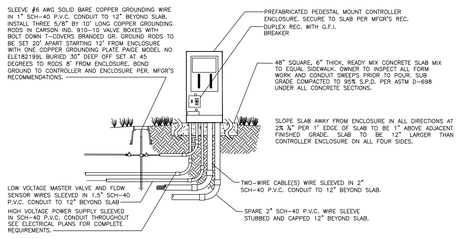
THRUST BLOCK CONFIGURATION DETAIL

MAIN LINE THRUST BLOCK SURFACE AREA AGAINST UNDISTURBED EARTH IN SQ. FT., AND TOTAL CONCRETE VOLUME IN CU. FT. Assumes an estimated soil bearing strength of 1,000 lbs/sq. ft.					
FITTING SIZE	90 DEGREE	45 DEGREE	22.5 DEGREE	11.25 DEGREE	TEE OR CAP
2.5" & SMALLER	2 SQ. FT. 4 CU. FT.	1 SQ. FT. 2 CU. FT.	1 SQ. FT. 2 CU. FT.	1 SQ. FT. 2 CU. FT.	2 SQ. FT. 4 CU. FT.
3" & LARGER	4 SQ. FT. 8 CU. FT.	2 SQ. FT. 4 CU. FT.	2 SQ. FT. 4 CU. FT.	2 SQ. FT. 4 CU. FT.	4 SQ. FT. 8 CU. FT.
INSTALLATION NOTES: CONCRETE TO BE 3,500 P.S.I. MIN. INSTALL BOND BREAKER AROUND PIPE AND FITTING PRIOR TO POUR, DO NOT POUR CONCRETE ON TOP OF PIPE.					

THRUST BLOCKING

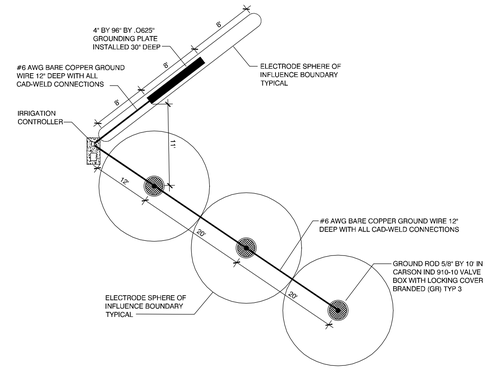
9

SEE ELECTRICAL ENGINEERING DRAWINGS FOR NEW CONTROLLER ELECTRICAL SERVICE.



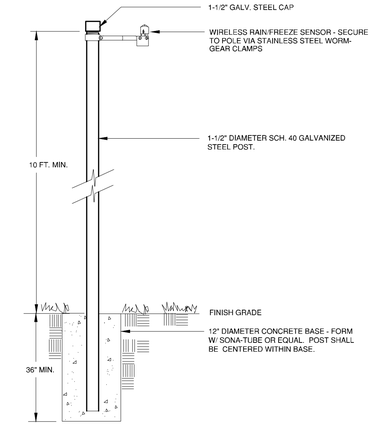
ELECTRIC CONTROLLER

10



RAIN/FREEZE SENSOR

11



RAIN/FREEZE SENSOR

12

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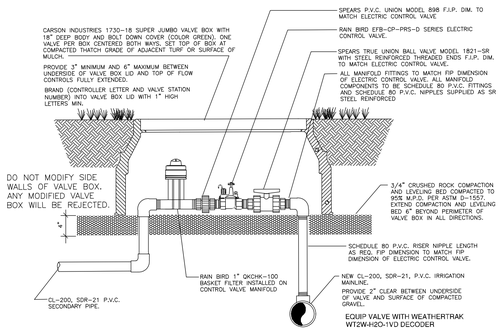
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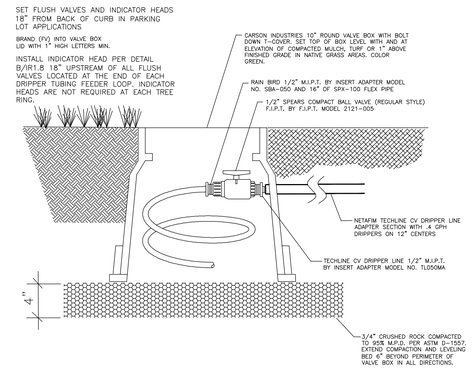
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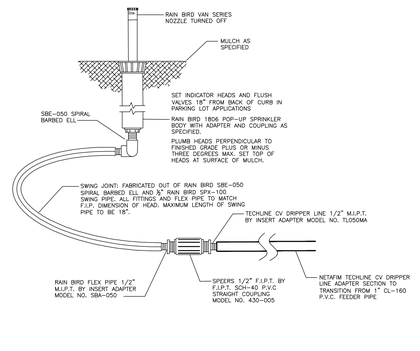
THE SQUARES ABOVE ARE COLOR WITH CYAN, MAGENTA, YELLOW & BLACK. IF PRINTER IS SETUP CORRECTLY



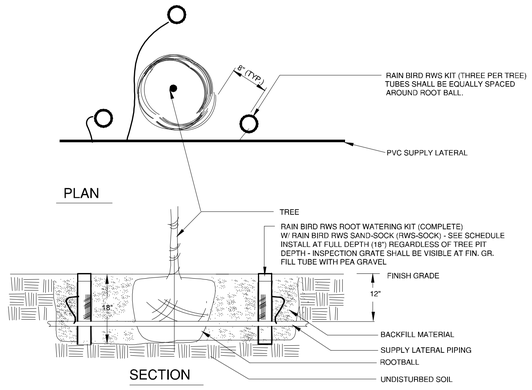
DRIP VALVE ASSEMBLY — 13



IN-LINE DRIP FLUSH-OUT — 14



IN-LINE DRIP OPERATIONAL INDICATOR HEAD — 15



NOTE:
ALL PERFORATED PIPE SHALL BE INSTALLED PLUMB
INSTALL ALL CAPS AND BUBBLERS WITH SOFT ADHESIVE TAPE TO PERFORATED PIPE.
INSTALL BUBBLERS ON OPPOSING SIDES OF ROOT BALL

ROOT WATERING SYSTEM — 16

DETAIL APPLIES TO ALL TREES

DRIPPERLINE SHALL BE WATER TESTED IN PRESENCE OF TOWN OF PARKER REP. PRIOR TO PLACEMENT OF WEED FABRIC AND MULCH

INSTALL TUBING WITH EMITTERS IN TRIANGULATED PATTERN BETWEEN ROWS OF TUBING

IN-LINE DRIP SAMPLE LAY-OUT — 17

NOTES:
1. SEE PLANS & LEGEND FOR ALL DIMENSIONS AND DRIPPERLINE LATERAL SPACING
2. RATIO OF Techline LATERALS TO START CONNECTIONS IS SHOWN AT 2:1, BUT MAY VARY PER HYDRAULIC DEMAND ON START CONNECTION.

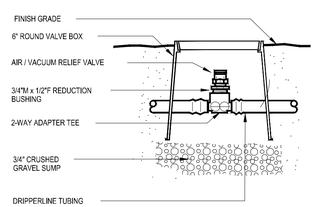
NO SCALE

DRIPPERLINE PIPE LAY-OUT ISOMETRIC VIEW — 18

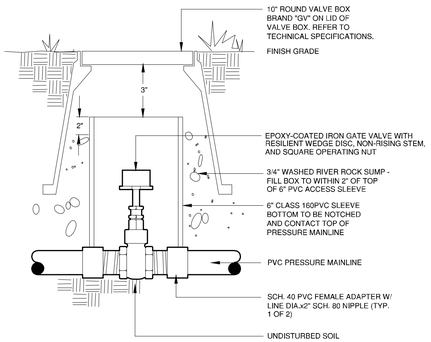
IN-LINE DRIP CONNECTION — 19

DRIPPERLINE SHALL BE INSTALLED UNDER WEED BARRIER FABRIC
DRIPPERLINE LAY-OUT SHALL BE REVIEWED AND APPROVED BY TOWN STAFF PRIOR TO PLACEMENT OF WEED BARRIER FABRIC

SURFACE IN-LINE DRIP TUBING — 20

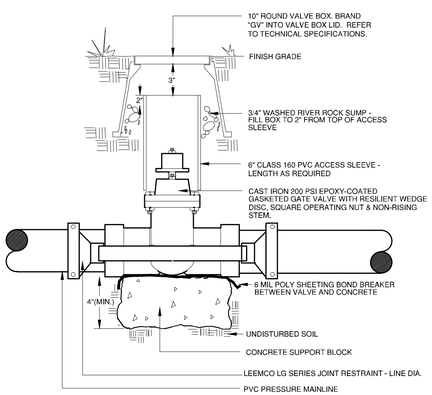


AIR/VACUUM RELEASE VALVE — 21



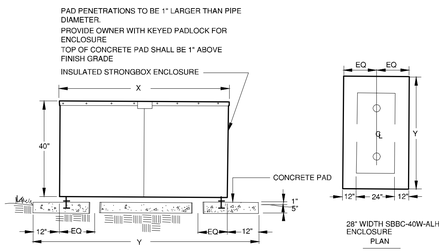
APPLY NON-HARDENING THREAD SEALANT CONTAINING NO PETROLEUM DISTILLATES TO ALL MALE-THREADED FITTINGS (SPEARS BLUE 75 OR EQUAL)

GATE VALVE - 2" AND 2-1/2" DIA. — 22



GATE VALVE - 3" DIA. and LARGER — 23

BFP MODEL/SIZE	ENCLOSURE MODEL	X	Y
FEBCO 860 LARGE - 4"	SBBC-90ALIHP	90"	114"



BACKFLOW ENCLOSURE — 24

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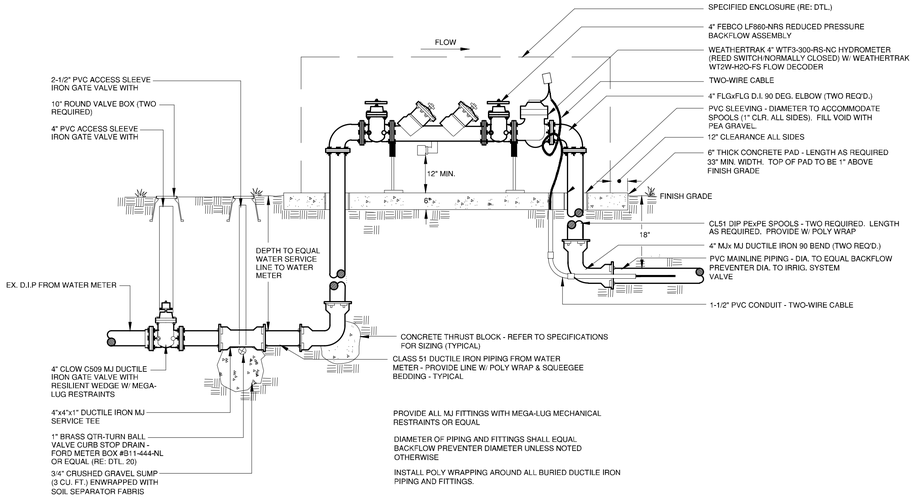
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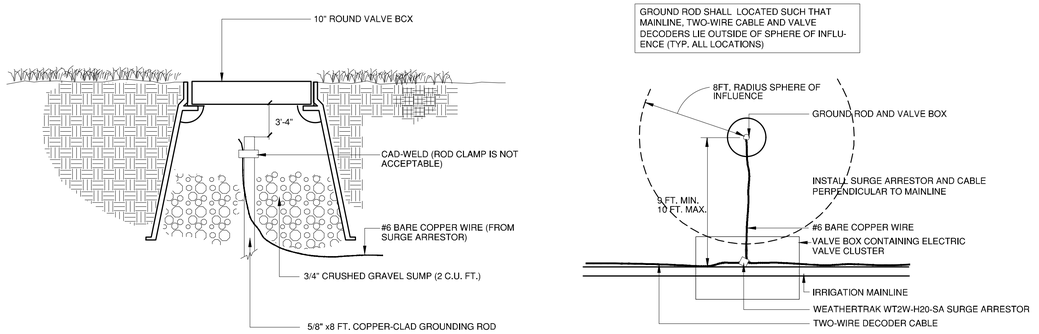
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THE SQUARES ABOVE ARE COLOR, WITH BLACK BEING LITHOL / PRINTER CORRECTLY



BACKFLOW PREVENTER

25



GROUND ROD SHALL BE INSTALLEDAT LOCATION PERPENDICULAR TO MAINLINE. GROUND SHALL BE LOCATED 9 FT. FROM MAINLINE TRENCH

GROUND ROD SHALL LOCATED SUCH THAT MAINLINE, TWO-WIRE CABLE AND VALVE DECODERS LIE OUTSIDE OF SPHERE OF INFLUENCE (TYP. ALL LOCATIONS)

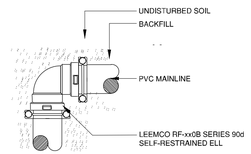
PLAN VIEW

TWO-WIRE SURGE ARRESTOR

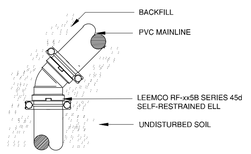
26

MECHANICAL MAINLINE PIPE RESTRAINTS

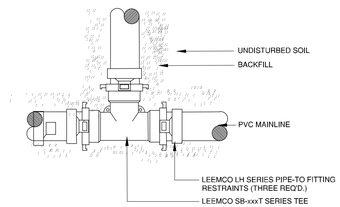
INDICATES MAINLINE PIPING TO BE SECURED VIA MECHANICAL PIPE RESTRAINTS. ALL PIPE-TO-PIPE JOINTS, FITTINGS (I.E. CHANGES IN DIRECTION), SERVICES TEES AND/OR GATE VALVES THAT FALL WITHIN INDICATED DISTANCE SHALL BE RESTRAINED. REFER TO DETAILS. RESTRAINTS SHALL BE MANUFACTURED BY LEEMCO. ALL DISTANCES SHOWN BELOW ARE TYPICAL.



90 DEGREE ELBOW

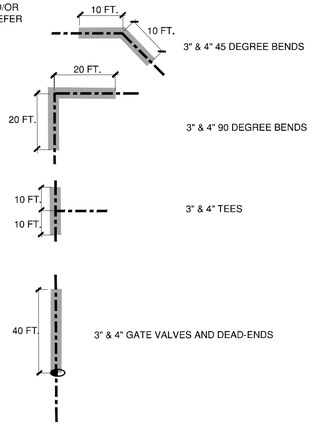


45 DEGREE ELBOW



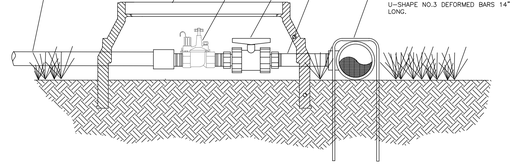
STRAIGHT TEE

INSTALL FITTINGS PER LEEMCO DIRECTIONS INCLUDING USE OF TORQUE WRENCH FOR RESTRAINTS



MAINLINE FITTINGS - 3" DA. and LARGER

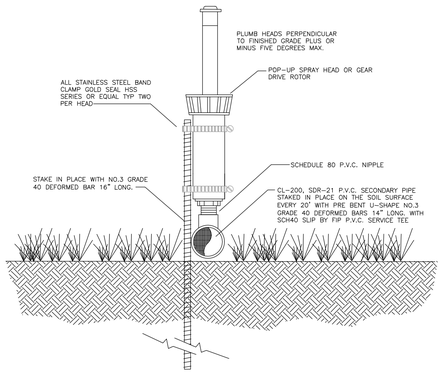
27



ELECTRIC CONTROL VALVE

TEMPORARY IRRIGATION

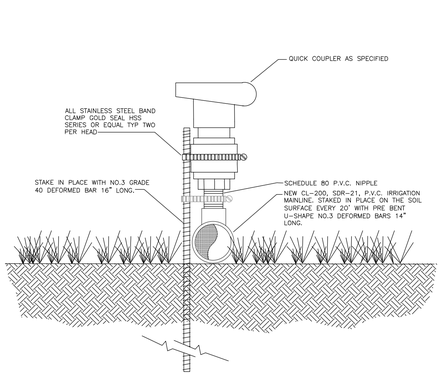
28



ON-GRADE SPRAY HEAD OR ROTOR

TEMPORARY IRRIGATION

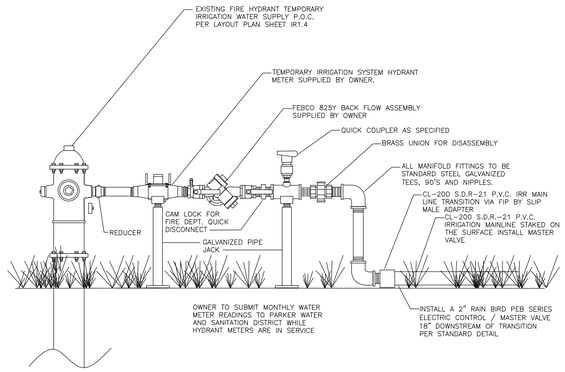
29



QUICK COUPLING VALVE

TEMPORARY IRRIGATION

30



FIRE HYDRANT CONNECTION

TEMPORARY IRRIGATION

31

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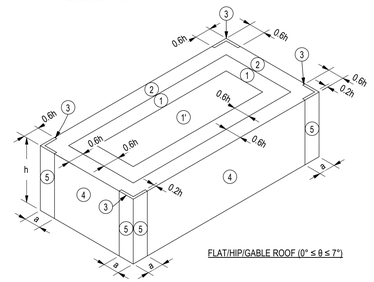
STRUCTURAL GENERAL NOTES

DESIGN LOADS:

1. DESIGN LOADS: 2021 INTERNATIONAL BUILDING CODE WITH TOWN OF PARKER AMENDMENTS, ASCE 7-16
2. RISK CATEGORY: II STANDARD
3. ROOFS:
   - A. ROOF DEAD LOAD 25 PSF
   - B. ROOF LIVE LOAD 20 PSF
   - C. GROUND SNOW LOAD, Pg 30 PSF
   - D. FLAT-ROOF SNOW LOAD, Pf 30 PSF
   - E. SNOW EXPOSURE FACTOR, Ce 1.0
   - F. SNOW IMPORTANCE FACTOR, Is 1.0
   - G. THERMAL FACTOR, Ct 1.2 OPEN-AIR STRUCTURES
   - H. THERMAL FACTOR, CT 1.0 HEATED STRUCTURES
4. FLOOR LIVE LOADS:

OCCUPANCY OR USE	UNIFORMLY DISTRIBUTED (PSF)	CONCENTRATED LOAD (LBS)	LIVE LOAD REDUCTION
PUBLIC SPACES	100	2,000	NO
STORAGE AREAS	125	N/A	NO
RETAIL STORES FIRST FLOOR	100	1,000	YES

5. WIND:
   - A. BASIC DESIGN WIND SPEED, V_{ULT}, (3-SECOND GUST) 115 MPH
   - B. INTERNAL PRESSURE COEFFICIENT 0.18 (ENCLOSED)
   - C. WIND EXPOSURE C
   - D. GROUND ELEVATION FACTOR 0.81
   - E. COMPONENTS AND CLADDING ULTIMATE DESIGN WIND PRESSURES
     1. PRESSURES MAY BE REDUCED FOR EFFECTIVE WIND AREAS LARGER THAN 10 SQUARE FEET, BUT NOT BELOW 16 PSF.
     2. ALLOWABLE WIND PRESSURE (ASD) MAY BE DETERMINED BY MULTIPLYING THE ULTIMATE PRESSURE BY 0.6.



FLAT/HIP/GABLE ROOF (0° ≤ θ ≤ 7°)

COMPONENT AND CLADDING ULTIMATE WIND PRESSURE - FLAT ROOF

ROOF (EFFECTIVE WIND AREA)	ROOF SURFACE PRESSURE (psf)	
	10 sf	100 sf
Negative Zone 1	-56.6	-42.0
Negative Zone 1'	-32.5	-29.9
Negative Zone 2	-74.7	-55.9
Negative Zone 3	-101.7	-64.2
Positive all Zones	16.0	16.0
Overhang Zone 1 & 1'	-51.2	-43.6
Overhang Zone 2	-69.2	-44.2
Overhang Zone 3	-96.3	-52.6

WALL (EFFECTIVE WIND AREA)	WALL SURFACE PRESSURE (psf)	
	10 sf	100 sf
Negative Zone 4	-33.8	-31.7
Negative Zone 5	-40.5	-36.3
Positive Zone 4 & 5	31.1	29.0

6. SEISMIC:
   - A. SPECTRAL RESPONSE ACCELERATION PARAMETERS
     1. SHORT PERIOD
        - a. S_S 0.202g
        - b. S_{DS} 0.216g
     2. ONE SECOND
        - a. S_1 0.056g
        - b. S_{D1} 0.090g
   - B. SOILS SITE CLASS D
   - C. SEISMIC IMPORTANCE FACTOR 1.0
   - D. SEISMIC DESIGN CATEGORY B
   - E. BASIC SEISMIC-FORCE-RESISTING SYSTEM(S)
     - INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
     - STEEL ORDINARY MOMENT FRAMES
   - F. DESIGN BASE SHEAR(S)
   - G. SEISMIC RESPONSE COEFFICIENT(S), C_S
   - H. RESPONSE MODIFICATION COEFFICIENT(S), R
   - I. ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE

FOUNDATION DESIGN:

1. REFER TO SOILS REPORT NO. 502725001 BY NINYO & MOORE, DATED 11-10-2023.
2. GEOTECHNICAL ENGINEER SHALL VERIFY SOIL CONDITIONS AND TYPES DURING EXCAVATION AND PRIOR TO PLACEMENT OF FORMWORK OR CONCRETE. OVER-EXCAVATE BENEATH SLAB-ON-GRADE TO A DEPTH OF AT LEAST 3 FEET BELOW THE PROPOSED SLAB AND REPLACE WITH MOISTURE-CONDITIONED, PROPERLY COMPACTED ENGINEERED FILL.
3. MINIMUM FROST DEPTH SHALL BE 3'-0 BELOW EXTERIOR GRADE.

