TRAILS AT CROWFOOT
PHASE I & II
PRELIMINARY DRAINAGE REPORT

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*This report for the preliminary design of **Trails at Crowfoot** was prepared by me or under my direct supervision in accordance with the provisions of the Town of Parker Storm Drainage and Enviromental Criteria Manual. I understand that the Town of Parker and its designated town authority do not and will not assume liability for drainage facilities designed by other.*

_____
Signature

_____
Colorado P.E. License No.

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Seal and Date

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The following narrative and supporting calculations provide the drainage design associated with the *Trails at Crowfoot Phase I & II Preliminary Drainage Report*. The report primarily addresses drainage concepts related to Pond and storm system infrastructure sizing.

I. INTRODUCTION

1. Location

The Trails at Crowfoot Development is located within the Town of Parker, Douglas County, Colorado. The boundary of the development spans several sections, all of which are located within Township 7 South, Range 66 west of the 5th principal meridian.

More specifically, the southwest quarter, and majority of the north half, of Section 4, the south half, and majority of the north half, of Section 5, the east half of Section 6, a small portion along the east edge of Section 8, the north half, and the north half of the southwest quarter, of Section 9.

On a broader scale, the site is bounded by Rueter-Hess Reservoir to the northwest, Stroh Road to the north, Lemon Gulch Way to the south and east, and Cherry Creek to the east. More easily put, a majority of the site itself is bounded by the city limits of Parker.

2. Proposed Development

The proposed site spans roughly 400 acres along the southwest edge of the Parker city limits. The topography of the site generally slopes from southwest to northeast and west to east for the Lemon Gulch Basin, with many ravines, valleys, rolling hills, and channels conveying stormwater runoff to the northeast, where it will ultimately converge with Cherry Creek. The site itself is situated on five, previously studied, regional sub catchment basins, Oak Gulch, West Stroh, North Crowfoot Valley, Scott Gulch, and Lemon Gulch. Oak Gulch and Lemon Gulch drainage basins encompass the majority of the site.

Little exists in terms of drainage or utility infrastructure in the area. Drainage wise, there are several CMP culvert crossings along North Crowfoot Valley Road to the east, Lemon Gulch Way to the west, and Stroh Road to the North. Utility wise, raw water and potable water lines run down Stroh Road along the northern boundary of the site. Water mains cross the western half of the site, of which they generally follow a connection road between Lemon Gulch Way and Stroh Road.

Existing vegetation on site is comprised of native grasses, shrubs, trees, and weedy species indigenous to the area.

The site in its current state contains Soil Groups B, C, and D with the following breakdown: 49.8% B, 38% C, and 12.2% D. [Ref 3]

3. Previous Studies

The regional sub catchment basins within the area have been analyzed extensively by the Urban Drainage and Flood Control District (UDFCD). The foundation of this conceptual report was based upon, and compared against, the following Outfall Systems Planning (OSP) and Master Drainage studies:

1. Oak Gulch and Stroh Ranch Area Outfall Systems Planning – Preliminary Design Report by Knight Piesold Consulting
2. Master Drainage Plan for Anthology by Harris Kocher Smith (HKS)
3. Scott Gulch and Lemon Gulch Watershed Outfall systems Planning – Preliminary Design Report by CH2M Hill
4. Cherry Creek Corridor – Reservoir to Scott Road Major Drainageway Planning – Preliminary Design Report by URS
5. Conceptual Drainage Report for Hess Ranch Development by Manhard Consulting

The previous OSPs provide detailed CUHP/SWMM analyses regarding the regional sub catchment flows, recommendations for erosion mitigation, regional detention pond system sizing, and design alternatives, with cost breakdowns, for the various channels in the area.

The HKS master drainage report provides a master drainage plan, for what was at the time, the latest proposed land use plan within the area. It makes use of previous UDFCD studies to compile a master drainage plan for all of Anthology

4. Proposed Development

Trails at Crowfoot development consists of approximately 890 single family residential lots. The site also comprise of school, multi-family development, parks and mixed use. The intent of this preliminary drainage report is to size storm infrastructure for Trails at Crowfoot Development. The report addresses land use and the relevant percentage imperviousness for the site. Ponds have been sized based on UD-Detention spreadsheets. CUHP/ SWMM analysis was used to verify the UD-Detention spreadsheet design.