FOOTINGS:

1. DESIGN OF FOOTINGS IS BASED ON
   - A. MAXIMUM ALLOWABLE BEARING PRESSURE 3000 PSF
2. BEAR ON 2 FEET OF OVER-EXCAVATED SOIL REPLACED WITH MOISTURE-CONDITIONED COMPACTED FILL. ON-SITE SOILS ARE ACCEPTABLE AS ENGINEERED FILL IF THEY ARE FREE OF ORGANIC MATERIALS. OVER-EXCAVATIONS BENEATH BOTTOM OF FOUNDATIONS SHALL EXTEND LATERALLY BEYOND THE EDGES OF THE FOUNDATIONS AT LEAST 8 INCHES PER FOOT OVER THE EXCAVATION DEPTH BELOW THE FOUNDATION BASE ELEVATION. EXTERIOR FOOTINGS SHALL BEAR BELOW FROST DEPTH.

EARTH RETAINING STRUCTURES:

1. EARTH EQUIVALENT FLUID LATERAL PRESSURE:
   - A. WALLS RESTRAINED AT TOP (AT REST) 64 PCF
   - B. CANTILEVERED WALLS (ACTIVE) 45 PCF
   - C. PASSIVE RESISTING 271 PCF
2. COEFFICIENT OF SLIDING FRICTION 0.35

REINFORCED CONCRETE:

1. DESIGN IS BASED ON ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE."
2. CONCRETE WORK SHALL CONFORM TO ACI 301 "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE."
3. STRUCTURAL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

INTENDED USE	EXPOSURE CLASS	f'c, PSI 28 DAYS	MAX W/CM RATIO	MAXIMUM AGGREGATE	SLUMP, INCHES (+/- 1")	AIR CONTENT PERCENT (+/- 1.5%)	CEMENT TYPE	ADMIXTURES / COMMENTS
FOOTINGS	F0-S0-W0-C1	3000	0.52	3/4" STONE	5	N/A	I/II	
STEM WALLS	F2-S0-W0-C1	4500	0.45	3/4" STONE	4	6%	I/II	
INTERIOR SLAB ON GRADE	F0-S0-W0-C0	4000	0.45	3/4" STONE	4	N/P	I/II	
EXTERIOR SLAB ON GRADE	F3-S0-W0-C2	5000	0.40	3/4" STONE	4	6%	I/II	25% MAX FLY ASH

4. CONCRETE MIX TABLE NOTES:
   - A. SLUMP VALUES INDICATED ARE SUGGESTED BASED ON USE AND TYPICAL PLACEMENT METHODS. CONTRACTOR MAY ADJUST SLUMP AS NECESSARY FOR FIELD CONDITIONS AND INSTALLATION METHOD USED PROVIDED REMAINING REQUIREMENTS ARE MET.
   - B. AIR CONTENT:
     - a. N/P: AIR ENTRAINING ADMIXTURES NOT PERMITTED, ENTRAPPED AIR ONLY
     - b. N/A: NOT APPLICABLE, NO STRUCTURAL AIR CONTENT REQUIREMENTS
   - C. SHRINKAGE STRAIN: MIXES NOTED AS "SHRINKAGE XX%" SHALL BE LIMITED TO A MAXIMUM DRYING SHRINKAGE (0.05% UNLESS NOTED OTHERWISE) AT 28 DAYS (500 MICROSTRAIN) AS MEASURED BY ASTM C157. SHRINKAGE TEST RESULTS TO BE INCLUDED WITH MIX DESIGN SUBMITTAL.
   - D. GENERAL CONTRACTOR TO COORDINATE CONCRETE MOISTURE LEVEL AND ANTICIPATED MOISTURE MITIGATION PROCEDURES WITH CONCRETE SUPPLIER/MIX DESIGNER AND OTHER AFFECTED SUBCONTRACTORS (INCLUDING BUT NOT LIMITED TO FLOORING) TO ADDRESS ALL POTENTIAL SCHEDULE AND INSTALLATION CONFLICTS.
5. DETAILING, FABRICATION, AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT."
6. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, EXCEPT BARS SHOWN TO BE FIELD-BENT SHALL BE ASTM A706, GRADE 60.
7. BARS TO BE WELDED SHALL CONFORM TO ASTM A706.
8. UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, LAP BARS PER THE CONCRETE LAP SPLICE SCHEDULE.
9. AT CORNERS AND INTERSECTIONS, MAKE HORIZONTAL BARS CONTINUOUS OR PROVIDE MATCHING CORNER BARS FOR EACH LAYER OF REINFORCEMENT.
10. TRIM OPENINGS IN WALLS AND SLABS WITH (2) #5 FOR EACH LAYER OF REINFORCEMENT, FULLY DEVELOPED BY EXTENSION OR HOOK.
11. IN CONTINUOUS MEMBERS, SPLICE TOP BARS AT MID-SPAN AND SPLICE BOTTOM BARS OVER SUPPORTS.
12. FORM INTERMITTENT SHEAR KEYS AT ALL CONSTRUCTION JOINTS AND AS SHOWN ON THE STRUCTURAL DRAWINGS.
13. EXCEPT AS NOTED ON THE DRAWINGS, CONCRETE PROTECTION FOR REINFORCEMENT IN CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
    - A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
    - B. EXPOSED TO EARTH OR WEATHER:
      1. #6 THROUGH #18 BARS 2"
      2. #5 BAR, W31 OR D31 WIRE, AND SMALLER 1-1/2"
    - C. NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
      1. SLABS, WALLS, JOISTS: #11 BARS AND SMALLER 3/4"
      2. BEAMS AND COLUMNS:
         - a. PRIMARY REINFORCEMENT 1-1/2"
         - b. STIRRUPS, TIES, SPIRALS 1-1/2"
14. ANCHOR BOLTS AND RODS FOR BEAM AND COLUMN-BEARING PLATES SHALL BE PLACED WITH SETTING TEMPLATES.

STRUCTURAL MASONRY:

1. GENERAL CONTRACTOR SHALL HOLD A MASONRY PRECONSTRUCTION MEETING AT THE PROJECT SITE WITH REPRESENTATION FROM THE GC, MASON, TESTING AGENCY AND STRUCTURAL ENGINEER.
2. GENERAL CONTRACTOR SHALL SUBMIT COORDINATED ELEVATION DRAWINGS FOR REVIEW OF ALL MASONRY WALLS SHOWING:
   - A. ALL CONTROL JOINTS, BOND BEAMS, BEAM AND JOIST POCKETS, AND OPENINGS INCLUDING MECHANICAL AND PLUMBING PENETRATIONS GREATER THAN 3" IN ANY DIMENSION.
   - B. TYPICAL WALL REINFORCING
   - C. ADDITIONAL WALL REINFORCING AT MASONRY LINTELS, JAMBS, OPENINGS, AND AS NOTED ON STRUCTURAL DRAWINGS.
3. DESIGN IS BASED ON ACI 530/ASCE 5/TMS 402, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
4. 28-DAY COMPRESSIVE STRENGTH OF MASONRY ASSEMBLY USED FOR DESIGN IS 2,000 PSI, BASED ON NET-BEDDED AREA.
5. MASONRY LINTELS SHALL USE STANDARD LINTEL UNITS. BOND BEAMS SHALL USE UNITS PRODUCED FROM STANDARD VERTICALLY VOIDED UNITS WITH PRE-CUT KNOCKOUT CROSS WALLS.
6. HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS (CMU) SHALL BE LIGHTWEIGHT, 105 PCF DENSITY, CONFORMING TO ASTM C90, WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,000 PSI BASED ON AVERAGE NET AREA.
7. MORTAR SHALL BE TYPE S CONFORMING TO ASTM C270.
8. MASONRY CEMENT SHALL NOT BE USED UNLESS PART OF A PRE-PACKAGED MORTAR OR GROUT MIX APPROVED BY THE STRUCTURAL ENGINEER.
9. ADMIXTURES SHALL NOT BE USED UNLESS APPROVED BY THE ARCHITECT AND/OR STRUCTURAL ENGINEER.
10. GROUT USED IN MASONRY WALLS AND BLOCK CELLS SHALL BE COARSE GROUT, AS DEFINED BY ARTICLE 2.2 OF TMS 602/ACI530.1/ASCE 6, WITH A MINIMUM CUBE STRENGTH = 2,000 PSI OR 3,000 PSI CONCRETE USING 3/8" DIAMETER AGGREGATE AND PLACED BY VIBRATING UNLESS AN APPROVED SELF-CONSOLIDATING MIX IS USED.
11. PLACEMENT OF MORTAR, GROUT, MASONRY UNITS AND WALL TIES SHALL COMPLY WITH TMS 602 / ACI 530.1 / ASCE 6.
12. PROVIDE MORTAR FOR FULL THICKNESS OF SHELL IN ALL HEAD AND BED JOINTS.
13. "LOW-LIFT" GROUTING SHALL NOT EXCEED 5 FEET IN HEIGHT UNLESS ACI 530.1 "HIGH-LIFT" GROUTING PROCEDURES ARE REVIEWED AND APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER.
14. VERTICALLY SPACE CONTINUOUS HORIZONTAL JOINT REINFORCING AT 16" MAXIMUM IN ALL CMU WALLS. JOINT REINFORCING SHALL BE WELDED TYPE WITH 9 GAGE SIDE RODS AND 9 GAGE LADDER CROSS RODS. IN EXTERIOR WALLS, JOINT REINFORCEMENT SHALL BE STAINLESS STEEL OR HOT-DIP GALVANIZED. ALL OTHER JOINT REINFORCEMENT SHALL BE MILL GALVANIZED, HOT-DIP GALVANIZED, OR STAINLESS STEEL.
15. REINFORCING BARS SHALL HAVE MATERIAL PROPERTIES AS SPECIFIED FOR REINFORCED CONCRETE. LAP BARS PER THE LAP SPLICE SCHEDULE (48 DIAMETERS MINIMUM) UNLESS OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS.
16. REINFORCEMENT SHALL BE SECURED AGAINST DISPLACEMENT PRIOR TO GROUTING BY WIRE BAR LOCATORS OR OTHER SUITABLE DEVICES AT INTERVALS NOT EXCEEDING 200 BAR DIAMETERS OR 10 FEET.
17. REINFORCE AND GROUT VERTICAL CELLS AT CORNERS, ENDS OF WALLS, JAMBS OF OPENINGS, EACH SIDE OF VERTICAL CONTROL JOINTS, AND AT SPACING SHOWN ON DRAWINGS.
18. WHERE NOTED ON THE DRAWINGS, PROVIDE CLEARANCE BETWEEN MASONRY AND STRUCTURAL ELEMENTS, OR WRAP STEEL WITH POLYETHYLENE FILM.
19. LOCATE VERTICAL CONTROL JOINTS IN ALL MASONRY WALLS AS SHOWN ON THE ARCHITECTURAL DRAWINGS, STRUCTURAL DRAWINGS, OR SPACED HORIZONTALLY AT 25'-0 MAXIMUM SPACING WHERE NOT SHOWN.

STRUCTURAL STEEL:

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" (AISC 360) AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (AISC 303) BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC).
2. STRUCTURAL STEEL WIDE FLANGE BEAMS AND WTS SHALL CONFORM TO ASTM A992, 50 KSI YIELD.
3. OTHER ROLLED SHAPES, INCLUDING PLATES, CHANNELS, AND ANGLES SHALL CONFORM TO ASTM A36, 36 KSI YIELD.
4. HOLLOW STRUCTURAL SECTION (HSS) RECTANGULAR SHAPES SHALL CONFORM TO ASTM A500, GRADE C, 50 KSI YIELD.
5. EXCEPT AS NOTED, FRAMED BEAM CONNECTIONS SHALL BE BEARING-TYPE WITH 3/4" DIAMETER, SNUG TIGHT, ASTM F3125 BOLTS, DETAILED IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND THE "STEEL CONSTRUCTION MANUAL" BY THE AISC. INSTALL BOLTS IN ACCORDANCE WITH AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS".
6. ALL BEAMS SHALL HAVE FULL DEPTH WEB STIFFENERS EACH SIDE OF WEBS ABOVE AND BELOW COLUMNS.
7. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE (36, 55 WITH WELDABILITY SUPPLEMENT S1, AND/OR 105) AS NOTED ON THE STRUCTURAL DRAWINGS.
8. WELDING SHALL BE DONE BY A CERTIFIED WELDER IN ACCORDANCE WITH THE AISC DOCUMENTS LISTED ABOVE, THE AMERICAN WELDING SOCIETY (AWS) D1.1; STRUCTURAL WELDING CODE, AND THE RECOMMENDATIONS FOR USE OF WELD E70 ELECTRODES. WHERE NOT SPECIFICALLY NOTED, MINIMUM WELD SHALL BE 3/16" FILLET BY LENGTH OF CONTACT EDGE.
9. GROUT BENEATH COLUMN BASE AND BEAM BEARING PLATES SHALL HAVE A MINIMUM 28-DAY, COMPRESSIVE STRENGTH OF 7,500 PSI AND SHALL BE NON-SHRINK, NON-METALLIC, AND TESTED IN ACCORDANCE WITH ASTM C1107.

CORROSION CONTROL:

1. ALL STEEL MEMBERS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED PER ASTM A123.
2. FASTENERS AND HARDWARE SHALL BE HOT DIPPED GALVANIZED PER ASTM A153 OR ASTM B695 CLASS 50 (A490 BOLTS SHALL NOT BE HOT DIPPED GALVANIZED). STAINLESS STEEL FASTENERS AND HARDWARE MAY ALSO BE USED.
3. ALL FIELD CUT OR DAMAGED SURFACES, FIELD WELDED AREAS AND AUTHORIZED NON-GALVANIZED MEMBERS AS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE REPAIRED WITH (2) COATS OF A 95% ZINC RICH PAINT PER ASTM A780 (ZRC PREFERRED).

WOOD SHEATHING:

1. PLYWOOD AND ORIENTED STRAND BOARD (OSB) FLOOR AND ROOF SHEATHING SHALL BE APA RATED WITH STAMP INCLUDING APA TRADEMARK AND PANEL SPAN RATING.
   - A. MINIMUM ROOF SHEATHING: 15/32" OSB OR CDX PLYWOOD, APA 32/16, NAILED.
2. NAIL WALL SHEATHING WITH MINIMUM 8D COMMON OR 10D BOX AT 6" AT PANEL EDGES, AND 12" AT INTERMEDIATE FRAMING EXCEPT AS NOTED. BLOCK AND NAIL ALL EDGES BETWEEN STUDS.
3. MINIMUM (3) 8D NAILS PER STUD. NAIL ALL PLATES USING EDGE NAIL SPACING INDICATED.
4. SHEATHE ALL EXTERIOR WALLS. SHEATHE INTERIOR WALLS AS DESIGNATED ON THE DRAWINGS.
5. SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO TOP PLATE. CUT IN "L" AND "T" SHAPES AROUND OPENINGS. LAP SHEATHING OVER SINGLE 2X PLATE MEMBER AT RIM JOIST. AT RIM JOIST PROVIDE A MINIMUM OF 3" BETWEEN SHEATHING EDGE AND TOP/BOTTOM EDGE OF RIM.
6. MINIMUM HEIGHT OF SHEATHING PANELS SHALL BE 16" TO ENSURE THAT PLATES ARE TIED TO STUDS.
7. ALL SHEATHING SHEETS SHALL HAVE 1/8" GAP AT ALL EDGES AND JOINTS.
8. PROVIDE (1) PANEL SHEATHING CLIP AT ALL UNSUPPORTED ROOF SHEATHING PANEL EDGES. WHERE SPANS ARE GREATER THAN 32" PROVIDE (2) CLIPS.

STRUCTURAL WOOD FRAMING:

1. IN-GRADE BASE VALUES HAVE BEEN USED FOR DESIGN.
2. DIMENSIONAL LUMBER FRAMING SHALL BE S4S HEM FIR NO. 2 AND BETTER UNO.
3. SOLID TIMBER BEAMS AND POSTS SHALL BE DOUGLAS FIR-LARCH NO. 1 AND BETTER UNO.
4. STUDS SHALL BE HEM FIR STUD GRADE AND BETTER UNO.
5. TOP AND BOTTOM PLATES SHALL BE DOUGLAS FIR-LARCH NO. 2 AND BETTER UNO.
6. ALL LUMBER SHALL BE 19% MAXIMUM MOISTURE CONTENT AT THE TIME OF INSTALLATION UNO.
7. ALL WOOD EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED DOUGLAS FIR-LARCH OR SOUTHERN YELLOW PINE. PRESERVATIVE-TREATED WOOD SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARDS U1 AND M4. TREATMENTS SHALL HAVE NO AMMONIA ADDED AND SHALL BE THE FOLLOWING USE CATEGORY:
   - A. UC2 AT INTERIOR
   - B. UC3B AT EXTERIOR WITH NO GROUND CONTACT
   - C. UC4B AT EXTERIOR WITH GROUND CONTACT
8. FASTENERS FOR USE WITH TREATED WOOD SHALL BE CORROSION RESISTANT IN ACCORDANCE WITH SECTION 2304.10.6 OF THE IBC.
9. ALL CONNECTORS USED WITH PRESSURE-TREATED MATERIAL SHALL BE STAINLESS STEEL ASTM 304 OR 316, OR HAVE A SIMPSON Z-MAX (G185) OR HDG COATING. STANDARD COATING (G90) IS ACCEPTABLE AT INTERIOR CONDITIONS WITH NON PRESSURE-TREATED LUMBER ONLY. CONNECTORS ARE TO BE IN ACCORDANCE WITH ASTM A653 OR ASTM 123.
10. ALL IRON AND STEEL PRODUCTS ATTACHED TO TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 OR SHALL BE TYPE 304 OR 316 STAINLESS STEEL.
11. STRUCTURAL MEMBERS SHALL NOT BE CUT FOR PIPES, ETC. UNLESS SPECIFICALLY NOTED OR DETAILED ON THE STRUCTURAL DRAWINGS.
12. ALL BOLTS SHALL BE RETIGHTENED PRIOR TO CLOSING IN OF WALLS, FLOORS, AND ROOFS.
13. ALL BOLTS BEARING ON WOOD SHALL HAVE STANDARD CUT WASHERS UNDER HEAD AND/OR NUT, UNO.
14. METAL FRAMING ANCHORS SHOWN OR REQUIRED, SHALL BE SIMPSON STRONG-TIE OR EQUAL CODE APPROVED CONNECTORS AND INSTALLED WITH ALL HOLES FILLED (ROUND AND TRIANGULAR) WITH THE MAXIMUM SIZE NAIL RECOMMENDED BY THE MANUFACTURER TO DEVELOP THE MAXIMUM RATED CAPACITY.
15. CONNECTOR BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME B18.2.1 AND ASTM SAE J429 GRADE 1.
16. NAILS AND SPIKES SHALL CONFORM TO ASTM F1667.
17. WOOD SCREWS SHALL CONFORM TO ANSI/ASME B18.6.1.
18. CONVENTIONAL LIGHT FRAMING SHALL COMPLY WITH IBC SECTION 2308.
19. COLUMNS / MULTIPLE STUDS IN BEARING WALLS SUPPORTING ALL BEAMS AND HEADERS SHALL OCCUR CONTINUOUSLY THROUGH EACH FLOOR LEVEL DOWN TO THE FOUNDATION OR ANOTHER SUPPORT BEAM. SOLID SQUASH BLOCKING EQUIVALENT IN AREA TO THE COLUMN/MULTIPLE STUDS ABOVE SHALL BE PROVIDED WITHIN THE JOIST SPACE BENEATH THE COLUMN/MULTIPLE STUDS.
20. ALL BEAMS AND TRUSSES SHALL BE BRACED AGAINST ROTATION AT POINTS OF BEARING.
21. 2X BLOCKING SHALL BE PLACED BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS, UNO.
22. CROSS-BRIDGING OR SOLID BLOCKING SHALL BE PROVIDED AT 8'-0" MAX. FOR ALL JOISTS AND RAFTERS MORE THAN 10" IN DEPTH, 2X3 OR APPROVED METAL TYPE BRIDGING MAY BE USED.
23. PROVIDE A MINIMUM OF (3) STUDS AT EACH CORNER, UNO.
24. ALL JOISTS AND BEAMS (EXCLUDING I-JOISTS) SHALL BE SEAT-CUT FOR FULL UNIFORM BEARING AT SUPPORTS, SEATS, CAPS, ETC.
25. VENTING IS REQUIRED IN ALL ENCLOSED ROOF AND CRAWL SPACE FRAMING CAVITIES, SEE ARCHITECTURAL DRAWINGS.
26. EXCEPT AS NOTED OTHERWISE, MINIMUM NAILING SHALL BE PROVIDED AS SPECIFIED IN TABLE 2304.10.2 "FASTENING SCHEDULE" OF THE IBC.
27. ALL MULTIPLE MEMBER BEAMS SHALL BE NAILED TOGETHER WITH MAX NUMBER OF 10D NAILS VERTICALLY @ 3" AND HORIZONTALLY @ 12" PER PLY.
28. ALL ROOF RAFTERS, JOISTS, TRUSSES, AND BEAMS SHALL BE ANCHORED TO SUPPORTS WITH H2.5A METAL FRAMING ANCHORS UNO. PROVIDE (2) WITHIN 4'-0" OF ALL CORNERS.

SHOP DRAWINGS:

1. THE STRUCTURAL DRAWINGS ARE COPYRIGHTED AND SHALL NOT BE COPIED FOR USE AS ERECTION PLANS OR SHOP DETAILS. USE OF JVA'S ELECTRONIC FILES AS THE BASIS FOR SHOP DRAWINGS REQUIRES PRIOR APPROVAL BY JVA. A SIGNED RELEASE OF LIABILITY BY THE GENERAL CONTRACTOR AND/OR HIS SUBCONTRACTORS, AND DELETION OF JVA'S NAME AND LOGO FROM ALL SHEETS SO USED.
2. THE GENERAL CONTRACTOR SHALL SUBMIT IN WRITING ANY REQUESTS TO MODIFY THE STRUCTURAL DRAWINGS OR PROJECT SPECIFICATIONS.
3. ALL SHOP AND ERECTION DRAWINGS SHALL BE CHECKED AND STAMPED (AFTER HAVING BEEN CHECKED) BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION FOR STRUCTURAL ENGINEER'S REVIEW; SHOP DRAWING SUBMITTALS NOT CHECKED BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION TO THE STRUCTURAL ENGINEER WILL BE RETURNED WITHOUT REVIEW.
4. FURNISH ELECTRONIC VERSION (PDF) OF SHOP AND ERECTION DRAWINGS TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION FOR:
   - A. CONCRETE MIX DESIGNS
   - B. CONCRETE REINFORCING STEEL
   - C. CONCRETE SLAB ON GRADE CONTROL JOINT LAYOUT
   - D. MASONRY WALL COORDINATION DRAWING ELEVATIONS
   - E. MASONRY REINFORCING STEEL
   - F. STRUCTURAL STEEL
   - G. STEEL FORM, FLOOR, AND ROOF DECK
5. SUBMIT IN A TIMELY MANNER TO PERMIT 10 WORKING DAYS FOR REVIEW BY THE STRUCTURAL ENGINEER.
6. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "REQUEST FOR CHANGE IN WRITING" UNLESS SPECIFIC SUGGESTED CHANGES ARE CLEARLY MARKED. IN ANY EVENT, CHANGES MADE BY MEANS OF THE SHOP DRAWING SUBMITTAL PROCESS BECOME THE RESPONSIBILITY OF THE ONE INITIATING THE CHANGE.

SPECIAL INSPECTIONS - 2021:

1. THE FOLLOWING SPECIAL INSPECTIONS AND TESTING SHALL BE PERFORMED BY A QUALIFIED SPECIAL INSPECTOR, RETAINED BY THE OWNER, IN ACCORDANCE WITH THE FOLLOWING SECTIONS OF IBC CHAPTER 17:
   - A. SECTION 1704 SPECIAL INSPECTIONS, CONTRACTOR RESPONSIBILITY, AND STRUCTURAL OBSERVATIONS AND THE FOLLOWING SUB-SECTIONS:
     1. 1704.2 SPECIAL INSPECTIONS AND TESTS
     2. 1704.3 STATEMENT OF SPECIAL INSPECTIONS
   - B. SECTION 1705 REQUIRED VERIFICATION AND INSPECTION AND THE FOLLOWING SUB-SECTIONS:
     1. 1705.2 STEEL CONSTRUCTION
     2. 1705.3 CONCRETE CONSTRUCTION
     3. 1705.4 MASONRY CONSTRUCTION
     4. 1705.6 SOILS
2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THE APPROVED INSPECTOR MUST BE INDEPENDENT FROM THE CONTRACTOR RESPONSIBLE FOR THE WORK BEING INSPECTED.
3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR SHALL BE TO INSPECT AND/OR TEST THE WORK OUTLINED ABOVE AND WITHIN THE STATEMENT OF SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF THE IBC FOR CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
4. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION.
5. PER SECTION 1704.2.4 THE SPECIAL INSPECTOR SHALL FURNISH REGULAR REPORTS TO THE BUILDING OFFICIAL AND THE STRUCTURAL ENGINEER. PROGRESS REPORTS FOR CONTINUOUS INSPECTION SHALL BE FURNISHED WEEKLY. INDIVIDUAL REPORTS OF PERIODIC INSPECTIONS SHALL BE FURNISHED WITHIN ONE WEEK OF INSPECTION DATES. THE REPORTS SHALL NOTE UNCORRECTED DEFICIENCIES, CORRECTION OF PREVIOUSLY REPORTED DEFICIENCIES, AND CHANGES TO THE APPROVED CONSTRUCTION DOCUMENTS AUTHORIZED BY THE STRUCTURAL ENGINEER OF RECORD.
6. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT WITHIN 10 DAYS OF THE FINAL SPECIAL INSPECTION STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC. WORK NOT IN COMPLIANCE SHALL BE NOTED IN THE REPORT.
7. THE CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON A MAIN WIND- OR SEISMIC-FORCE-RESISTING SYSTEM PER SECTION 1704.4.
8. EXCEPT AS NOTED, THE SPECIAL INSPECTIONS OUTLINED ABOVE ARE IN ADDITION TO, AND BEYOND THE SCOPE OF, PERIODIC STRUCTURAL OBSERVATIONS AS DEFINED IN SECTION 1704.6. STRUCTURAL OBSERVATIONS ARE INCLUDED IN THE STRUCTURAL ENGINEERING DESIGN AND CONSTRUCTION ADMINISTRATION SERVICES PROVIDED BY THE STRUCTURAL ENGINEER.

STRUCTURAL DRAWING LIST

SHEET NO	SHEET TITLE
S-001	GENERAL NOTES
S-002	ABBREVIATIONS & SMBOLS KEY
S-003	IBC 2021 STATEMENT OF SPECIAL INSPECTION
S-111A	COMMUNITY HUB FOUNDATION & ROOF PLANS
S-111B	CONCESSIONS & RESTROOM FOUNDATION & ROOF PLANS
S-411A	COMMUNITY HUB COLUMN PLANS & SECTIONS
S-411B	CONCESSIONS & RESTROOMS COLUMN PLANS & SECTION
S-500	TYPICAL CONCRETE DETAILS
S-501	TYPICAL CMU DETAILS
S-502	TYPICAL CMU DETAILS
S-503	TYPICAL FOUNDATION DETAILS
S-504	TYPICAL STEEL DETAILS
S-510	FOUNDATION DETAILS
S-511	ROOF DETAILS

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LANDSCAPE ARCHITECT / ARCHITECT
1800 Wazee Street, Suite 450
Denver, CO 80202
p.303.607.0977

CIVIL ENGINEER / STRUCTURAL ENGINEER
JVA Incorporated
1675 Larimer Street, #520
Denver, CO 80202
p. 303.444.1951

ELECTRICAL ENGINEER
Ackerman Engineering, Inc.
3000 Youngfield Street, #264
Wheat Ridge, CO 80215
p. 303.278.7297

IRRIGATION
Avocet Irrigation
11757 W. Ken Caryl Ave., Suite F-509
Littleton, CO 80127
p. 303.986.2175

MECHANICAL ENGINEER
ENVISION Mechanical Engineers, Inc.
9777 Pyramid Court, #260
Englewood, CO 80112
p. 303.688.0223

Town of Parker
SALISBURY PARK NORTH - PHASE 1
11700 MOTSENBOCKER RD
PARKER, CO 80134

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ARCHITECTURE

LANDSCAPE ARCHITECTURE

PLANNING

INTERIOR DESIGN

Δ	DATE	DESCRIPTION

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Checked By: BAR

Key Map

Drawing

GENERAL NOTES

S-001

SITE PLAN SUBMITTAL
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THE LINE SHOWN ABOVE IS EXACTLY 1" WHEN PLOTTED ON 24 X 36 SHEETS ORIGINAL PAGE SIZE

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THE LINE SHOWN ABOVE IS 1" LONG WHEN PLOTTED ON ANSI D SHEETS ORIGINAL PAGE SIZE

THE SQUARES ABOVE ARE COLOR WITH BLACK AND WHITE LEFTOVER, IF PRINTED CORRECTLY

TYPICAL 6" AND 8" NON-STRUCTURAL BLOCK PARTITIONS:
BLOCK PARTITIONS SHALL HAVE #4 VERTICALS CENTERED IN WALL IN GROUTED CELLS AT CORNERS, JAMBS, WALL INTERSECTIONS, AND @ 8'-0 MAXIMUM. DOWEL TO SLAB WITH 1/2'Ø x 1'-6 ALL THREAD BARS INTO A 1/2'Ø DROP-IN EXPANSION ANCHOR WITH A 2" EMBEDMENT INTO THE SLAB. LOCATE DOWELS TO MATCH VERTICAL BAR SPACING. PROVIDE 2'-0 DEEP SOLID GROUTED MASONRY LINTEL AT TOP OF ALL WALLS WITH MATCHING CORNER BARS. WALL SHALL HAVE HORIZONTAL JOINT REINFORCING @ 16". BRACE TOP OF PARTITIONS PER SECTION X/XXX. SEE ARCHITECTURAL DRAWINGS FOR EXTENT OF PARTITIONS AND CONTROL JOINTS

MASONRY, STEEL, AND MECHANICAL SUBCONTRACTORS' NOTE:
STRUCTURAL DRAWINGS DO NOT INDICATE ALL WALL, FLOOR, OR ROOF PENETRATIONS FOR MECH DUCTS, DRAINS, VENTS, ETC., DRAWINGS INDICATE TYPICAL AND SPECIAL CONDITIONS FOR FRAMING AT THE PENETRATIONS, SEE X/XXX; GENERAL CONTRACTOR AND SUBCONTRACTORS' SHALL BE RESPONSIBLE FOR DETERMINING AND/OR MODIFYING OPENING LOCATIONS, ELEVATIONS AND DIMENSIONS FOR MECH UNLESS NOTED OTHERWISE. COORDINATION TO BE COMPLETED PRIOR TO FABRICATION OF STRUCTURAL STEEL AND ROOF JOISTS

FIELD VERIFICATION:
- ALL DIMENSIONS AND CONDITIONS SHALL BE FIELD VERIFIED BY CONTRACTOR
- IF DIMENSIONS AND CONDITIONS DIFFER THAN THOSE SHOWN ON DRAWINGS, NOTIFY ARCHITECT AND ENGINEER
- NOTIFY ARCHITECT AND ENGINEER ONCE FINISHES ARE REMOVED & FOUNDATION IS EXCAVATED TO ALLOW OBSERVATION

ABBREVIATIONS KEY

@	ON CENTER SPACING	DWG	DRAWING	LGS	LIGHT GAGE STEEL	
(E)	EXISTING	DWL	DOWEL	LL	LIVE LOAD	REINF	REINFORCE, -ED, -ING
(N)	NEW	EA	EACH	LLH	LONG LEG HORIZONTAL	REQ	REQUIRED
(R)	REMOVE	ECC	ECCENTRIC	LLV	LONG LEG VERTICAL	REQMT	REQUIREMENT
AB	ANCHOR ROD (BOLT)	E-E	END TO END	LOC	LOCATION	RET	RETAINING
ADDL	ADDITIONAL	EF	EACH FACE	LP	LOW POINT	RM	ROOM
ADJ	ADJUSTABLE	EJ	EXPANSION JOINT	LSL	LAMINATED STRAND LUMBER (GENERIC TERM)	RMO	ROUGH MASONRY OPENING
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	EL	ELEVATION	LT	LIGHT	RO	ROUGH OPENING
AFF	ABOVE FINISHED FLOOR	ELEC	ELECTRIC, ELECTRICAL	LVL	LAMINATED VENEER LUMBER (GENERIC TERM)	SC	SLIP-CRITICAL
ALT	ALTERNATE	EMBED	EMBEDMENT	MACH	MACHINE	SCH	SCHEDULE
AMT	AMOUNT	ENGR EOR	ENGINEER ENGINEER OF RECORD	MASY	MASONRY	SDST	SELF-DRILLING/ SELF-TAPPING
ANCH	ANCHOR, ANCHORAGE	EQ	EQUAL	MATL	MATERIAL	SECT	SECTION
APPROX	APPROXIMATE	EQUIP	EQUIPMENT	MAX	MAXIMUM	SF	SQUARE FEET, SUB-FLOOR
ARCH	ARCHITECT, -URAL	EQUIV	EQUIVALENT	MB	MACHINE BOLT	SHT	SHEET
ATR	ALL THREAD ROD	ES	EACH SIDE	MECH	MECHANICAL	SHTG	SHEATHING
AVG	AVERAGE	EST	ESTIMATE	MEZZ	MEZZANINE	SIM	SIMILAR
BC	BOTTOM OF CONCRETE	E-W	EAST TO WEST	MFR	MANUFACTURE, -ER, -ED	SLH	SHORT LEG HORIZONTAL
BL	BRICK LEDGE	EXC	EXCAVATE	MIN	MINIMUM	SLV	SHORT LEG VERTICAL
BLK	BLOCK	EXP	EXPANSION	ML	MICROLLAM (TRUS-JOIST BRAND LVL), MASONRY LINTEL	SOG	SLAB ON GRADE
BLKG	BLOCKING	EXT	EXTERIOR	MO	MASONRY OPENING	SP	SPACES, SPACED
BM	BEAM	FD	FLOOR DRAIN	MTL	METAL	SPEC	SPECIFICATIONS
BOT	BOTTOM	FDN	FOUNDATION	NF	NEAR FACE	SQ	SQUARE
BRG	BEARING	FF	FINISHED FLOOR, FAR FACE	NIC	NOT IN CONTRACT	SSR	SHEAR STUD RAIL
BW	BOTTOM OF WALL	F-F	FACE TO FACE	NS	NEAR SIDE	ST	SNUG-TIGHT
CB	COUNTERBORE	FIG	FIGURE	N-S	NORTH TO SOUTH	STD	STANDARD
CF	CUBIC FOOT	FL	FLUSH	NTS	NOT TO SCALE	STIFF	STIFFENER
CFS	COLD-FORMED STEEL	FLG	FLANGE	OCJ	OSHA COLUMN JOIST	STL	STEEL
CG	CENTER OF GRAVITY	FLR	FLOOR	OD	OUTSIDE DIAMETER	STRUCT	STRUCTURE, -AL
CIP	CAST-IN-PLACE	FO	FACE OF	OF	OUTSIDE FACE	SUPT	SUPPORT
CJ	CONSTRUCTION JOINT, CONTROL JOINT	FP	FULL PENETRATION	OH	OPPOSITE HAND	SY	SQUARE YARD
CJP	COMPLETE JOINT PENETRATION	FS	FOOTING STEP, FAR SIDE	OPNG	OPENING	SYM	SYMMETRICAL
CL	CENTER LINE	FTG	FOOTING	OPP	OPPOSITE	T&B	TOP AND BOTTOM
CLG	CEILING	GA	GAGE, GAUGE	OSB	ORIENTED STRAND BOARD	T&G	TONGUE AND GROOVE
CLR	CLEAR	GALV	GALVANIZED	PAF	POWDER ACTUATED FASTENER	TB	TOP OF BEAM
CM	CONSTRUCTION MANAGER, -MENT	GC	GENERAL CONTRACTOR	PC	PRECAST	TC	TOP OF CONCRETE
CMU	CONCRETE MASONRY UNIT	GEN	GENERAL	PCF	POUNDS PER CUBIC FOOT	TCA	TORQUE-CONTROLLED ANCHOR
COL	COLUMN	GL	GLUED LAMINATED, GLULAM	PE	PRE-ENGINEERED	TD	TOP OF DECK
COM	COMMON	GND	GROUND	PEN	PENETRATION	THD	THREAD
COMB	COMBINATION	GR	GRADE	PERP	PERPENDICULAR	THK	THICK, -NESS
CONC	CONCRETE	GT	GIRDER TRUSS	PJP	PARTIAL JOINT PENETRATION	TJ	TOP OF JOIST
CONN	CONNECTION	GYP BD	GYPSUM BOARD	PL	PLATE	TL	TOTAL LOAD
CONT	CONTINUOUS, CONTINUE	HAS	HEADED ANCHOR STUD	PLF	POUND PER LINEAR FOOT	TPG	TOPPING
COORD	COORDINATE, COORDINATION	HDG	HOT-DIP GALVANIZED	PNL	PANEL	TRANS	TRANSVERSE
CS	COUNTERSINK	HDR	HEADER	PP	PANEL POINT	TW	TOP OF WALL
CTR	CENTER	HORIZ	HORIZONTAL	PS	PRESTRESSED	TYP	TYPICAL
CY	CUBIC YARD	HP	HIGH POINT	PSF	POUNDS PER SQUARE FOOT	ULT	ULTIMATE
DAB	DEFORMED ANCHOR BAR	HT	HEIGHT	PSI	POUNDS PER SQUARE INCH	UNO	UNLESS NOTED OTHERWISE
DET	DETAIL	ID	INSIDE DIAMETER	PSL	PARALLEL STRAND LUMBER (GENERIC TERM)	VERT	VERTICAL
DEV	DEVELOP	IF	INSIDE FACE	PT	POST TENSIONED, PRESSURE TREATED	VIF	VERIFY IN FIELD
DIAG	DIAGONAL	INT	INTERIOR, INTERMEDIATE	PTN	PARTITION	WP	WORK POINT
DIM	DIMENSION	IT	INVERTED TEE	PWD	PLYWOOD	WT	WEIGHT
DL	DEAD LOAD	JB	JOIST BEARING	QTY	QUANTITY	WWF	WELDED WIRE FABRIC
DN	DOWN	JST	JOIST	R	RADIUS	XS	EXTRA STRONG
DP	DRILLED PIER	JT	JOINT	RE	REFERENCE, REFER TO	XSECT	CROSS SECTION
DT	DOUBLE TEE	K	KIP (1,000 LBS)	RECT	RECTANGLE	XXS	DOUBLE EXTRA STRONG

SYMBOLS KEY

DIRECTION OF DECK SPAN

GRID — GRID DESIGNATION

X — REVISION

X / SX — INDICATES STRUCTURAL ELEVATION

SWx — SHEAR WALL

SHORING

STEP IN FLOOR ELEVATION

CMU (CONCRETE MASONRY UNIT)

BRICK

CIP CONCRETE

PRECAST CONCRETE

EXISTING CONCRETE

EARTH

FX.X — ISOLATED SPREAD FOOTING MARK

FXX — SPREAD FOOTING MARK

STEP — STEP IN BOTTOM OF WALL/GRADE BEAM

DP-XXM (Y) (XX'-X") — DRILLED PIER: XX = Ø, M = PIER MARK, (Y) = BEDROCK PENETRATION (XX'-X") = TOP OF PIER ELEVATION

XX:12 — ROOF SLOPE

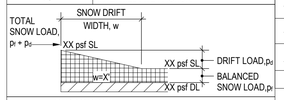
SLOPE — DIRECTION OF SLOPE (DOWN)

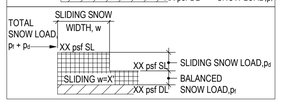
DN — UP — STAIR OR RAMP DIRECTION

STRESSING END ANCHOR

DEAD END ANCHOR

INTERMEDIATE ANCHOR





BUILDING COLUMN DESIGNATIONS

CONT C — COLUMN CONTINUOUS FROM LEVEL BELOW

CXX — COLUMN STARTING AT THIS LEVEL

B — COLUMN STOPPING BELOW THIS LEVEL, SEE FRAMING PLAN AT NEXT LOWER LEVEL

CXX STUB — COLUMN STARTING AND ENDING AT THIS LEVEL OF FRAMING

CXX HGR — COLUMN CONNECTING A LOWER BEAM TO A HIGHER BEAM AT THIS LEVEL OF FRAMING

BRACED/FRAME BAY SYMBOLS

B — INDICATES BRACED BAY MARK

X / SX — INDICATES BRACED BAY ELEVATION

INDICATES BRACED MEMBER (ON PLAN)

INDICATES CONFIGURATION OF INVERTED CHEVRON-TYPE BRACED BAY WITH HSS DIAGONAL BRACES

INDICATES CONFIGURATION OF SINGLE DIAGONAL BRACED BAY WITH HSS DIAGONAL BRACE

RF — INDICATES RIGID (MOMENT) FRAME WITH FULL PENETRATION WELDED BEAM FLANGE TO COLUMN CONNECTIONS

X / SX — INDICATES RIGID (MOMENT) FRAME ELEVATION W/ FULL PENETRATION WELDED BEAM FLANGE TO COLUMN CONNECTIONS

INDICATES BRACED BAY OR FRAMED BAY COLUMN BASE

FULLY WELDED MOMENT FRAME CONNECTION

CANTILEVER MOMENT CONNECTION

LOCATION OF BEND IN BENT BEAM

<X> — NUMBER OF HEADED ANCHOR STUDS

XXX'-X — TOP OF CONCRETE OR MASONRY ELEVATION

[XXX'-X] — TOP OF BEAM ELEVATION

JB XXX'-X — JOIST BEARING ELEVATION

BL XXX'-X — BRICK LEDGE ELEVATION

(XXX'-X) — TOP OF FOOTING ELEVATION

XXX'-X — TOP OF FLOOR ELEVATION

BUILDING COLUMN DESIGNATIONS

XXX'-X — TOP OF CONCRETE OR MASONRY ELEVATION

STEP TOP OF WALL

BL XXX'-X — BRICK LEDGE ELEVATION

(XXX'-X) — TOP OF FOOTING ELEVATION

XXX'-X — TOP OF FLOOR ELEVATION

WOOD BEARING WALL

WOOD SHEAR WALL

A — COLUMN ABOVE

B — COLUMN BELOW

Cxx — COLUMN OR OTHER ELEMENT BELOW SEE SCHEDULES & NOTES
Cx = COLUMN
BPx = BASE PLATE

CONT — COLUMN CONTINUOUS FROM LEVEL BELOW

XX YY — "X" NUMBER OF KING STUDS BELOW
"Y" NUMBER OF TRIMMER STUDS BELOW

WPXY — "WP" = WOOD POST
"X" = NUMBER OF STUDS
"Y" = NOMINAL STUD DIMENSION

LVLXY — "LVL" = LAMINATED VENEER LUMBER
"X" = NUMBER OF PLY'S
"Y" = WIDTH OF LVL

HDx — HOLDOWN

WOOD HEADER

WOOD JOIST OR BEAM SUPPORTED BY METAL HANGER

WOOD JOIST CONTINUOUS OVER INTERMEDIATE SUPPORT

WOOD JOIST BEARING ON TOP OF SUPPORT

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LANDSCAPE ARCHITECT / ARCHITECT
1800 Wazee Street, Suite 450
Denver, CO 80202
p.303.607.0977