II. HISTORIC DRAINAGE

1. Major Basin and Sub Basin Description

The Trails at Crowfoot Development site encompasses a fraction of five much larger major basins: Oak Gulch, West Stroh, North Crowfoot Valley, Scott Gulch, and Lemon Gulch. The Trails at Crowfoot Development is mostly in the Lemon Gulch Basin and OS-2 basin with a portion in the Scott Gulch Basin. The sub basins in this section are taken from the Hess Ranch Conceptual Design [Ref. 4].

1. Scott Gulch

Scott Gulch is located along the southernmost point of the development boundary and is adjacent to the Lemon Gulch and OS-2 drainage basins. The latest development plan indicates that Scott Gulch will primarily consist of single family residential and open space

Note: *A detention/water quality pond will be required for that portion of Basin 406 situated onsite. Coordination with the adjacent landowner will be required to discuss routing of historic release flows.*

2. Lemon Gulch

As a part of the Scott Gulch and Lemon Gulch OSP, Lemon Gulch makes up a large portion of the Trails at Crowfoot Development site. Lemon Gulch is centrally located within the development and is substantially larger than the other major basins that are contributed by the development area. The latest development plan indicates that Lemon Gulch will consist of single family residential and mixed use property, park space, open space, roadways, and a commercial development. A regional pond will capture runoff from the Trails development and release directly to Lemon Gulch.

3. Cherry Creek

The drainage Basin OS-2 conveys runoff to the east and into Cherry Creek. Cherry Creek parallels the east border of the property. The development plan for this area is single family residential and mixed us property, a large park space, roadways, and a school. A regional pond will capture runoff from the Trails development and release directly to Cherry Creek.

2. Floodplain

According to the *Federal Emergency Management Agencies (FEMA) Flood Emergency Rate Map (FIRM)* [Ref. 5], proposed development area lies within "Zone X" which is described as an area determined to be outside the 500 year floodplain limits or shallow

flooding areas with average depths of less than one foot or drainage areas less than one square mile.

There are no existing major irrigation facilities such as ditches and canals on the property or within 100-feet of the property.

III. DESIGN CRITERIA

1. List of References

This preliminary drainage report is in accordance with the *Storm Drainage and Environmental Criteria Manual* [Ref. 2] and *Urban Storm Drainage Criteria Manual, VOL 1, 2 &3* [Ref. 1 - UDFCD].

Along with these criteria manuals, this report also adheres to the general guidelines set forth by the *Hess Ranch Conceptual Drainage Report* [Ref. 4].

2. Hydrologic Criteria

For Trails at Crowfoot, the Rational Method was used to establish the peak flow rate at design points throughout the development. Per the SDECM [Ref. 2], rainfall intensity is determined using the one-hour rainfall depth, P_1, in the USDCM [Ref. 1] Equation RA-3 as shown below:

$$I = \frac{28.5\,P_1}{(10 + T_c)^{0.786}} \qquad \text{(RA-3)}$$

The 2-year storm event was determined to have a 0.99-inch 1-hour rainfall depth, and the 100-year storm event was determined to have a 2.60-inch 1-hour rainfall depth.

3. Hydraulic Criteria

The *"Urban Storm Drainage Criteria Manual, Volumes 1, 2, and 3"* (USDCM) [Ref. 1] and the *"Storm Drainage and Environmental Criteria Manual"* (SDECM) [Ref. 2] are the design guidelines for the design and analyses provided in this report. Inlets, street, and pipes are designed in this report, which utilizes the 2-year and 100-year design storm events respectively as the basis of the minor and major storm events. Urban Drainage and Flood Control District's UD-Inlet.xls software is employed to evaluate the streets and inlets capacity. UDFCD's UD-Sewer software will be utilized to determine the Hydraulic Grade Line for the storm sewer networks in the Final Drainage Report. UD-Detention_V3.03 was used to size detention ponds. The results were verified in a CUHP/SWMM model.

This report assumes on-grade inlets will be sized to intercept the 2-year design peak flow, and sump inlets will be sized to intercept up-to the 100-year design peak flow. The site inlets and street capacities are evaluated with UD-Inlet.xls in this report.

Storm pipe sizes will be finalized and hydraulic grade lines evaluated in the Final Drainage Report with UD-Sewer software.

IV. DRAINAGE PLAN

1. General Concept

Trails at Crowfoot is delineated into 64 sub-basins. Sub-basins with designations "A", "B" and "C" drain to Pond A. Sub-basins D1, D2, D6, D7, D8, D10 & D12 drain to Pond B. Sub-basins D3, D4, D5, D9, D11 and designations "E" & "F" drain to Pond C.

2. Specific Details

Trails at Crowfoot is delineated into 64 sub-basins and 4 off-site basins that discharge to three detention ponds.

Pond A is a maintenance eligible regional facility with a drainage area of 159 acres. The weighted average imperviousness for the area is 44.40%. The pond provide full spectrum detention per UDFCD requirements. The outlet discharges into Lemon Gulch. Detailed Design of the pond will be completed for the Final Report.

POND A	
Description	
EURV Volume (including WQVC)	7.03
EURV Water Surface*	-
100-YR Volume (including EURV)	14.85
100-yar water surface elevation*	-
Emergency Spillway Crest Elevation*	-
100-year Peak Inflow (cfs)	446.9
100-year Peak Outflow (cfs)	186.06
*To be finalized in Final Report	

Pond B is a sub-regional facility with a drainage area of 23 acres. The weighted average imperviousness for the area is 53.40%. The pond provide full spectrum detention per UDFCD requirements. The outlet discharges into a pipe provided by the downstream development. And ultimately discharges into Scott Gulch. Detailed Design of the pond will be completed for the Final Report.

POND B	
Description	
EURV Volume (including WQVC)	1.25
EURV Water Surface*	-
100-YR Volume (including EURV)	2.41
100-yar water surface elevation*	-
Emergency Spillway Crest Elevation*	-
100-year Peak Inflow (cfs)	106.28
100-year Peak Outflow (cfs)	31.53
*To be finalized in Final Report	

Pond C is a maintenance eligible regional facility with a drainage area of 161 acres. The weighted average imperviousness for the area is 53.43%. The pond provides full spectrum detention per UDFCD requirements. The outlet discharges to Cherry Creek and will comply with Cherry Creek Reservoir Control Regulations. Detailed Design of the pond will be completed for the Final Report.

POND C	
Description	
EURV Volume (including WQVC)	8.7
EURV Water Surface*	-
100-YR Volume (including EURV)	16.74
100-yar water surface elevation*	-
Emergency Spillway Crest Elevation*	-
100-year Peak Inflow (cfs)	541.71
100-year Peak Outflow (cfs)	177.21
*To be finalized in Final Report	

Sub-basins are described in detail as follows.

Sub-basin A1 primarily consists of the lots along Street H. Surface runoff generally drains to the curb and gutter, which continues northerly to the on-grade inlet at Design Point 1K where it is piped to sump inlet at DP 1B. Emergency flow from DP 1B will overland to west channel.

Sub-basin A2 primarily consists of the lots along Street F and G. Surface runoff generally drains to the curb and gutter to sump inlet at Design Point 1B where it is piped west channel via swale. Emergency flow from DP 1B will overland to west channel.

Sub-basin A3 primarily consists of the lots along Street F. Surface runoff generally drains to the curb and gutter to sump inlet at Design Point 1C where it is piped west channel via swale. Emergency flow from DP 1C will overland to west channel.

Sub-basin A4 primarily consists of the lots along Street A and D. Surface runoff generally drains to the curb and gutter to sump inlet at Design Point 1D where it is piped west channel via swale. Emergency flow from DP 1D will overland to west channel.

Sub-basin A5 primarily consists of the lots along Street A and C. Surface runoff generally drains to the curb and gutter to sump inlet at Design Point 1E where it is piped west channel via swale. Emergency flow from DP 1E will overland to west channel.

Sub-basin A6 primarily consists of the lots along Street A and B. Surface runoff generally drains to the curb and gutter, which continues westerly to the on-grade inlet at Design Point 1F where it is piped to sump inlet at DP 1N. Emergency flow from DP 1N will overland to west channel via swale.

Sub-basin A7 primarily consists of the lots along Street D and C. Surface runoff generally drains to the curb and gutter, which continues northerly to the sump inlet at DP 1E. Emergency flow from DP 1N will overland to west channel via swale.

Sub-basin A8 primarily consists of the lots along Street A. Surface runoff generally drains to the curb and gutter to the sump inlet at DP 1H. Emergency flow from DP 1H will overland to west channel via swale.

Sub-basin A9 is located along Bayou Gulch Rd. Surface runoff generally drains to the curb and gutter to DP 1I where it runs westerly to Design Point IJ. Emergency flow from DP 1J will overland to west channel.

Sub-basin A10 is located along Bayou Gulch Rd. Surface runoff generally drains to the curb and gutter to on grade inlet at DP 1J. Emergency flow from DP 1J will overland to west channel.

Sub-basin A11 primarily consists of the lots along Street G and H. Surface runoff generally drains to the curb and gutter to on grade inlet at Design Point 1K. 100 Year flow from 1K will overland to sump inlet at Design Point 1B where it is piped west channel via swale. Emergency flow from DP 1B will overland to west channel.