CIVIL ENGINEER / STRUCTURAL ENGINEER
JVA Incorporated
1675 Larimer Street, #320
Denver, CO 80202
p. 303.444.1951

ELECTRICAL ENGINEER
Ackerman Engineering, Inc.
3000 Youngfield Street, #264
Wheat Ridge, CO 80215
p. 303.278.7297

IRRIGATION
Avocet Irrigation
11757 W. Ken Caryl Ave., Suite F-509
Littleton, CO 80127
p. 303.986.2175

MECHANICAL ENGINEER
ENVISION Mechanical Engineers, Inc.
9777 Pyramid Court, #260
Englewood, CO 80112
p. 303.688.0223

Town of Parker
SALISBURY PARK NORTH - PHASE 1
11700 MOTSENBOCKER RD
PARKER, CO 80134

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ARCHITECTURE
LANDSCAPE ARCHITECTURE
PLANNING
INTERIOR DESIGN

Δ	DATE	DESCRIPTION

Project Number: 223072.00
Sheet Issue Date: 2024-11-22
Drawn By: DJM
Checked By: BAR

Key Map

Drawing
ABBREVIATIONS & SMBOLS KEY

S-002

SITE PLAN SUBMITTAL

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THE LINE SHOWN ABOVE IS EXACTLY 1" LONG ON ALL ANSI SHEETS ORIGINAL PAGE SIZE

THE SQUARES ABOVE ARE COLOR WITH BLACK AND WHITE LEFTOVER, IF PRINTED CORRECTLY

STEEL SPECIAL INSPECTION (IBC 1705.2, 1705.12.3 & 1705.13.1)

ITEM	REQUIRED QUALIFICATIONS	FREQUENCY	DETAILED INSTRUCTIONS
FABRICATORS			(IBC 1704.2.5 & 1705.11)
In-plant Inspection	AWS/AISC-SSI ICC-SWSI		Required unless Fabricator is approved and follows procedures of 1704.2.5.1
PRIOR TO WELDING			(TABLE N5.4-1, AISC 360-16)
Verify welding procedures (WPS) and consumable certificates	AWS-CWI ASNT	Continuous	
Material identification	AWS-CWI ASNT	Periodic	Verify type and grade of material.
Welder identification	AWS-CWI ASNT	Periodic	A system shall be maintained by which a welder who has welded a joint or member can be identified.
Fit-up groove welds	AWS-CWI ASNT	Periodic	Verify joint preparation, dimensions, cleanliness, tacking, and backing.
Fit-up CJP groove welds of HSS joints without backing	AWS-CWI ASNT	Periodic	Verify joint preparation, dimensions, cleanliness, and tacking
Access holes	AWS-CWI ASNT	Periodic	Verify configuration and finish.
Fit-up of fillet welds	AWS-CWI ASNT	Periodic	Verify alignment, gaps at root, cleanliness of steel surfaces, and tack weld quality and location.
DURING WELDING			(TABLE N5.4-2, AISC 360-16)
Use of qualified welders	AWS-CWI ASNT	Periodic	Verify that welders are appropriately qualified.
Control and handling of welding consumables	AWS-CWI ASNT	Periodic	Verify packaging and exposure control.
Cracked tack welds	AWS-CWI ASNT	Periodic	Verify that welding does not occur over cracked tack welds.
Environmental conditions	AWS-CWI ASNT	Periodic	Verify wind speed is within limits as well as precipitation and temperature.
WPS followed	AWS-CWI ASNT	Periodic	Verify items such as settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position.
Welding techniques	AWS-CWI ASNT	Periodic	Verify interpass and final cleaning, each pass is within profile limitations, and quality of each pass.
AFTER WELDING			(TABLE N5.4-3, AISC 360-10)
Welds cleaned	AWS-CWI ASNT	Periodic	Verify that welds have been properly cleaned.
Size, length, and location of welds	AWS-CWI ASNT	Continuous	
Welds meet visual acceptance criteria	AWS-CWI ASNT	Continuous	
Arc strikes	AWS-CWI ASNT	Continuous	
k-area	AWS-CWI ASNT	Continuous	
Weld access holes in heavy shapes	AWS-CWI ASNT	Continuous	
Backing & weld tabs removed	AWS-CWI ASNT	Continuous	
Repair activities	AWS-CWI ASNT	Continuous	
Document acceptance or rejection of welded joint/member	AWS-CWI ASNT	Continuous	
AFTER BOLTING			(TABLE N5.6-3, AISC 360-16)
Document acceptance or rejection of bolted connections	AWS/AISC-SSI ICC-SWSI	Continuous	
OTHER STEEL INSPECTIONS			(SECTION N5.7, AISC 360-16; Tables J8-1 & J10-1, AISC 341-16)
Structural steel details	PE/SE	Periodic	All fabricated steel or steel frames shall be inspected to verify compliance with the details shown in the construction documents, such as braces, stiffeners, member locations, and proper application of joint details at each connection.
Anchor rods and other embedments supporting structural steel	ACI-CCI	Periodic	Shall be on the premises during the placement of anchor rods and other embedments supporting structural steel for compliance with construction documents. Verify the diameter, grade, type, and length of the anchor rod or embedded item, and the extent or depth of embedment prior to placement of concrete.
STEEL ROOF AND FLOOR DECKS			(IBC 1705.2.2; Section 6.1 of SDI QA/QC - 2017)
Material verification of cold-formed steel deck	AWS/AISC-SSI ICC-SWSI	Periodic	Confirm that identification markings are provided to conform to ASTM standards specified on construction documents.
Roof deck welds	AWS-CWI	Periodic	Visual inspection is required to confirm that weld meets acceptance criteria of AWS D1.3 and SDI C, SDI NC, SDI RD and manufacturer's instructions.
Roof mechanical fasteners	AWS/AISC-SSI ICC-SWSI	Periodic	Visual inspection to confirm fasteners are installed per SDI C, SDI NC, SDI RD and manufacturer's instructions.
Steel deck installation	AWS/AISC-SSI ICC-SWSI	Periodic	Verify deck is installed per the approved construction documents, installation drawings, shop drawings and applicable reference standards.

MASONRY SPECIAL INSPECTION (IBC 1705.4)

ITEM	REQUIRED QUALIFICATIONS	FREQUENCY	DETAILED INSTRUCTIONS
PRIOR TO CONSTRUCTION			(TABLE 3, TMS-602/ACI530.1-16)
Review material certificates, mix designs, test results and construction procedures	PE	Periodic	Verify that materials conform to the requirements of the approved construction documents. Mix design, test results, material certificates, and construction procedures should be submitted for review. Mortar mix designs shall conform to ASTM C 270 while grout shall conform to ASTM C 476. Material certificates shall be provided for the following: reinforcement; anchors, ties, fasteners, and metal accessories; masonry units; mortar and grout materials. Construction procedures for cold-weather or hot-weather construction shall be reviewed.
AS CONSTRUCTION BEGINS			(TABLE 4, TMS-602/ACI530-16)
Proportions of site-prepared mortar	ICC-SMSI	Periodic	Verify that mortar is of the type and color specified on the construction documents, that it conforms to ASTM C 270, and that it is mixed in accordance with Article 2.6 A of TMS-602.
PRIOR TO GROUTING			(TABLE 4, TMS-602/ACI530-16)
Grout space	ICC-SMSI	Periodic	Verify that grout space is free of mortar droppings, debris, loose aggregate, and other deleterious materials and that cleanouts are provided per Article 3.2 D and 3.2 F of TMS-602.
DURING MASONRY CONSTRUCTION			(TABLE 4, TMS-602/ACI530-16)
Size and location of structural elements	ICC-SMSI	Periodic	Verify the locations of structural elements with respect to the approved plans and confirm that tolerances meet the requirements of Article 3.3 F of TMS 602.
Materials and procedures with the approved submittals	ICC-SMSI	Periodic	Verify materials and construction procedures are in accordance with approved submittals per Article 1.5 of TMS-602
Placement of masonry units and mortar joint construction	ICC-SMSI	Periodic	Verify placement of masonry units and mortar joints are in accordance with Article 3/3B of TMS-602
Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction	ICC-SMSI	Periodic	Verify that correct anchorages and connections are provided per the approved plans and Sections 1.16.4.3 and 1.17.1 of TMS 402
Welding of reinforcement	ICC-SMSI AWS-CWI	Continuous	
Preparation, construction, and protection of masonry during cold weather (<40°F) or hot weather (>90°F).	ICC-SMSI	Periodic	Verify that cold-weather construction is performed in accordance with Article 1.8 C of TMS 602 and hot weather construction per Article 1.8 D of TMS 602.
Construction of mortar joints	ICC-SMSI	Periodic	Verify that mortar joints are placed in accordance with Article 3.3 B of TMS 602.

CONCRETE SPECIAL INSPECTION (IBC 1705.3 & 1705.12.1)

ITEM	REQUIRED QUALIFICATIONS	FREQUENCY	DETAILED INSTRUCTIONS
Reinforcing steel	ACI-CCI ICC-RCSI	Periodic	Verify prior to placing concrete that reinforcing is of specified type, grade and size; that it is free of oil, dirt and rust; that it is located and spaced properly; that hooks, bends, ties, stirrups and supplemental reinforcement are placed correctly; that lap lengths, stagger and offsets are provided; and that all mechanical connections are installed per the manufacturer's instructions and/or evaluation report.
Welding of reinforcing steel	AWS-CWI	Periodic	Visually inspect all welds and also verify weldability of reinforcing steel based upon carbon equivalent and in accordance with AWS D1.4.
Cast-in bolts & embeds	ACI-CCI ICC-RCSI	Periodic	Inspection of anchors or embeds cast in concrete is required when allowable loads have been increased or where strength design is used.
Post-installed anchors or dowels	ACI-CCI ICC-RCSI	Periodic	All post-installed anchors/dowels shall be specially inspected as required by the approved ICC-ES report. Horizontally or upwardly inclined anchors that resist sustained tension loads require continuous inspection and approved installers.
Use of required mix design	ACI-CCI ICC-RCSI	Periodic	Verify that all mixes used comply with the approved construction documents; ACI 318: Ch. 19, 26.4.3, 26.4.4; and IBC 1904.1, 1904.2, 1908.2, 1908.3.
Concrete sampling for strength tests, slump, air content, and temperature	ACI-CFTT ACI-SIT	Continuous	
Concrete placement	ACI-CCI ICC-RCSI	Continuous	
Curing temperature and techniques	ACI-CCI ICC-RCSI	Periodic	Verify that the ambient temperature for concrete is kept at > 50°F for at least 7 days after placement. High-early-strength concrete shall be kept at > 50°F for at least 3 days. Accelerated curing methods may be used (see ACI 318: 26.4.7-26.4.9). The ambient temperature for shotcrete shall be > 40°F for the same period of time as noted for concrete. Shotcrete shall be kept continuously moist for at least 24 hours after shotcreting. All concrete materials, reinforcement, forms, fillers, and ground shall be free from frost. In hot weather conditions ensure that appropriate measures are taken to avoid plastic shrinkage cracking and that the specified water/cement ratio is not exceeded.
Formwork		Periodic	Verify that the forms are placed plumb and conform to the shapes, lines, and dimensions of the members as required by the approved construction documents.

SOIL SPECIAL INSPECTION (IBC 1705.6)

ITEM	REQUIRED QUALIFICATIONS	FREQUENCY	DETAILED INSTRUCTIONS
SHALLOW FOUNDATIONS			(IBC 1705.6)
Verify subgrade	PE/GE	Periodic	Prior to placement of concrete inspect soils below footings for adequate bearing capacity and consistency with geotechnical report.
CONTROLLED STRUCTURAL FILL			(IBC 1705.6)
Excavations	PE/GE	Periodic	Verify excavations extend to proper depth and material prior to placement of compacted fill or concrete.
Fill materials	PE/GE	Periodic	Perform classification and testing of compacted fill materials. Check for proper classifications and gradations at each lift and not less than once for each 10,000ft² of surface area.
Placement and compaction		Continuous	Verify proper materials, densities and lift thicknesses during placement and compaction.
Subgrade preparation	PE/GE	Periodic	Verify that subgrade has been appropriately prepared prior to placing compacted fill.
Density		Continuous	Test density of each lift by nuclear methods (ASTM D2922).

STATEMENT OF SPECIAL INSPECTIONS

This *Statement of Special Inspections* is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Special Inspection Coordinator and the identity of other approved agencies to be retained for conducting these inspections and tests. This *Statement of Special Inspections* encompass the following disciplines: Structural

The Special Inspection Coordinator shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

Interim reports shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge.
Interim Report Frequency: Within 72 hours of inspection, unless indicated otherwise.

A *Final Report of Special Inspections* documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy.

Job site safety and means and methods of construction are solely the responsibility of the Contractor.

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided if requested.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test or inspection have a specific certification or license as indicated below, such designation shall appear below the *Agency Number* on the Schedule.

PE/SE Structural Engineer – a licensed SE or PE specializing in the design of building structures
PE/GE Geotechnical Engineer – a licensed PE specializing in soil mechanics and foundations
EIT Engineer-In-Training – a graduate engineer who has passed the Fundamentals of Engineering examination

American Concrete Institute (ACI) Certification
ACI-CFTT Concrete Field Testing Technician – Grade 1
ACI-CCI Concrete Construction Inspector
ACI-LTT Laboratory Testing Technician – Grade 1 & 2
ACI-STT Strength Testing Technician

American Welding Society (AWS) Certification
AWS-CWI Certified Welding Inspector
AWS/AISC-SSI Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification
ASNT Non-Destructive Testing Technician – Level II or III

International Code Council (ICC) Certification
ICC-SMSI Structural Masonry Special Inspector
ICC-SWSI Structural Steel and Welding Special Inspector
ICC-SFSI Spray-Applied Fireproofing Special Inspector
ICC-PCSI Prestressed Concrete Special Inspector
ICC-RCSI Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)
NICET-CT Concrete Technician – Levels I, II, III & IV
NICET-ST Soils Technician - Levels I, II, III & IV
NICET-GET Geotechnical Engineering Technician - Levels I, II, III & IV

Exterior Design Institute (EDI) Certification
EDI-EIFS EIFS Third Party Inspector

Quality Assurance Plans

Quality Assurance for Seismic Resistance
Seismic Design Category: B
Quality Assurance Plan Required: No

Quality Assurance for Wind Requirements
Allowable Stress Design Wind Speed V_{asd} (3 second gust): 110 mph
Basic Wind Speed V_{ult} (3 second gust): 130 mph
Wind Exposure Category: C
Quality Assurance Plan Required: No

Statement of Responsibility
Each contractor responsible for the construction or fabrication of a system or component designated above must submit a Statement of Responsibility.

Prepared by:

BRETT A. ROBINSON, P.E. / Signature Date

Owner's Authorization:

Signature Date

Building Official's Acceptance:

Signature Date

SCHEDULE OF INSPECTION AND TESTING AGENCIES

SPECIAL INSPECTION AGENCIES	FIRM	ADDRESS, TELEPHONE, E-MAIL
Special Inspection Coordinator	TBD	
Inspector	TBD	
Inspector	TBD	
Testing Agency	TBD	
Testing Agency	TBD	
Continuous	TBD	
Other	TBD	

Structural Observations
The following structural observations will be made to visually observe representative locations of structural systems, details, and load paths for general conformance to the approved construction documents. The Contractor shall notify the Engineer a minimum of 5 working days before work is ready to be observed to allow scheduling of site visit.

1. FOUNDATIONS
   A. Grade Beams: Observation of layout and reinforcement placement near the start of operation.
   B. Footings: Observation of layout and reinforcement placement near the start of operation.
   C. Foundation Walls: Observation of layout and reinforcement placement near the start of operation.
2. CONCRETE
   A. Walls: Observation of reinforcement placement near the start of operation.
3. MASONRY
   A. Walls: Observation of layout, control joint placement, and reinforcement placement near the start of operation.
4. STEEL FRAMING
   A. Elevated Floors: Observation of steel deck, headed anchor studs, and slab reinforcement prior to first slab-on-metal deck placement.

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LANDSCAPE ARCHITECT / ARCHITECT
1800 Wazee Street, Suite 450
Denver, CO 80202
p.303.607.0977

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JVA Incorporated
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ARCHITECTURE

LANDSCAPE ARCHITECTURE

PLANNING

INTERIOR DESIGN

Δ	DATE	DESCRIPTION

Project Number: 223072.00
Sheet Issue Date: 2024-11-22
Drawn By: LB
Checked By: BAR

Key Map

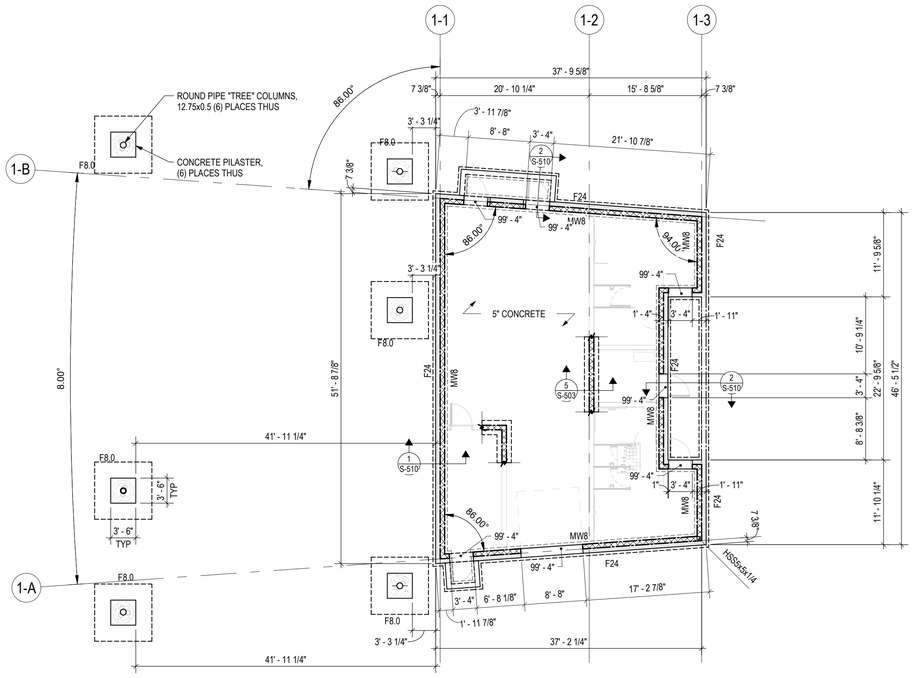
Drawing

IBC 2021 STATEMENT OF SPECIAL INSPECTION

S-003

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COMMUNITY HUB PAVILION FOUNDATION

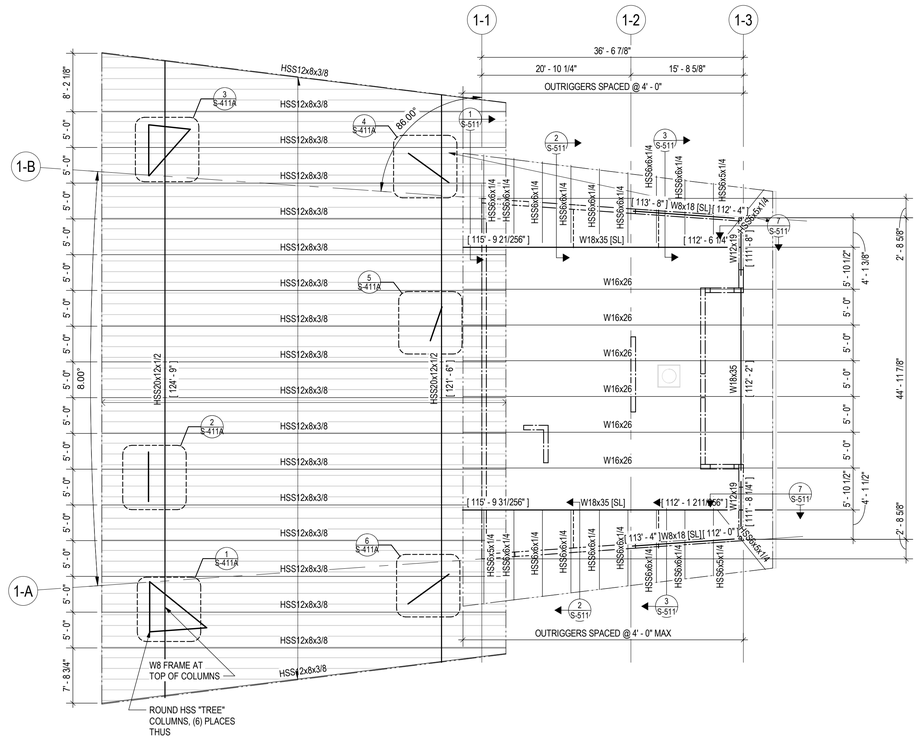
1" = 10'-0"

NORTH

TYPICAL EXTERIOR STEMWALL & FOOTING:
CONTINUOUS 1'-3 WIDE x 2'-0 DEEP CONCRETE STEMWALL REINFORCED W/ CONTINUOUS (2) #5 TOP & BOTTOM, CONTINUOUS F24 FOOTING CENTERED BELOW W/ #4 DOWELS PROJECTING INTO STEMWALL @ 16"

FOOTING NOTES:
- CENTER FOOTINGS UNDER STEMWALLS, PILASTERS, & COLUMNS, TYPICAL UNLESS NOTED OTHERWISE
- FOOTING WIDTHS SHALL NOT VARY IN THE FIELD FROM SIZE NOTED; OVERSIZE FOOTINGS ARE NOT ALLOWED
- LAPPED BOARD FORMING NOT ALLOWED
- TRENCH FORMING NOT ALLOWED

TYPICAL INTERIOR SLAB ON GRADE:
5" THICK CONCRETE SLAB ON UNDISTURBED NATURAL SOILS OR COMPACTED STRUCTURAL FILL PER SOILS REPORT; REINFORCE SLAB W/ #4 @ 16", EA WAY, AT MID-DEPTH; ADD (2) #4 x 5'-0 DIAGONAL BARS AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS. PROVIDE SAWCUT OR FORMED CONTROL JOINTS, PER 8/S-500, @ 12'-0± IN EACH DIRECTION. JOINTS SHALL BE CONTINUOUS, NOT STAGGERED OR OFFSET. ASPECT RATIO SHALL BE A MAXIMUM OF 1.5 TO 1.