Sub-basin A12 primarily consists of the lots along Street B and C. Surface runoff generally drains to the curb and gutter to on grade inlet at Design Point 1L. 100 Year flow from 1L will overland to sump inlet at Design Point 1N where it is piped west channel via swale. Emergency flow from DP 1N will overland to west channel.

Sub-basin A13 primarily consists of the lots along Street C and H. Surface runoff generally drains to the curb and gutter to on grade inlet at Design Point 1M. 100 Year flow from 1M

will overland to sump inlet at Design Point 1E where it is piped west channel via swale. Emergency flow from DP 1E will overland to west channel.

Sub-basin A14 primarily consists of the lots along Street A. Surface runoff generally drains to the curb and gutter to the sump inlet at DP 1N. Emergency flow from DP 1N will overland to west channel via swale.

Sub-basin A15 is located along Bayou Gulch Rd. Surface runoff generally drains to the curb and gutter to on-grade Inlet at DP 1O. Emergency flow from DP 1O will overland to west channel.

BASIN SUMMARY			
BASIN ID	AREA	Q_2	Q_{100}
	(AC)	(CFS)	(CFS)
A1	4.86	6.07	23.37
A2	4.37	6.12	23.00
A3	3.23	3.71	14.72
A4	4.07	3.34	16.42
A5	4.10	5.28	20.28
A6	4.96	4.11	20.28
A7	6.37	8.85	33.35
A8	2.86	4.04	15.17
A9	3.44	4.60	17.31
A10	0.72	1.41	4.68
A11	6.12	7.45	28.79
A12	5.98	7.54	29.19
A13	7.08	8.00	32.24
A14	1.43	2.12	8.02
A15	7.15	4.95	26.44

Sub-basin B1 is located south west of the project along Street J. Surface runoff generally overland flows to on-grade inlet at DP 2A. Emergency flow from 2A will street flow to sump inlet to DP to 2K.

Sub-basin B2 is located along Street N and D. Surface runoff generally overland flows to on-grade inlet at DP 2A. Emergency flow from 2A will street flow to sump inlet to DP 2K.

Sub-basin B3 is located along Street D. Surface runoff generally drains to the curb and gutter to DP 2C where it runs northerly to on-grade inlet to Design Point 2D. Emergency flow from DP 2D will street flow to sump inlet to DP 2K.

Sub-basin B4 primarily consists of the lots located along Street D. Surface runoff generally drains to on grade inlet at Design Point 2D. 100 year and Emergency flow from DP 2D will street flow to sump inlet to DP 2K.

Sub-basin B5 primarily consists of the lots located along Street D and J. Surface runoff generally drains to on grade inlet at Design Point 2E. Emergency flow from DP 2E will street flow to sump inlet to DP 2K.

Sub-basin B6 primarily consists of the lots located along Street D and J. Surface runoff generally drains to on grade inlet at Design Point 2F. Emergency flow from DP 2F will street flow to sump inlet to DP 2K.

Sub-basin B7 primarily consists of the lots located along Street I and J. Surface runoff generally drains to Design Point 2G. Flow from DP 2G will street flow to local sump inlet at Design Point 2I. Emergency flow from DP 2I will street flow to sump inlet to DP 2J.

Sub-basin B8 primarily consists of the lots located along Street D and J. Surface runoff generally drains to local sump inlet at Design Point 2H. Emergency flow from DP 2H will street flow to sump inlet to DP 2J.

Sub-basin B9 primarily consists of the lots located along Street D and J. Surface runoff generally drains to local sump inlet at Design Point 2I. Emergency flow from DP 2I will overland to sump inlet to DP 2J.

Sub-basin B10 is located along N Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 2J. Emergency flow from 2J will overland to west channel.

Sub-basin B11 is located along N Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 2K. Emergency flow from 2K will overland to west channel.

BASIN SUMMARY			
BASIN ID	AREA	Q_2	Q_{100}
	(AC)	(CFS)	(CFS)
B1	21.00	9.90	65.44
B2	3.13	4.40	16.85
B3	4.92	6.66	25.30
B4	4.03	10.52	30.33
B5	6.38	8.50	31.58
B6	6.38	8.50	31.58
B7	5.76	6.89	26.54
B8	4.93	6.02	23.77
B9	2.81	3.39	13.38
B10	0.65	1.89	5.29
B11	0.84	2.34	6.55

Sub-basin C1 primarily consists of the lots located along Street J. Surface runoff generally drains to sump inlet at Design Point 3A. Emergency flow from DP 3A overland flow to west channel via swale.