COMMUNITY HUB PAVILION ROOF PLAN

1" = 10'-0"

NORTH

TYPICAL ROOF DECK:
1 1/2" DEEP x 22 GAGE STEEL DECK (VULCRAFT TYPE 1.5B22) WITH 5/8" PUDDLE WELDS @ 12" (36/4) AT INTERMEDIATE SUPPORTS & EDGES AND #10 SDST SCREWS @ 12" AT SIDELAP CONNECTIONS. STEEL DECK SHALL BE CONTINUOUS OVER THREE OR MORE SUPPORTS.

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p.303.607.0977

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JVA Incorporated
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Denver, CO 80202
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PARKER, CO 80134

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ARCHITECTURE
LANDSCAPE ARCHITECTURE
PLANNING
INTERIOR DESIGN

Δ	DATE	DESCRIPTION

Project Number:	223072.00
Sheet Issue Date:	2024-11-22
Drawn By:	DJM
Checked By:	BAR

Key Map

Drawing
COMMUNITY HUB FOUNDATION & ROOF PLANS

S-111A

SITE PLAN SUBMITTAL

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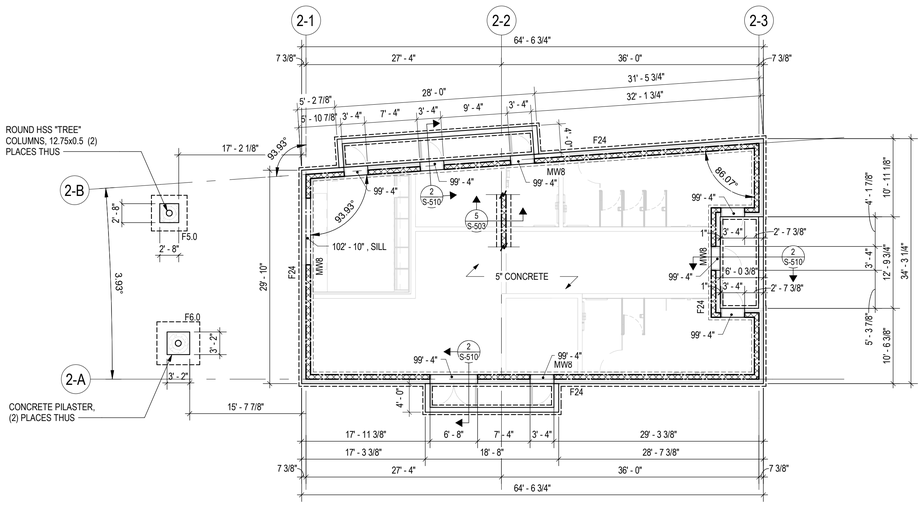
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CONCESSIONS & RESTROOMS FOUNDATION PLAN

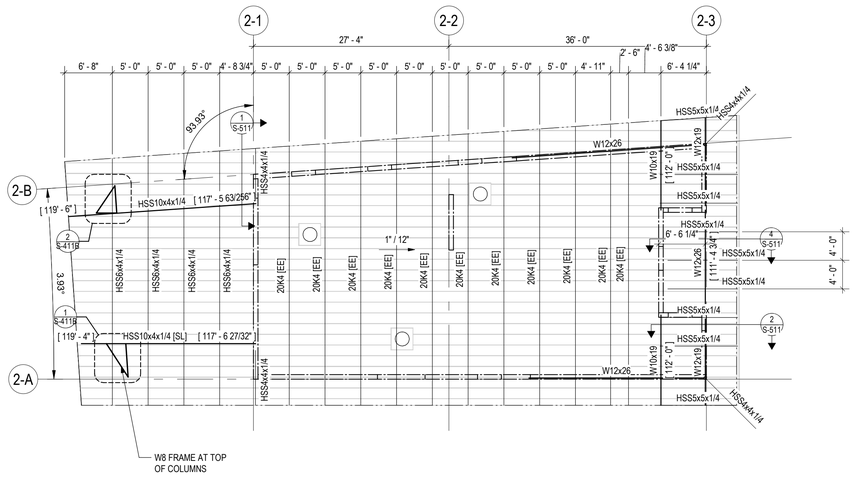
1" = 10'-0"

NORTH

TYPICAL EXTERIOR STEMWALL & FOOTING:
CONTINUOUS 1'-3 WIDE x 2'-0 DEEP CONCRETE STEMWALL REINFORCED W/ CONTINUOUS (2) #5 TOP & BOTTOM. CONTINUOUS F24 FOOTING CENTERED BELOW W/ #4 DOWELS PROJECTING INTO STEMWALL @ 16'

FOOTING NOTES:
- CENTER FOOTINGS UNDER STEMWALLS, PILASTERS, & COLUMNS, TYPICAL UNLESS NOTED OTHERWISE
- FOOTING WIDTHS SHALL NOT VARY IN THE FIELD FROM SIZE NOTED; OVERSIZE FOOTINGS ARE NOT ALLOWED
- LAPPED BOARD FORMING NOT ALLOWED
- TRENCH FORMING NOT ALLOWED

TYPICAL INTERIOR SLAB ON GRADE:
5" THICK CONCRETE SLAB ON UNDISTURBED NATURAL SOILS OR COMPACTED STRUCTURAL FILL PER SOILS REPORT; REINFORCE SLAB W/ #4 @ 16", EA WAY, AT MID-DEPTH; ADD (2) #4 x 5'-0 DIAGONAL BARS AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS. PROVIDE SAWCUT OR FORMED CONTROL JOINTS, PER 8/S-500, @ 12'-0± IN EACH DIRECTION. JOINTS SHALL BE CONTINUOUS, NOT STAGGERED OR OFFSET. ASPECT RATIO SHALL BE A MAXIMUM OF 1.5 TO 1.



CONCESSIONS & RESTROOMS ROOF PLAN

1" = 10'-0"

NORTH

OPEN WEB STEEL ROOF JOIST NOTES:
- K SERIES JOIST END BEARING SEAT DEPTH = 2 1/2", TYPICAL
- K SERIES JOISTS SPACED @ 5'-0 (MAX)
- LH SERIES JOIST END BEARING SEAT DEPTH = 5", TYPICAL
- LH SERIES JOISTS SPACED @ 10'-0 (MAX)
- '(EE)' INDICATES JOISTS WITH EXTENDED ENDS

TYPICAL ROOF DECK:
1 1/2" DEEP x 22 GAGE STEEL DECK (VULCRAFT TYPE 1.5B22) WITH 5/8" PUDDLE WELDS @ 12" (36/4) AT INTERMEDIATE SUPPORTS & EDGES AND #10 SDST SCREWS @ 12" AT SIDELAP CONNECTIONS. STEEL DECK SHALL BE CONTINUOUS OVER THREE OR MORE SUPPORTS.

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CONCESSIONS & RESTROOM FOUNDATION & ROOF PLANS

S-111B

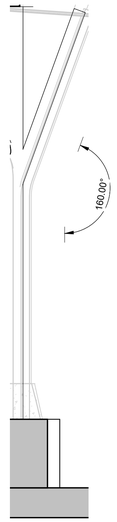
SITE PLAN SUBMITTAL

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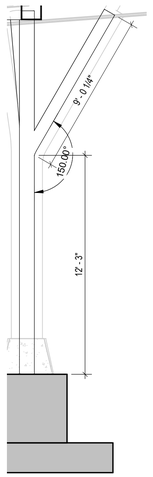
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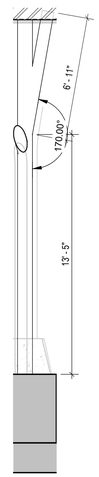
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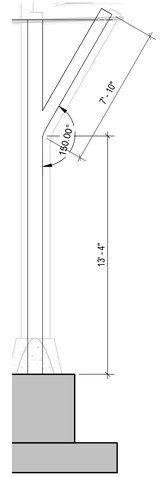
14 S-411A SECTION - A8 COLUMN
1/4" = 1'-0"



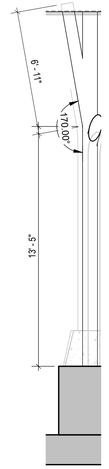
15 S-411A SECTION - A9 COLUMN
1/4" = 1'-0"



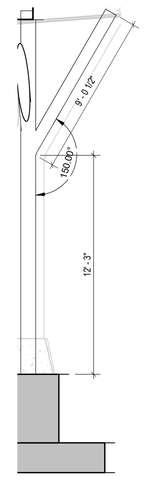
16 S-411A SECTION - A10 COLUMN
1/4" = 1'-0"



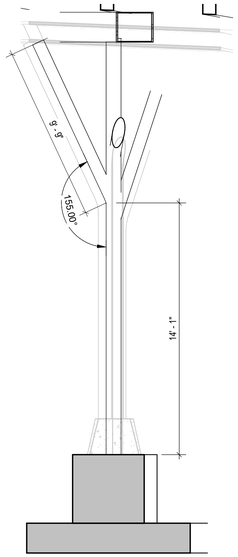
17 S-411A SECTION - A11 & A12 COLUMNS
1/4" = 1'-0"



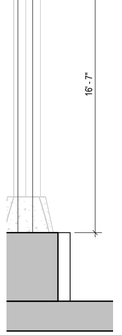
18 S-411A SECTION - A13 COLUMN
1/4" = 1'-0"



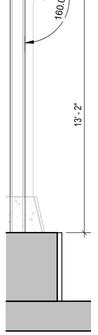
19 S-411A SECTION - A14 COLUMN
1/4" = 1'-0"



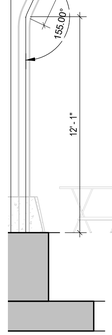
7 S-411A SECTION - A1 COLUMN
1/4" = 1'-0"



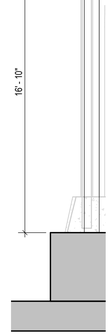
8 S-411A SECTION - A2 COLUMN
1/4" = 1'-0"



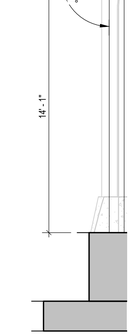
9 S-411A SECTION - A3 COLUMN
1/4" = 1'-0"



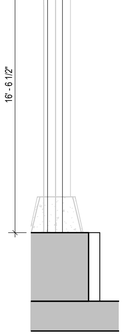
10 S-411A SECTION - A4 COLUMN
1/4" = 1'-0"



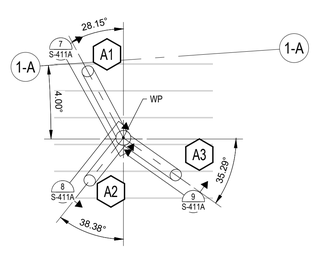
11 S-411A SECTION - A5 COLUMN
1/4" = 1'-0"



12 S-411A SECTION - A6 COLUMN
1/4" = 1'-0"

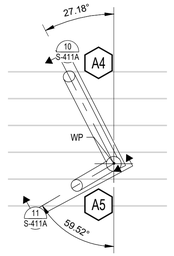


13 S-411A SECTION - A7 COLUMN
1/4" = 1'-0"

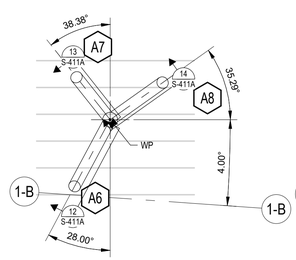


1 S-411A COLUMN PLAN 1
1/4" = 1'-0"

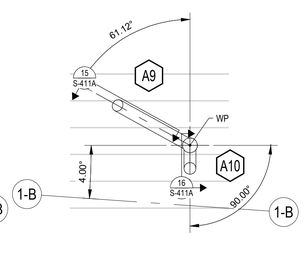
0 1' 2' 4' 8'



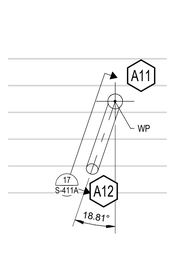
2 S-411A COLUMN PLAN 2
1/4" = 1'-0"



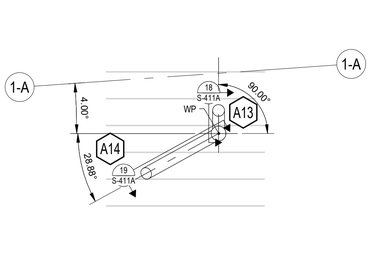
3 S-411A COLUMN PLAN 3
1/4" = 1'-0"



4 S-411A COLUMN PLAN 4
1/4" = 1'-0"



5 S-411A COLUMN PLAN 5
1/4" = 1'-0"



6 S-411A COLUMN PLAN 6
1/4" = 1'-0"

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Denver, CO 80202
p.303.607.0977

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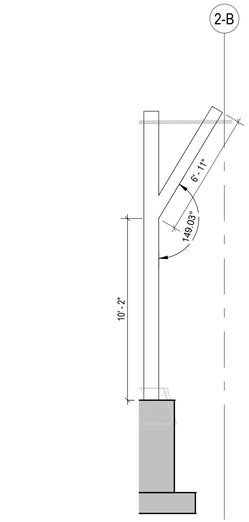
SITE PLAN SUBMITTAL

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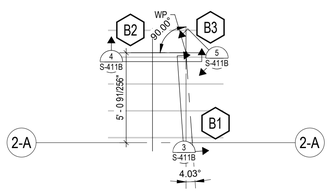
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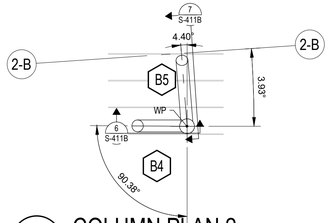
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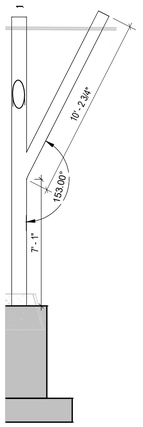
7 S-411B **SECTION - B5 COLUMN**
1/4" = 1'-0"



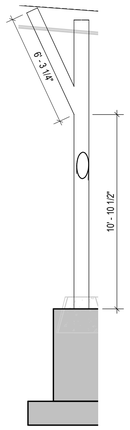
1 S-411B **COLUMN PLAN 1**
1/4" = 1'-0"



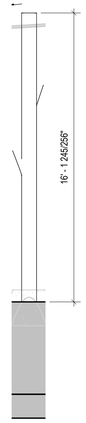
2 S-411B **COLUMN PLAN 2**
1/4" = 1'-0"



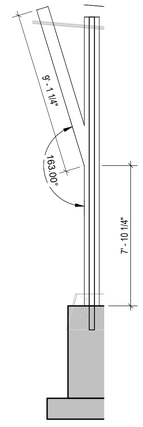
3 S-411B **SECTION - B1 COLUMN**
1/4" = 1'-0"



4 S-411B **SECTION - B2 COLUMN**
1/4" = 1'-0"



5 S-411B **SECTION - B3 COLUMN**
1/4" = 1'-0"



6 S-411B **SECTION - B4 COLUMN**
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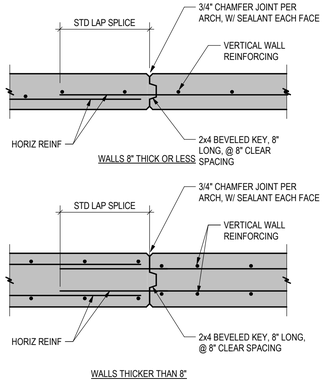
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CONCESSIONS & RESTROOMS COLUMN PLANS & SECTION

S-411B

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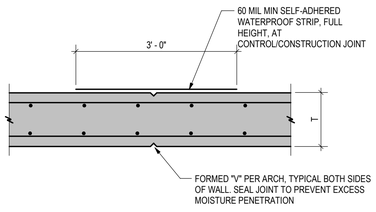
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5 S-500 TYP CONC VERT JOINT IN WALL

3/4" = 1'-0"

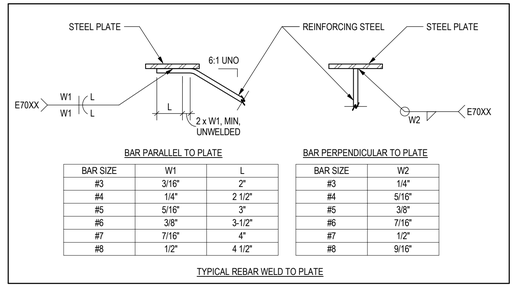


1. PROVIDE FULL-HEIGHT VERTICAL "V" GROOVE CONTROL JOINTS.
   JOINT SPACING = 3 X WALL HEIGHT FOR WALLS <= 8'-0 HIGH.
   JOINT SPACING = WALL HEIGHT FOR WALLS >= 12'-0 HIGH.
   JOINT SPACING NOT TO EXCEED 25'-0.
   DEPTH OF GROOVE = T / 8.

2. LOCATION AND DETAIL OF JOINTS SHALL BE APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO PLACING CONCRETE

6 S-500 TYP CONC VERT JOINT IN SITE WALL

3/4" = 1'-0"



BAR PARALLEL TO PLATE

BAR SIZE	W1	L
#3	3/16"	2"
#4	1/4"	2 1/2"
#5	5/16"	3"
#6	3/8"	3-1/2"
#7	7/16"	4"
#8	1/2"	4 1/2"

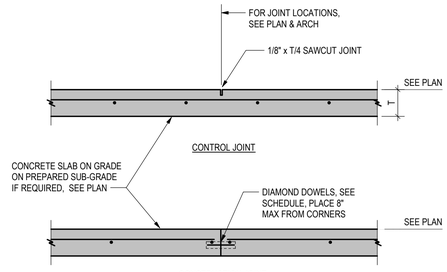
BAR PERPENDICULAR TO PLATE

BAR SIZE	W2
#3	1/4"
#4	5/16"
#5	3/8"
#6	7/16"
#7	1/2"
#8	9/16"

TYPICAL REBAR WELD TO PLATE

7 S-500 TYP REINFORCING WELD DETAILS

3/4" = 1'-0"



DOWEL SIZE & SPACING AT CONSTRUCTION JOINTS		
SLAB DEPTH (INCHES)	DOWEL DIMENSIONS (INCHES)	DOWEL SPACING (INCHES)
5 TO 6	1/4 x 4-1/2 x 4-1/2	18
7 TO 8	3/8 x 4-1/2 x 4-1/2	18
9 TO 11	3/4 x 4-1/2 x 4-1/2	20

8 S-500 TYP REINF SLAB ON GRADE JOINTS

3/4" = 1'-0"

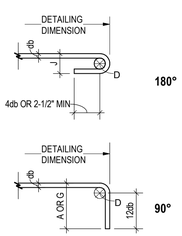
TYPICAL CONCRETE REINFORCING LAP & EMBEDMENT LENGTHS (UNO)							
		f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI	
BAR SIZE	TYPE	TOP BAR	OTHER BAR	TOP BAR	OTHER BAR	TOP BAR	OTHER BAR
#4	EMBED	29	22	25	19	22	17
	LAP	37	29	32	25	29	22
#5	EMBED	36	28	31	24	28	22
	LAP	47	36	40	31	36	28
#6	EMBED	43	33	37	29	33	26
	LAP	56	43	48	37	43	33
#7	EMBED	63	48	54	42	49	37
	LAP	81	63	70	54	63	49
#8	EMBED	72	55	62	48	55	43
	LAP	93	72	80	62	72	55
#9	EMBED	81	62	70	54	63	48
	LAP	105	81	91	70	81	63
#10	EMBED	91	70	79	61	70	54
	LAP	118	91	102	79	91	70
#11	EMBED	101	78	87	67	78	60
	LAP	131	101	113	87	101	78

NOTES:
1. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW BAR
2. TABULATED VALUES ARE BASED ON GRADE 60 NON-EPOXY-COATED REINFORCING BARS AND NORMAL WEIGHT CONCRETE
3. VALUES ARE IN INCHES

1 S-500 TYP CONC EMBED & LAP

NO SCALE

STANDARD HOOKS

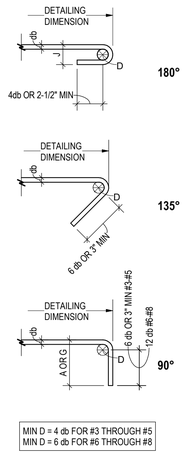


MIN D = 6 db FOR #3 THROUGH #8
MIN D = 8 db FOR #9, #10, AND #11

	STANDARD HOOKS		STIRRUPS & TIE HOOKS	
BAR SIZE	180° (J)	90° (A OR G)	180° (J)	90° (A OR G)
#3	3"	6"	2 1/4"	4 1/8"
#4	4"	8"	3"	4 1/2"
#5	5"	10"	3 3/4"	5 5/8"
#6	6"	1'-0	6"	1'-0
#7	7"	1'-2	7"	1'-2
#8	8"	1'-4	8"	1'-4
#9	11 3/4"	1'-7		
#10	1'-1 1/4	1'-10		
#11	1'-2 3/4	2'-0		

ALL GRADES OF STEEL
D = FINISHED INSIDE BEND DIAMETER
db = NOMINAL BAR DIAMETER

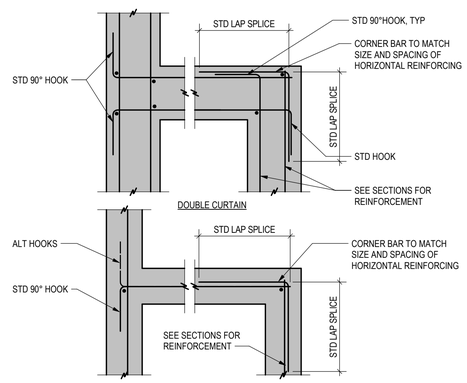
STIRRUP & TIE HOOKS



MIN D = 4 db FOR #3 THROUGH #5
MIN D = 6 db FOR #6 THROUGH #8

2 S-500 TYP CONC HOOKS & STIRRUPS

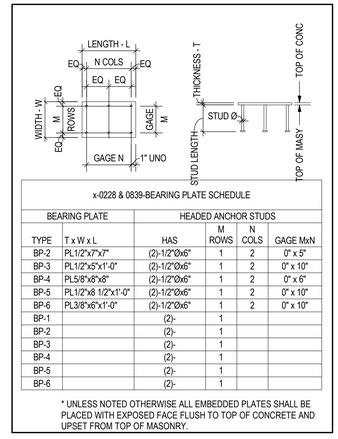
NO SCALE



3 S-500 TYP CONC WALL INTERSECTIONS

3/4" = 1'-0"

0 6" 1' 2' 3'



x-0228 & 0839-BEARING PLATE SCHEDULE

BEARING PLATE		HEADED ANCHOR STUDS			
TYPE	T x W x L	HAS	M ROWS	N COLS	GAGE M x N
BP-2	PL1/2"x7"x7"	(2)-1/2"Ø x6"	1	2	0" x 5"
BP-3	PL1/2"x5"x1'-0"	(2)-1/2"Ø x6"	1	2	0" x 10"
BP-4	PL5/8"x8"x8"	(2)-1/2"Ø x6"	1	2	0" x 6"
BP-5	PL1/2"x8 1/2"x1'-0"	(2)-1/2"Ø x6"	1	2	0" x 10"
BP-6	PL3/8"x6"x1'-0"	(2)-1/2"Ø x6"	1	2	0" x 10"
BP-1		(2)-	1		
BP-2		(2)-	1		
BP-3		(2)-	1		
BP-4		(2)-	1		
BP-5		(2)-	1		
BP-6		(2)-	1		

* UNLESS NOTED OTHERWISE ALL EMBEDDED PLATES SHALL BE PLACED WITH EXPOSED FACE FLUSH TO TOP OF CONCRETE AND UPSET FROM TOP OF MASONRY.

4 S-500 TYP BEARING PL DETAIL & SCHEDULE

3/4" = 1'-0"

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PLANNING
INTERIOR DESIGN

Δ	DATE	DESCRIPTION

Project Number:	223072.00
Sheet Issue Date:	2024-11-22
Drawn By:	LB
Checked By:	BAR

Key Map

Drawing
TYPICAL CONCRETE DETAILS

S-500

SITE PLAN SUBMITTAL

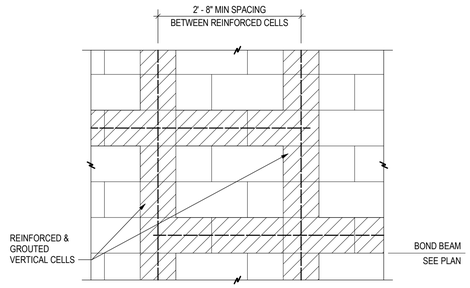
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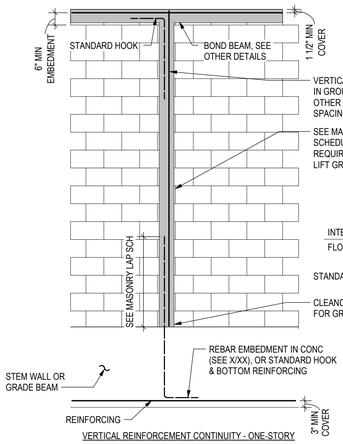
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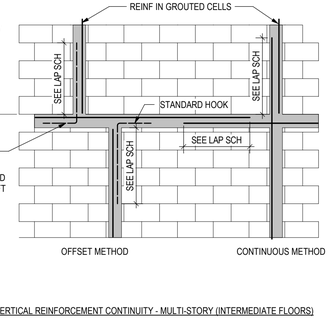
K Y M C



TYPICAL WHERE BOND BEAMS OVERLAP AT DIFFERENT HEIGHTS:
OVERLAP BOND BEAM REINFORCEMENT FROM ONE REINFORCED & GROUTED VERTICAL CELL TO THE NEXT ADJACENT REINFORCED & GROUTED VERTICAL CELL



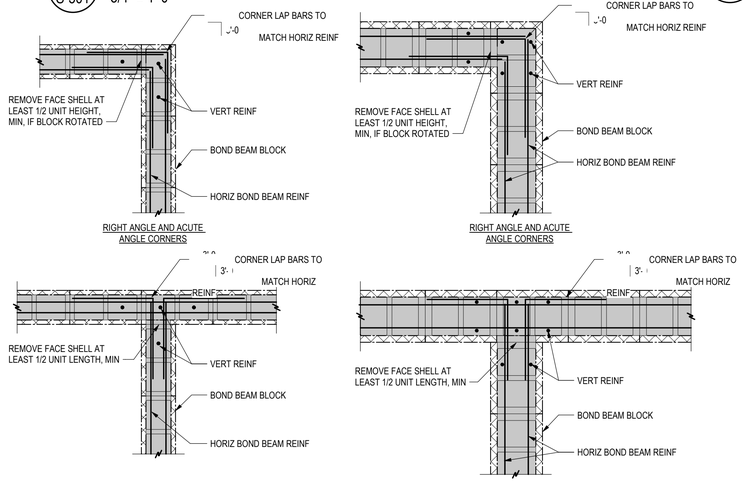
VERTICAL REINFORCEMENT CONTINUITY - ONE-STORY



VERTICAL REINFORCEMENT CONTINUITY - MULTI-STORY (INTERMEDIATE FLOORS)

8 S-501 TYP CMU BOND BEAM LAP

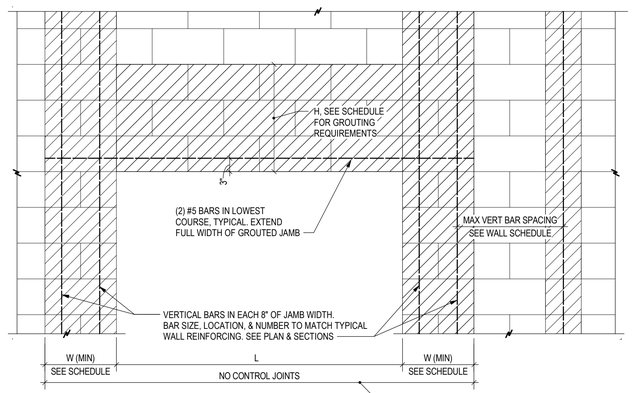
3/4" = 1'-0"



9 S-501 TYP CMU WALL CONTINUITY

3/8" = 1'-0"

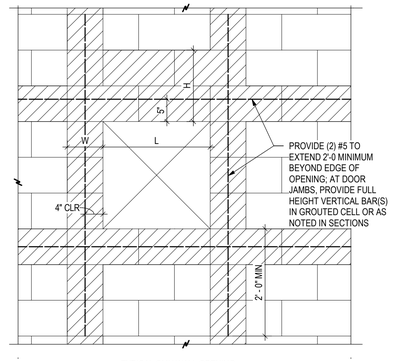
TYPICAL MASONRY LINTELS OVER 4'-1 (UNO)		
L (OPENING WIDTH)	H (HEIGHT OF SOLID GROUTED LINTEL)	W (SOLID GROUTED JAMB WIDTH)
4'-1 TO 6'-0	2'-0	1'-4
6'-1 TO 10'-0	3'-4	2'-0



TYPICAL CMU LINTEL

NOTE:
MASONRY CONTROL JOINTS MAY NOT OCCUR AT LINTEL OR JAMB LOCATIONS. SEE ARCH FOR JOINT LOCATIONS. CONTRACTOR TO CONTACT JVA IF JOINTS OCCUR AT LINTELS OR JAMBS

TYPICAL MASONRY LINTELS UP TO 6'-0 (UNO)		
L (OPENING WIDTH)	H (HEIGHT OF SOLID GROUTED LINTEL)	W (SOLID GROUTED JAMB WIDTH
UP TO 4'-0	1'-4	8"
4'-1 TO 6'-0	2'-0	1'-4

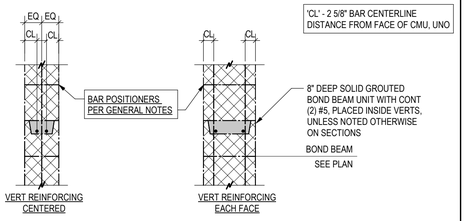


TYPICAL CMU WALL OPENING

5 S-501 TYP CMU WALL INTERSECTIONS

3/4" = 1'-0"

TYPICAL CMU WALL/REINFORCING ASSEMBLIES



STRUCTURAL MASONRY WALL SCHEDULE

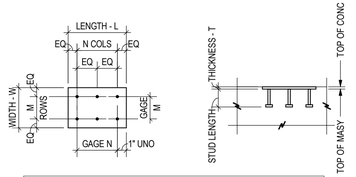
MASONRY WALL SCHEDULE		
MARK	WIDTH	VERT REINF
MW8	7 5/8"	#5 @ 32"

1. VERTICAL REINFORCING INSTALLED FULL HEIGHT IN SOLID GROUTED VERTICAL CELLS AT SPACING SHOWN AND LOCATED AT OPENING JAMBS, VERTICAL CONTROL JOINTS (AS IN 6/S-501 & 7/S-501), WALL ENDS, CORNERS, AND INTERSECTIONS
2. STANDARD LADDER-TYPE CONTINUOUS HORIZONTAL JOINT REINFORCING VERTICALLY @ 16" UNLESS OTHERWISE NOTED
3. ADD BOND BEAMS AT ALL FLOOR AND ROOF ELEVATIONS AS NOTED IN SECTIONS AND AT ALL TOP OF WALL LOCATIONS, MAXIMUM VERTICAL SPACING = 8'-0.

MASONRY LAP SPLICE LENGTH SCHEDULE

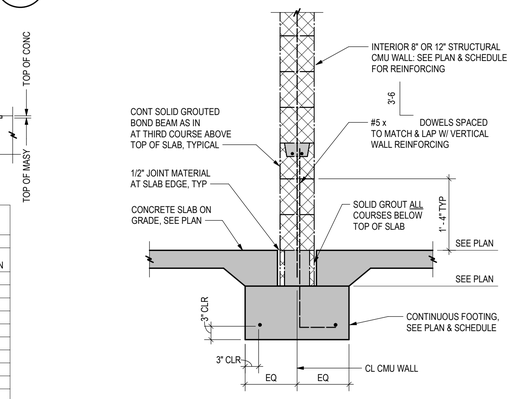
	REINFORCING AT FACE		REINFORCING CENTERED			
BAR SIZE	CL	8" CMU & GREATER	6" CMU	8" CMU	10" CMU	12" CMU & GREATER
#3	2 5/8"	18"	18"	18"	18"	18"
#4	2 5/8"	24"	24"	24"	24"	24"
#5	2 5/8"	30"	30"	30"	30"	30"
#6	2 5/8"	54"	0" NP	36"	36"	36"
#7	2 5/8"	63"	0" NP	46"	42"	42"
#8	2 5/8"	0" CR	0" NP	0" NP	54"	48"
#9	2 5/8"	0" CR	0" NP	0" NP	0" CR	57"

NOTES:
1. REINFORCING STEEL fy = 60,000 PSI
2. MASONRY f'm = 2,000 PSI
3. DISTANCE FROM FACE OF MASONRY TO BAR CENTERLINE NOTED AS 'CL'
4. MECHANICAL COUPLER REQUIRED WHERE NOTED 'C' 'CR'
5. BAR SIZE NOT PERMITTED WHERE NOTED 'C' 'NP'
6. USE MECHANICAL COUPLER WHERE REQUIRED LAP LENGTH EXCEEDS GROUT LIFT HEIGHT
7. FOR EPOXY COATED BARS MULTIPLY TABLE VALUES BY 1.5
8. OPEN END MASONRY UNITS MAY BE USED AT VERTICAL REINFORCING LOCATIONS
9. WALL BRACING DESIGN AND IMPLEMENTATION SHALL BE BY CONTRACTOR



x-0228 & 0839-BEARING PLATE SCHEDULE

BEARING PLATE		HEADED ANCHOR STUDS			
TYPE	T x W x L	HAS	M ROWS	N COLS	GAGE MxN
BP-2	PL1/2"x7"x7"	(2)-1/2"Øx6"	1	2	0" x 5"
BP-3	PL1/2"x5"x1'-0"	(2)-1/2"Øx6"	1	2	0" x 10"
BP-4	PL5/8"x5"x8"	(2)-1/2"Øx6"	1	2	0" x 6"
BP-5	PL1/2"x8 1/2"x1'-0"	(2)-1/2"Øx6"	1	2	0" x 10"
BP-6	PL3/8"x6"x1'-0"	(2)-1/2"Øx6"	1	2	0" x 10"
BP-1		(2)-	1		
BP-2		(2)-	1		
BP-3		(2)-	1		
BP-4		(2)-	1		
BP-5		(2)-	1		
BP-6		(2)-	1		



1 S-501 TYP CMU WALL/REINF ASSEMEBLIES & SCHEDULE

3/4" = 1'-0"

0 6" 1' 2' 3'

2 S-501 TYP CMU MASONRY LAP SPLICE LENGTH & SCHEDULE

NO SCALE

3 S-501 TYP CMU BEARING PLATE

3/4" = 1'-0"

4 S-501 TYP CMU FOUNDATION AT INTERIOR LOAD BEARING WALLS

3/4" = 1'-0"

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Key Map

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TYPICAL CMU DETAILS

S-501

SITE PLAN SUBMITTAL
Page 247 of 311

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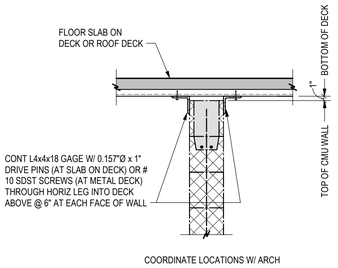
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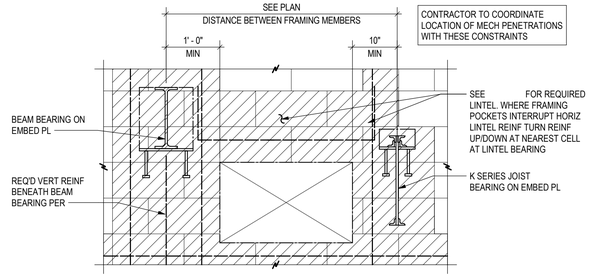
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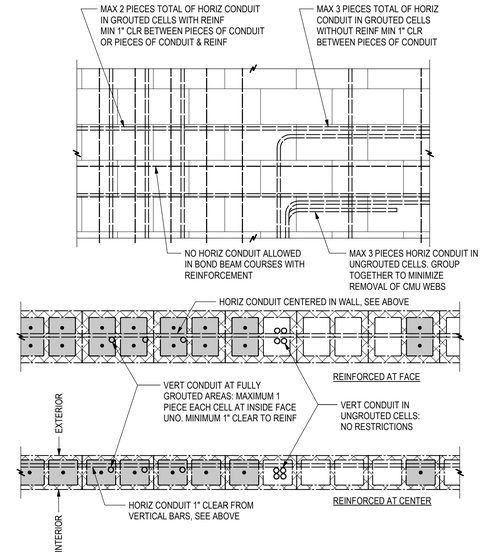
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COORDINATE LOCATIONS W/ ARCH

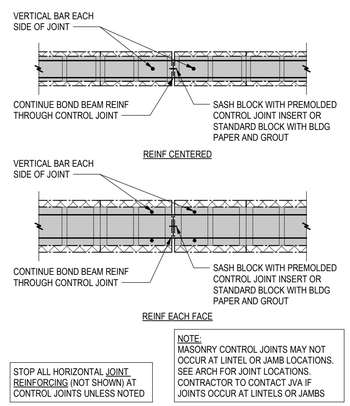


(APPLICABLE FOR PENETRATIONS 5'-0 WIDE OR LESS)



5 S-502 TYP CMU FULL HEIGHT PARTITION BRACING DETAIL

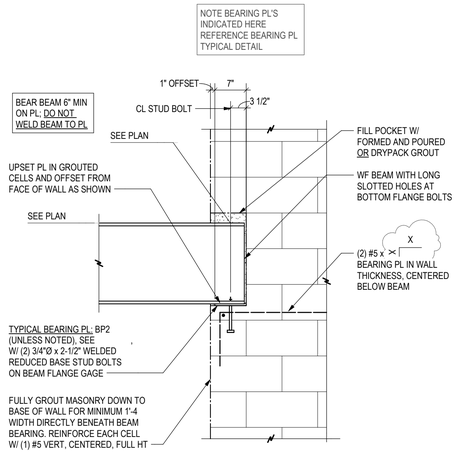
3/4" = 1'-0"



SEE ARCH DRAWINGS FOR LOCATIONS

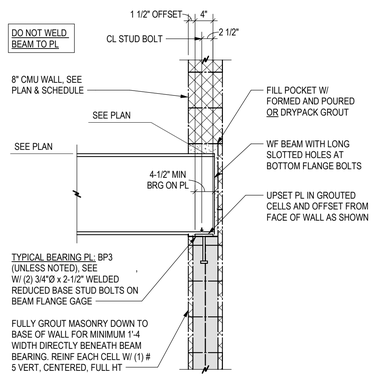
6 S-502 TYP CMU SPACING CONSTRAINTS OF MECH PENETRATIONS TO ROOF/FLOOR BEARING ON STRUCTURE

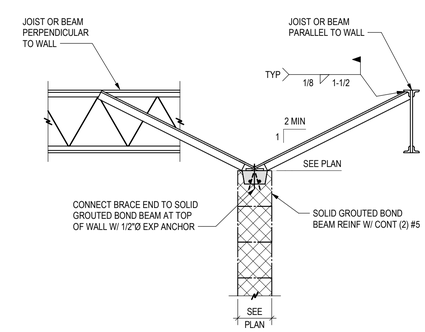
3/4" = 1'-0"



7 S-502 TYP CMU CONDUIT PLACEMENT IN WALL

3/4" = 1'-0"





BRACE ALL MASONRY PARTITIONS WHEN HORIZONTAL LENGTH OF WALL EXCEEDS 12'-0 BETWEEN CORNERS AND CROSS WALLS. BRACE AT FREESTANDING ENDS OF WALLS.

1 S-502 TYP CMU VERT CONTROL JOINT IN WALL

3/4" = 1'-0"

0 6" 1' 2' 3'

2 S-502 TYP CMU BEAM BEARING ON PARALLEL WALL END

3/4" = 1'-0"

3 S-502 TYP CMU BEAM BRG ON PERPENDICULAR WALL

3/4" = 1'-0"

4 S-502 TYP CMU NON-LOAD BEARING PARTITION BRACING

3/4" = 1'-0"

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Checked By:	BAR

Key Map

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TYPICAL CMU DETAILS

S-502

SITE PLAN SUBMITTAL

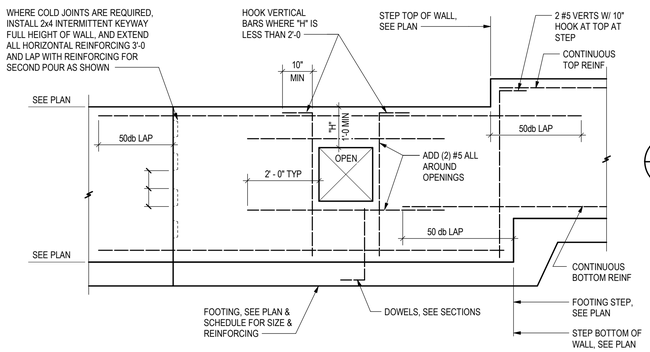
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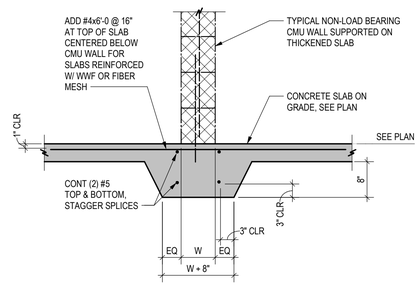


8 S-503 TYP CONC REINF AT STEPS & OPENINGS

1/2" = 1'-0"

0 6" 1' 2' 4'

1. REINFORCE WITH #4 VERTICALS CENTERED IN WALL IN GROUTED CELLS AT CORNERS, JAMBS, WALL INTERSECTIONS, AND @ 6'-0 MAXIMUM
2. DOWEL WALL TO SLAB WITH #4 x 2'-4 DOWELS IN EPOXY FILLED HOLES (4" EMBED) LOCATE DOWELS TO MATCH VERTICAL BAR SPACING IN WALL
3. PROVIDE MASONRY LINTEL OVER WALL OPENINGS PER
4. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCING @ 16"
5. BRACE TOP OF PARTITIONS PER &
6. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF PARTITIONS AND CONTROL JOINTS



TYPICAL 6" OR 8" NON-LOAD BEARING CMU WALL (MAXIMUM HEIGHT = 15'-0")

5 S-503 TYP CONC NON-LOAD BEARING CMU PARTITIONS

3/4" = 1'-0"

WALL FOOTING SCHEDULE				
MARK	WIDTH	THICKNESS	REINFORCING	COMMENTS
F16	1' - 4"	1' - 0"		
F24	2' - 0"	1' - 0"		

FOOTING NOTES:
1. CENTER FOOTINGS UNDER STEMWALLS, PILASTERS, & COLUMNS, TYPICAL UNLESS NOTED OTHERWISE
2. FOOTING WIDTHS SHALL NOT VARY IN THE FIELD FROM SIZE NOTED; OVERSIZE FOOTINGS ARE NOT ALLOWED
3. LAPPED BOARD FORMING NOT ALLOWED
4. TRENCHFORMING NOT ALLOWED

12 S-503 CONTINUOUS WALL FOOTING SCHEDULE

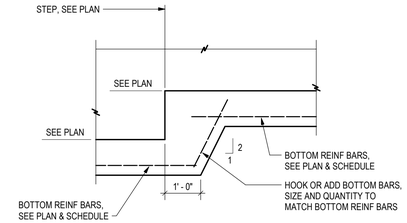
NO SCALE

ISOLATED FOOTING SCHEDULE					
MARK	WIDTH	LENGTH	THICKNESS	REINFORCING	COMMENTS
F4.0	4' - 0"	4' - 0"	1' - 0"		
F5.0	5' - 0"	5' - 0"	1' - 2"		
F6.0	6' - 0"	6' - 0"	1' - 4"		
F8.0	8' - 0"	8' - 0"	1' - 8"		

FOOTING NOTES:
1. * MATCH BOTTOM REINFORCING AT TOP OF FOOTING PER XXXX INDICATED THUS ' ► ' ON PLAN
2. ** SEE XXXX FOR INCREASE FOOTING THICKNESS REQUIREMENT WHEN FOOTING SUPPORTS BRACED BAY COLUMN INDICATES THUS ► ' ON PLAN
3. CENTER FOOTINGS UNDER STEMWALLS, PILASTERS, & COLUMNS, TYPICAL UNLESS NOTED OTHERWISE
4. FOOTING WIDTHS SHALL NOT VARY IN THE FIELD FROM SIZE NOTED; OVERSIZE FOOTINGS ARE NOT ALLOWED
5. LAPPED BOARD FORMING NOT ALLOWED
6. TRENCHFORMING NOT ALLOWED

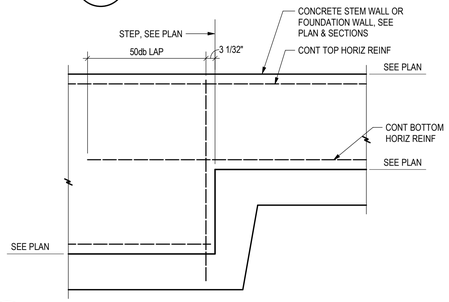
13 S-503 ISOLATED FOOTING SCHEDULE

NO SCALE



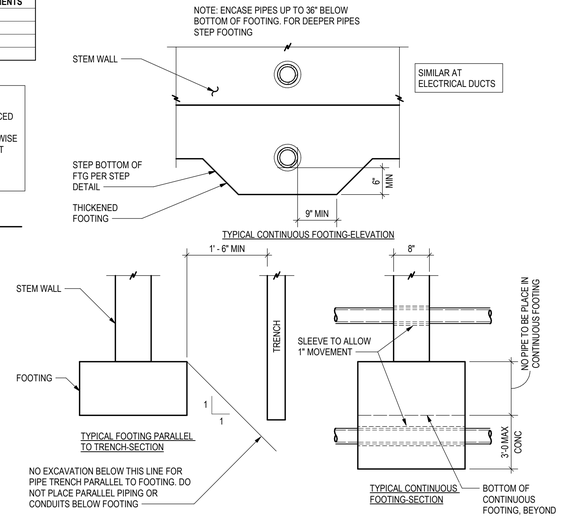
9 S-503 TYP CONC FOOTING STEP

1/2" = 1'-0"



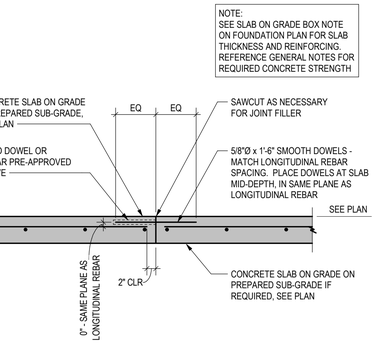
10 S-503 TYP CONC STACKED STEP IN STEM WALL

1/2" = 1'-0"



11 S-503 TYP CONC PIPING AT FOOTING

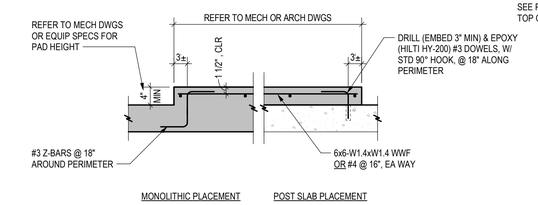
3/4" = 1'-0"



1 S-503 TYP JOINT AT INTERIOR SLAB ON GRADE

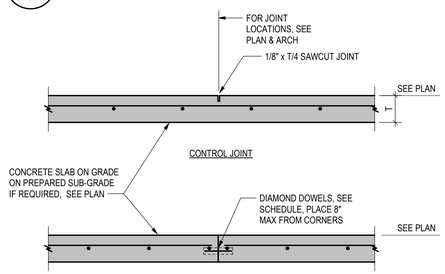
3/4" = 1'-0"

0 6" 1' 2' 3'



6 S-503 DETAIL

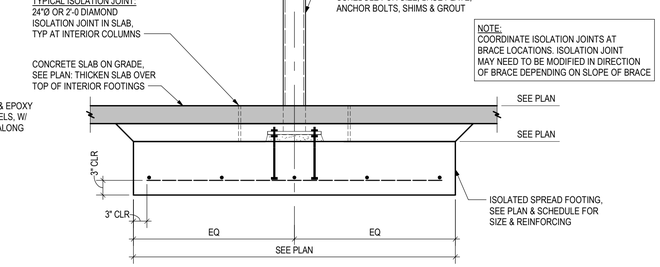
3/4" = 1'-0"



DOWEL SIZE & SPACING AT CONSTRUCTION JOINTS		
SLAB DEPTH (INCHES)	DOWEL DIMENSIONS (INCHES)	DOWEL SPACING (INCHES)
5 TO 6	1/4 x 4-1/2 x 4-1/2	18
7 TO 8	3/8 x 4-1/2 x 4-1/2	18
9 TO 11	3/4 x 4-1/2 x 4-1/2	20

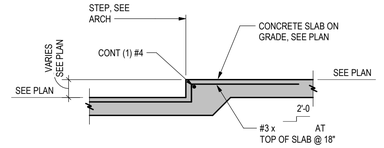
2 S-503 SCHEDULE

3/4" = 1'-0"



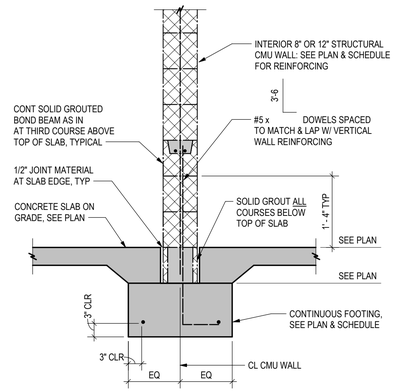
7 S-503 TYP CONC ISOLATED SPREAD FOOTING AT COL

3/4" = 1'-0"



3 S-503 DETAIL

3/4" = 1'-0"



4 S-503 DETAIL

3/4" = 1'-0"

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Δ	DATE	DESCRIPTION

Project Number:	223072.00
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Drawn By:	LB
Checked By:	BAR

Key Map

Drawing
TYPICAL FOUNDATION DETAILS

S-503

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1 2 3 4 5

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THE SQUARES ABOVE ARE COLOR WITH BLACK AND WHITE LETTER, IF PRINTED CORRECTLY

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1 2 3 4 5

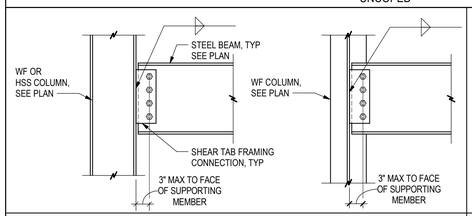
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STL BEAM SHEAR TAB CONNECTION SCHEDULE - 3/4"Ø BOLTS (LRFD)

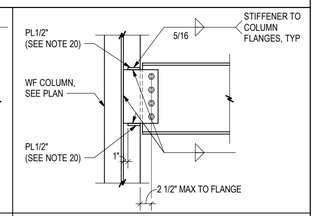
BEAM SIZE	# OF 3/4"Ø BOLT ROWS	SHEAR TAB THICKNESS x LENGTH, SEE NOTE #14	FILLET WELD SIZE	CONNECTION CAPACITY -ØRn (KIPS)				REMARKS
				UNCOPED	SINGLE COPE 1	SINGLE COPE 2	DOUBLE COPE	
W8	2	3/8"x6"	1/4"	24.8	12.7	N/A	7.1	
W10	2	3/8"x6"	1/4"	24.8	20.1	N/A	7.9	
W12*	3	3/8"x9"	1/4"	43.4	32.6	15.5	18.7	
W14	3	3/8"x9"	1/4"	43.4	43.4	29.2	25.0	
W16**	4	3/8"x12"	1/4"	62.5	62.5	43.4	41.5	
W18	4	3/8"x12"	1/4"	62.5	62.5	62.5	62.5	
W21	5	3/8"x15"	1/4"	81.3	81.3	81.3	81.3	
W24	6	3/8"x18"	1/4"	100.0	100.0	100.0	100.0	
W27	7	3/8"x21"	1/4"	118.0	118.0	118.0	118.0	
W30	8	3/8"x24"	1/4"	137.0	137.0	137.0	137.0	
W33	9	3/8"x27"	1/4"	155.0	155.0	155.0	155.0	
W36	10	3/8"x30"	1/4"	173.0	173.0	173.0	173.0	
W40	11	3/8"x33"	1/4"	190.0	190.0	190.0	190.0.	

1. DESIGN BASIS OF SCHEDULE BASED ON AISC 14TH EDITION (ASD).
2. MINIMUM YIELD STRESS FY: PLATES - 36 KSI, W-SHAPES - 50 KSI, HSS SHAPES - 46 KSI.
3. MINIMUM TENSILE STRESS, FU: PLATES - 58 KSI, W-SHAPES - 65 KSI, HSS SHAPES - 58 KSI.
4. MINIMUM EDGE DISTANCE: VERTICAL EDGE DISTANCE Lev = 1 1/2", HORIZONTAL EDGE DISTANCE Leh = 1 1/2"
5. MINIMUM HSS WALL THICKNESS = 1/4"
6. ALL WELDS ARE TO USE E70XX ELECTRODES.
7. ALL BOLTS ARE TO BE A325-N BEARING TYPE BOLTS, EXCEPT CONNECTIONS DESIGNATED AS 'SC' ON PLANS ARE TO BE CLASS 'A' SLIP-CRITICAL JOINTS WITH F1852 TENSION-CONTROL BOLTS. NOTE THAT VALUES LISTED DO NOT APPLY TO THE SLIP-CRITICAL CONNECTIONS.
8. A325-N BOLTS ARE TO BE FULLY THREADED BEARING TYPE CONNECTIONS AND ARE TO BE INSTALLED TO A 'SNUG-TIGHT' CONDITION AS DEFINED IN THE AISC MANUAL OF STEEL CONSTRUCTION, UNLESS NOTED OTHERWISE.
9. STANDARD HOLES ARE TO BE PROVIDED IN SUPPORTED WEB.
10. SHORT-SLOTTED HOLES TO BE PROVIDED IN SHEAR TAB PLATES, UNLESS NOTED OTHERWISE.
11. BOLT SPACING = 3".
12. BEAM WEB DEPTH FOR DOUBLE COPE CONDITION MUST BE GREATER THAN OR EQUAL TO SHEAR TAB PLATE LENGTH. COPE BEAM ACCORDINGLY. (2" MAXIMUM AT W8-W24 AND 3" MAXIMUM AT W24-W40).
13. DETAILER TO INFORM ENGINEER OF COPE CONDITIONS NOT COMPLYING WITH CONDITIONS SHOWN IN DETAILS BELOW.
14. FOR SKEWED SHEAR TAB PLATE CONNECTIONS. WELD DETAILS ARE TO FOLLOW TABLE 10-14C, AISC 14TH EDITION.
15. SEE DETAILS BELOW FOR ADDITIONAL DETAILING REQUIREMENTS. THE CONNECTION ENGINEER IS TO BE NOTIFIED OF ANY FRAMING CONDITION WHICH DOES NOT MEET THE CRITERIA PROVIDED IN THIS SCHEDULE.
16. * PROVIDE 2 BOLTS WITH 6" LONG SHEAR TAB PLATE FOR W12 BEAM AT 'SINGLE COPE 2" CONDITION ONLY.
17. ** PROVIDE 3 BOLTS WITH 9" LONG SHEAR TAB PLATE FOR W16 BEAM AT 'SINGLE COPE 2" CONDITION ONLY.
18. N/A INDICATES CONDITION WHERE CONNECTION FRAMING TYPES OUTLINED IN THIS SCHEDULE TO NOT APPLY.
19. FOR ANY COLUMNS LARGER THAN A W18, THE ALTERNATE BEAM TO COLUMN WEB EXTENDED PLATE CONNECTION IS NOT APPLICABLE.
20. AT THE FOLLOWING COLUMNS PROVIDE 3/4" STIFFENER PLATES TOP AND BOTTOM AT THE ALTERNATE BEAM TO COLUMN WEB EXTENDED PLATE CONNECTION: W8X13, W8X15, W10X26, W10X30, W14X43, W14X48, W14X53, W16X57, W18X65 AND W18X71.

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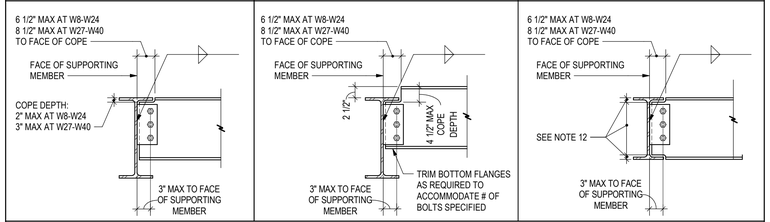


TYPICAL BEAM-TO-COLUMN CONNECTIONS



ALTERNATE BEAM-TO-COLUMN WEB EXTENDED PLATE CONNECTION (SEE NOTE 19 & 20)

SINGLE COPE 1 | SINGLE COPE 2 | DOUBLE COPE

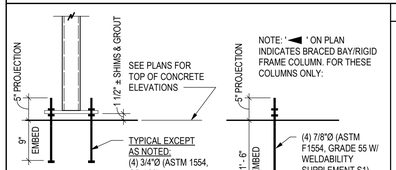


TYPICAL BEAM-TO-BEAM CONNECTIONS

ENGINEER NOTES:
1. *REVIEW AND IMPLEMENT PRE-ESTABLISHED SUPPORTING CALCULATION PACKAGE VERIFYING CONNECTION CAPACITIES PRESENTED*

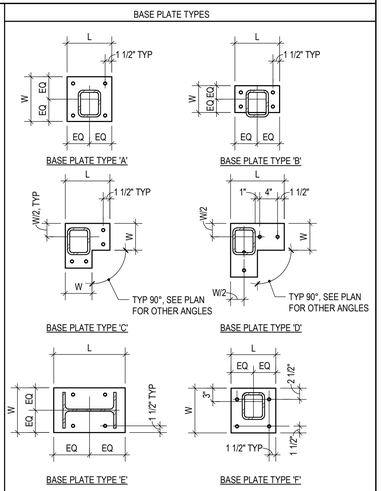
4 S-504 SCHEDULE
3/4" = 1'-0"

BASE PLATE SCHEDULE



BASE PLATE SCHEDULE					
MARK	TYPE	LENGTH	WIDTH	THICKNESS	COMMENTS
BPA	A	1' - 0"	1' - 0"	3/4"	
BPB	B	1' - 0"	10"	3/4"	
BPC	C	1' - 2"	1' - 0"	1"	

BASE PLATE TYPES



1 S-504 TYP STL BASE PLATE TYPES & SCHEDULE
NO SCALE

COLUMN SCHEDULE

TYPICAL COLUMN MARKS:

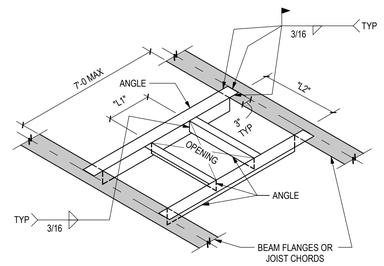
CXX OR CXXX

HSS COLUMN WALL THICKNESS IN 16ths OF AN INCH (EXAMPLE: '5' INDICATES 5/16" WALL THICKNESS, '10' INDICATES 5/8", ETC...)

SQUARE OR RECTANGULAR HSS COLUMN SIZE (EXAMPLE: '6' INDICATES HSS6x6, '75' INDICATES HSS7x5)

STEEL COLUMN SCHEDULE	
MARK	STEEL COLUMN SCHEDULE
C44	HSS4x4x1/4
C54	HSS5x5x1/4

2 S-504 TYP STL COL MARKS & SCHEDULE
NO SCALE



"L1" OR "L2 (USE LARGER VALUE)	ANGLE SIZES	REMARKS
UP TO 1'-0	NONE REQUIRED	
UP TO 3'-0	L3-1/2x3-1/2x1/4	
UP TO 5'-0	L5x3x1/4 (LLV)	
UP TO 7'-0	L6x4x5/16 (LLV)	

- USE ABOVE FRAMING AT ALL FLOOR & ROOF OPENINGS WHERE EITHER "L1" OR "L2" EXCEEDS 1'-0 , UNLESS NOTED OTHERWISE
- VERIFY WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS

3 S-504 TYP STL AT FLOOR/ROOF OPENING
3/4" = 1'-0"

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Δ	DATE	DESCRIPTION

Project Number:	223072.00
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Checked By:	BAR

Key Map

Drawing
TYPICAL STEEL DETAILS

S-504

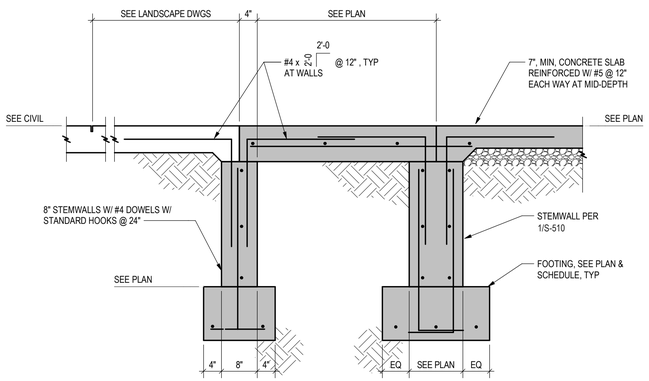
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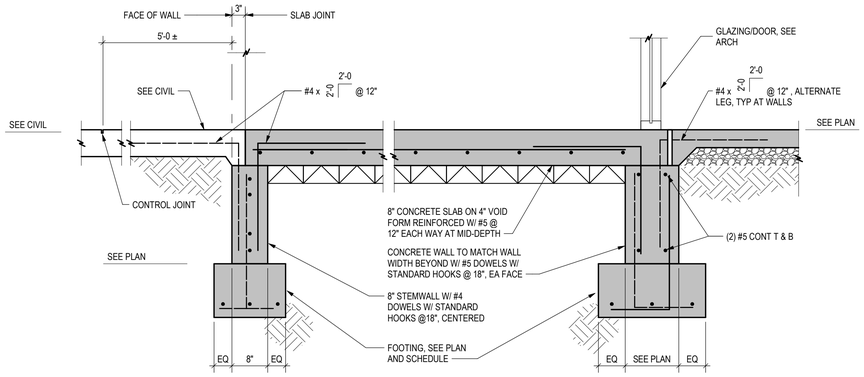
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1 S-510 SECTION
3/4" = 1'-0"



2 S-510 SECTION
3/4" = 1'-0"



3 S-510 SECTION
3/4" = 1'-0"

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p. 303.444.1951

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Key Map

Drawing

FOUNDATION DETAILS

S-510

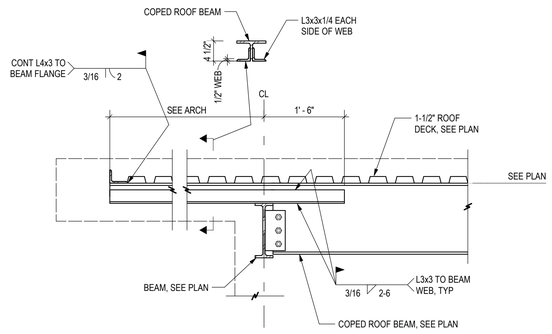
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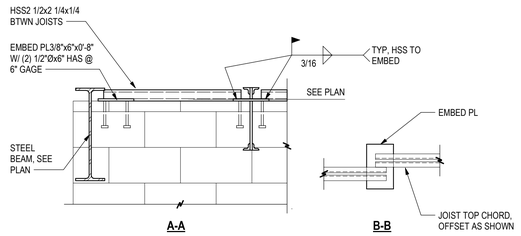
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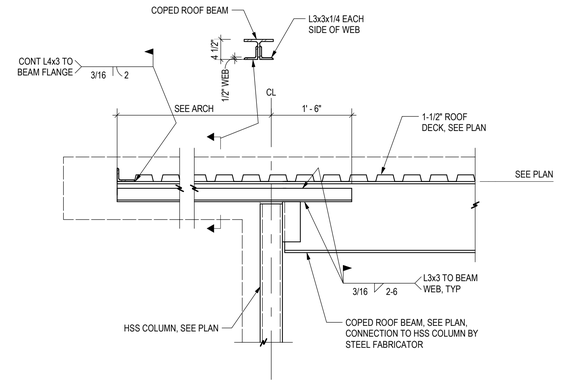
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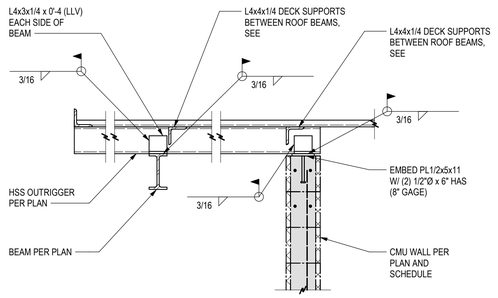


7 S-511 SECTION
3/4" = 1'-0"

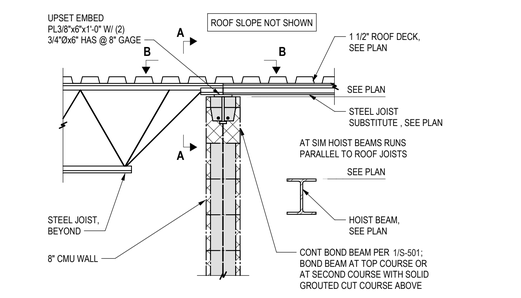




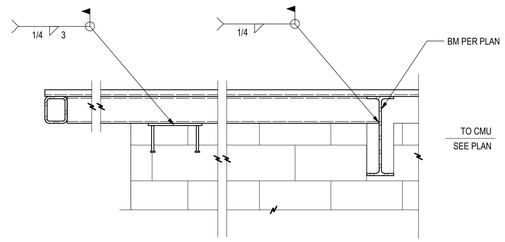
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3/4" = 1'-0"



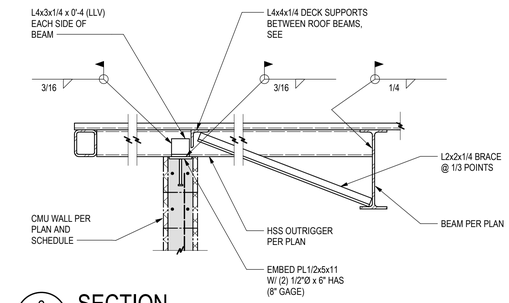
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3/4" = 1'-0"



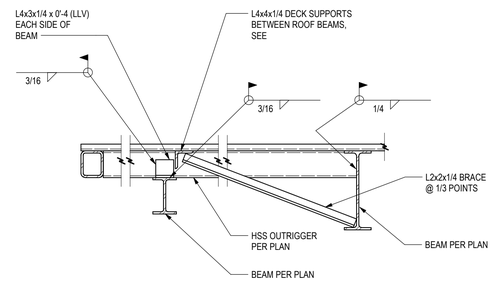
5 S-511 SECTION
3/4" = 1'-0"



1 S-511 OUTRIGGER AT CMU WALL
3/4" = 1'-0"



2 S-511 SECTION
3/4" = 1'-0"



3 S-511 SECTION
3/4" = 1'-0"

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1675 Larimer Street, #520
Denver, CO 80202
p. 303.444.1951

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p. 303.278.7297

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Drawn By:	Author
Checked By:	Checker

Key Map

Drawing
ROOF DETAILS

S-511

SITE PLAN SUBMITTAL

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THE LINE SHOWN ABOVE IS ENGINEERING SCALE 1" = 10'0 SHEETS ORIGINAL PAGE SIZE

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FINISH SYMBOL LEGEND

XXX WALL FINISH SYMBOL

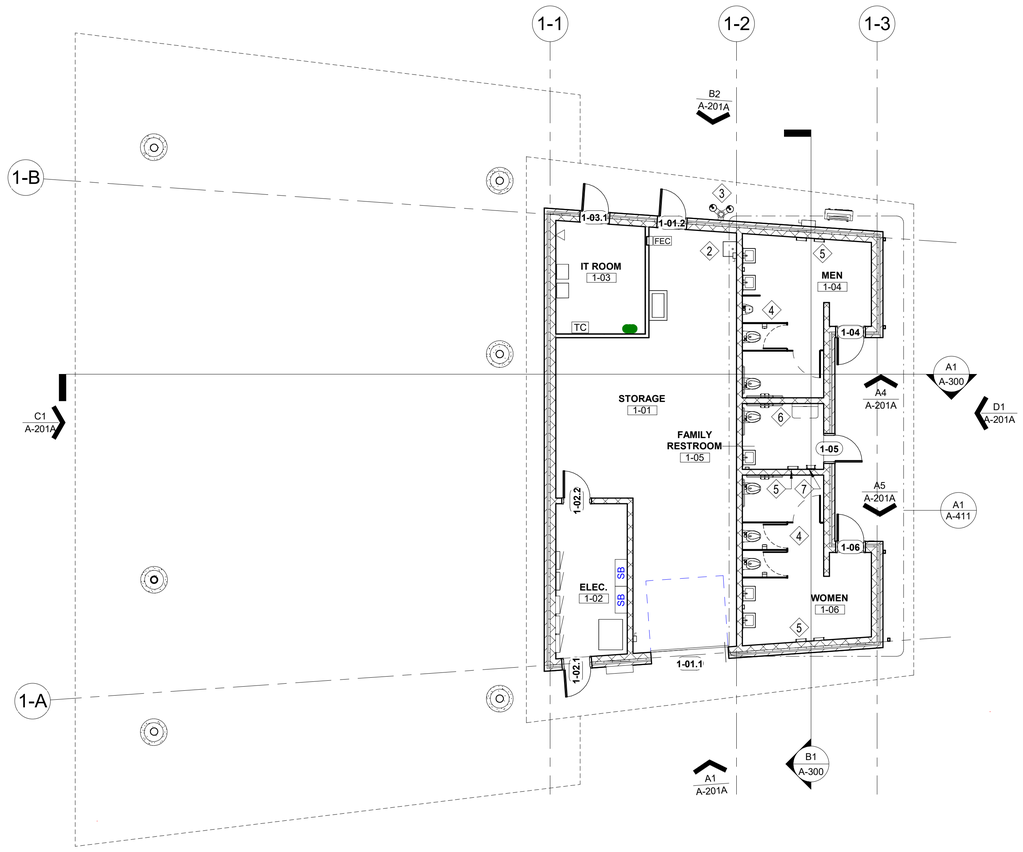
SHEET NOTE SYMBOL

FLOOR PLAN GENERAL NOTES

1. REFER TO FINISH PLANS FOR INTERIOR ELEVATION CALLOUTS.
2. PROVIDE PAINTED ACCESS PANELS IN WALLS & GYPSUM BOARD CEILINGS AT CONCEALED ITEMS (VALVES, CONTROLS, SWITCHES, AND ANY OTHER ITEM THAT REQUIRES ACCESS. COORDINATE LOCATION WITH ARCHITECT PRIOR TO INSTALLATION.
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4. VERIFY & COORDINATE ALL REQUIREMENTS FOR OWNER FURNISHED ITEMS PRIOR TO PERFORMANCE OF WORK.
5. REFER TO A-511 FOR ASSEMBLY TYPE TAGS SHOWN ON PLANS CORRESPONDING WITH EXTERIOR WALL AND ROOF ASSEMBLIES.

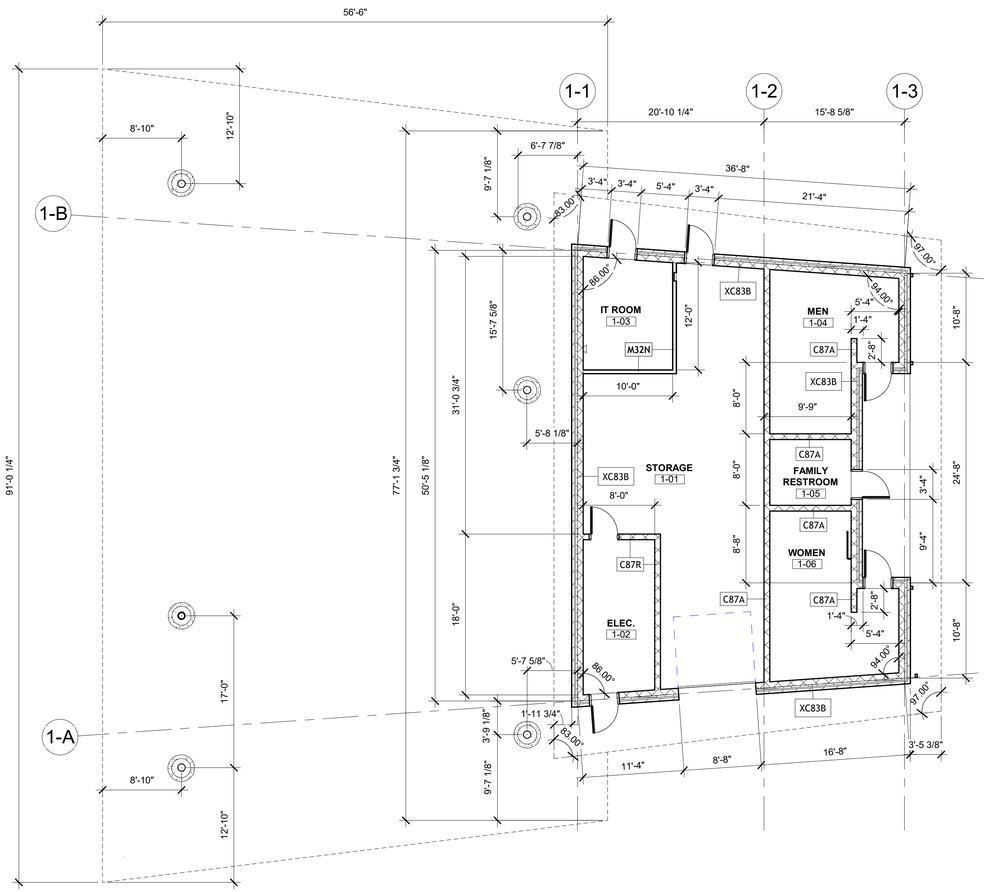
SHEET NOTES - CONSTRUCTION PLANS

NOTE #	NOTE
1	NOT USED
2	MOP SINK
3	HI-LO DRINKING FOUNTAIN AND WATER BOTTLE FILLER
4	TOILET PARTITIONS
5	ELECTRIC HAND DRYER
6	CHILD CHANGING TABLE
7	CHILD PROTECTION SEAT
8	MOTORIZED ADULT CHANGING TABLE



A1 COMMUNITY HUB BUILDING - FLOOR PLAN - PHASE 1

A-111A 1/8" = 1'-0"



A3 COMMUNITY HUB BUILDING - DIMENSION FLOOR PLAN - PHASE 1

A-111A 1/8" = 1'-0"

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Key Map

Drawing

COMMUNITY HUB - FLOOR PLANS

A-111A

SITE PLAN SUBMITTAL
Page 253 of 311

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FINISH SYMBOL LEGEND

XXX WALL FINISH SYMBOL

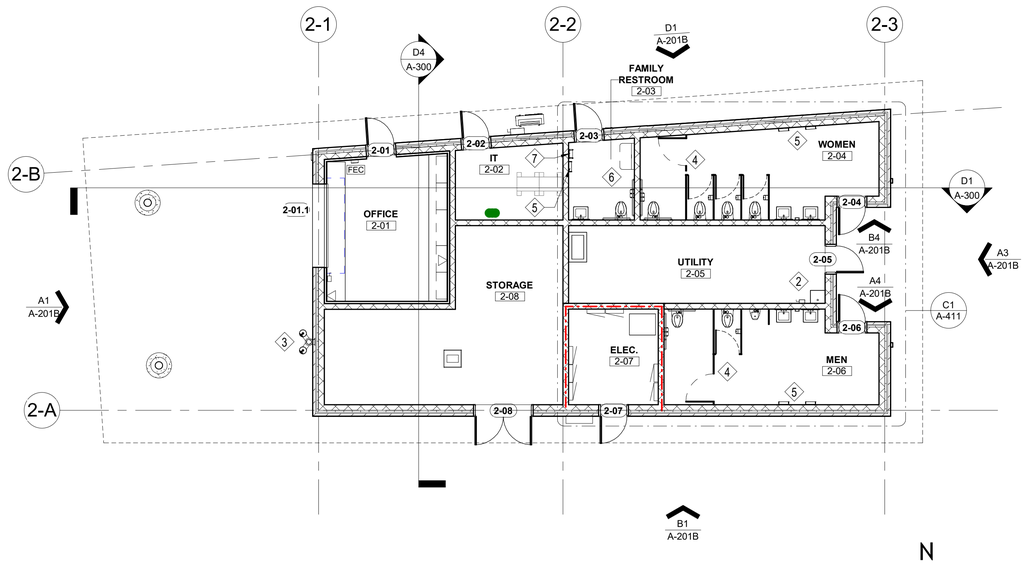
SHEET NOTE SYMBOL

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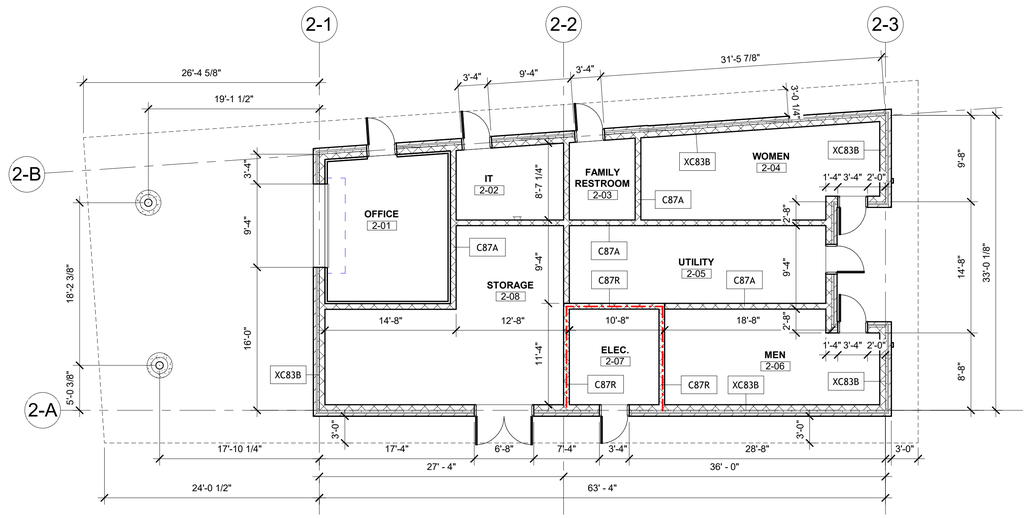
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A1 HEADQUARTERS & RESTROOM BUILDING - FLOOR PLAN
A-111B 1/8" = 1'-0"

N



A3 HEADQUARTERS & RESTROOM BUILDING - DIMENSION FLOOR PLAN
A-111B 1/8" = 1'-0"

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Denver, CO 80202
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1675 Larimer Street, #520
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Key Map

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HEADQUARTERS & RESTROOM - FLOOR PLANS

A-111B

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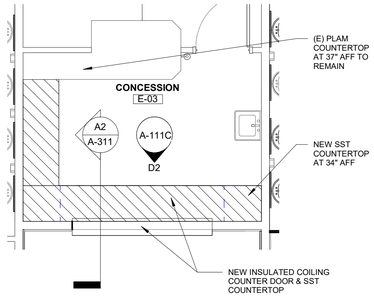
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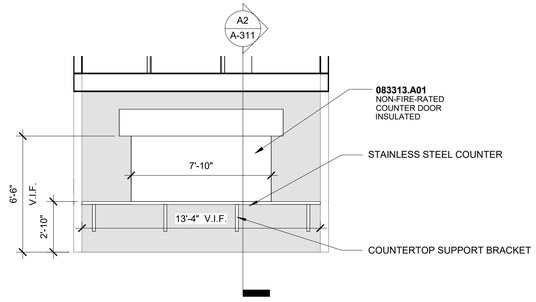
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D1 EXISTING CONCESSIONS BUILDING - FLOOR PLAN

A-111C A-111C 1/4" = 1'-0"



D2 EXISTING CONCESSIONS - SOUTH ELEVATION

A-111C A-111C 1/4" = 1'-0"

SHEET NOTES - DEMO FLOOR PLANS

NOTE #	NOTE
1	REMOVE (E) COILING DOOR, TRACKS, ASSOCIATED HARDWARE AND COUNTERTOP
2	REMOVE PORTION OF WALL AT SILL FOR NEW ADA COMPLIANT OPENING
4	REMOVE PORTION OF EXISTING COUNTER
5	REMOVE ALL (E) FLUSH VALVES
6	PREP FLOOR FOR NEW EPOXY FLOOING W/ INTEGRAL COVE BASE
7	REMOVE (E) TOILET PARTITIONS
8	REMOVE (E) GRAB BARS & SLAVAGE FOR REINSTALL
9	REMOVE (E) COUNTER

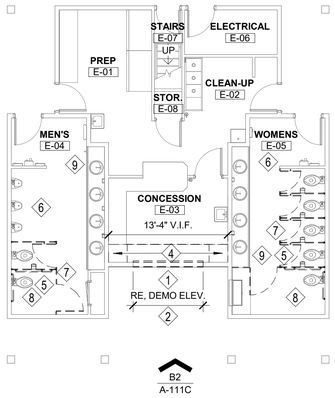
FINISH SYMBOL LEGEND

XXX WALL FINISH SYMBOL

SHEET NOTE SYMBOL

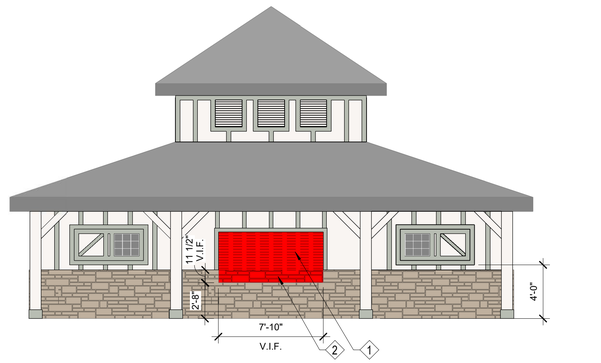
DEMOLITION GENERAL NOTES

1. GENERAL CONTRACTOR TO COORDINATE SCHEDULING ALL WORK WITH OWNER.
2. WORK LABELED (NIC), OR OTHERWISE NOT NOTED IS NOT IN CONTRACT FOR ANY ARCHITECTURAL IMPROVEMENTS.
3. THE GENERAL CONTRACTOR & THEIR SUBCONTRACTORS SHALL VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS WITH CONDITIONS SHOWN IN THE CONTRACT DOCUMENTS AND SHALL REPORT ANY DEVIATIONS, DISCREPANCIES AND/OR CONFLICTS TO ARCHITECT.
4. THE GENERAL CONTRACTOR & SUBCONTRACTOR SHALL TAKE EXTREME CARE DURING DEMOLITION NOT TO DAMAGE OR DISTURB ANY EXISTING CONDITIONS THAT ARE TO REMAIN. GENERAL CONTRACTOR OR SUBCONTRACTOR SHALL REPAIR ANY DAMAGE OR DISTURBANCE TO EXISTING CONDITIONS AT NO COST TO THE OWNER.
5. PROVIDE PROTECTION FOR FLOORS, WALLS, & CEILING AT ALL EXISTING CONDITIONS TO REMAIN, INCLUDING TRAFFIC AREA FOR DEMOLITION REMOVAL IN COMMON BUILDING AREAS & FREIGHT ELEVATORS. DAMAGED FLOORS WILL BE REPLACED @ NO COST TO OWNER.
6. REMOVE ALL WALLS, DOORS (AND OTHER ITEMS) SHOWN DASHED - FIELD VERIFY CONSTRUCTION OF ALL WALLS TO BE REMOVED - PROVIDE SHORING AND BRACING AS REQUIRED.
7. REFER TO SPECIFICATIONS REGARDING; EXISTING CONDITIONS, CUTTING AND PATCHING AND SELECTIVE DEMOLITION REQUIREMENTS THAT APPLY TO ALL WORK KEY NOTES DESCRIBED ON THIS SHEET.
8. COORDINATE W/ BUILDING OWNER DURING DEMOLITION TO DETERMINE WHETHER EXISTING FIRE & SMOKE DETECTION SYSTEMS ARE TO BE BAGGED, PROTECTED & REMAIN IN OPERATION OR TO BE TAKEN OFF LINE.
9. AREAS OF DEMOLITION SHALL BE FREE OF (FURNITURE AND MOBILE EQUIPMENT AND ACCESSORIES) PRIOR TO START OF WORK.



B1 EXISTING CONCESSIONS BUILDING - DEMO FLOOR PLAN

A-111C 1/8" = 1'-0"

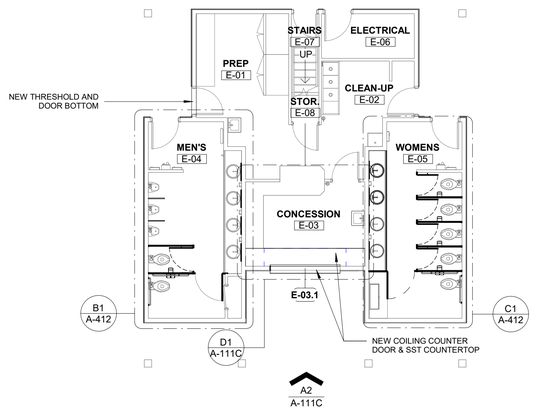


B2 EXISTING CONCESSIONS BUILDING - DEMOLITION ELEVATION

A-111C A-111C 3/16" = 1'-0"

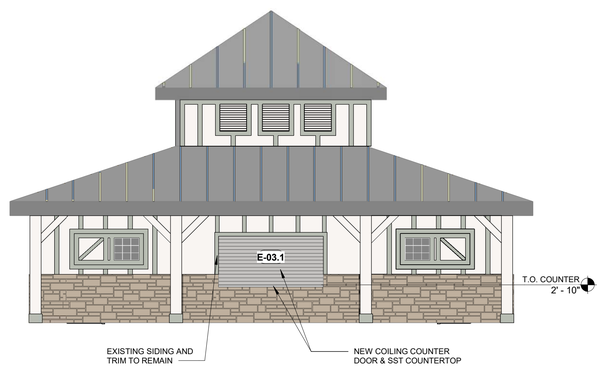
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A1 EXISTING CONCESSIONS BUILDING - FLOOR PLAN

A-111C 1/8" = 1'-0"



A2 EXISTING CONCESSIONS BUILDING - ELEVATION

A-111C A-111C 3/16" = 1'-0"

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Denver, CO 80202
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Key Map

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EXISTING CONCESSIONS BUILDING - FIRST FLOOR PLAN

A-111C

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