Sub-basin D1 primarily consists of the lots located along Street J. Surface runoff generally drains to sump inlet at Design Point 4A. Emergency flow from DP 4A will overland Pond B.

Sub-basin D2 primarily consists of the lots located along Street N and J. Surface runoff generally drains to Design Point 4B. Flow from DP 4B street flows to sump inlet at Design Point 4J. Emergency flow from DP 4J will overland to Pond B.

Sub-basin D3 primarily consists of the lots located along Street D. Surface runoff generally drains to Design Point 4C. Flow from DP 4C street flows to sump inlet at Design Point 4E. Emergency flow from DP 4E will street flow on N Pinery Parkway to Pond C.

Sub-basin D3 primarily consists of the lots located along Street D. Surface runoff generally drains to Design Point 4C. Flow from DP 4C street flows to sump inlet at Design Point 4E. Emergency flow from DP 4E will street flow on N Pinery Parkway to Pond C.

Sub-basin D4 primarily consists of the lots located along Street K. Surface runoff generally drains to Design Point 4D. Flow from DP 4D street flows to sump inlet at Design Point 4E. Emergency flow from DP 4E will street flow on N Pinery Parkway to Pond C.

Sub-basin D5 primarily consists of the lots located along Bayou Gulch Rd and N Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 4E. Emergency flow from DP 4E will street flow on N Pinery Parkway to Pond C.

Sub-basin D6 is located along Bayou Gulch Rd. Surface runoff generally drains to sump inlet at Design Point 4F. Emergency flow from DP 4F will overland flow to Pond B.

Sub-basin D7 is located along Street D. Surface runoff generally drains to on-grade inlet at Design Point 4G. Emergency flow from DP 4G will street flow to Pond B.

Sub-basin D8 primarily consists of the lots located along Street K. Surface runoff generally drains to Design Point 4H. Flow from DP 4H street flows to sump inlet at Design Point 4F. Emergency flow from DP 4F will overland to Pond B.

Sub-basin D9 primarily consists of the lots located along N. Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 4I. Emergency flow from DP 4I street flow to Pond C via N Pinery Parkway.

Sub-basin D10 primarily consists of the lots located along Street P and J. Surface runoff generally drains to sump inlet at Design Point 4J. Emergency flow from DP 4J will overland to Pond B.

Sub-basin D11 is located along N. Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 4K. Emergency flow from DP 4K will street flow on N. Pinery Parkway.

Sub-basin D12 is located along Bayou Gulch. Surface runoff generally drains to on-grade inlet at Design Point 4L. Emergency flow from DP 4L will overland to Pond B.

BASIN SUMMARY			
BASIN ID	AREA	Q_2	Q_{100}
	(AC)	(CFS)	(CFS)
C1	7.47	7.20	30.57
D1	5.94	5.89	26.25
D2	5.33	6.52	26.60
D3	5.67	7.12	27.00
D4	2.91	2.73	11.96
D5	9.10	12.05	42.92
D6	2.57	2.80	11.66
D7	2.58	2.97	12.69
D8	0.85	1.22	4.69
D9	4.62	5.03	20.89
D10	4.80	6.30	24.78
D11	3.29	7.86	22.03
D12	1.13	2.70	7.58

Sub-basin E1 is located along Street Q. Surface runoff generally drains to on-grade inlet at Design Point 5A. Emergency flow from DP 5A street flow to Pond C via Street C.

Sub-basin E2 primarily consists of lots located along Street R. Surface runoff generally drains to sump inlet at Design Point 5B. Emergency flow from DP 5B overland flows to Pond C.

Sub-basin E3 primarily consists of lots located along Street R and S. Surface runoff generally drains to on-grade inlet at Design Point 5C.100 Year and Emergency flow from DP 5C street flows to Design Point 5E.

Sub-basin E4 primarily consists of lots located along Street S and T. Surface runoff generally drains to Design Point 5D where it street flows southerly Design Point 5E. Emergency flow from DP 5E overland flows to Pond C.

Sub-basin E5 primarily consists of lots located along Street R. Surface runoff generally drains to sump inlet at Design Point 5E. Emergency flow from DP 5E overland flows to Pond C.

Sub-basin E6 primarily consists of lots located along Street V and 2. Surface runoff generally drains to on-grade inlet at Design Point 5F. 100 Year flow street flows to sump inlet at Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E7 primarily consists of lots located along Street X and R. Surface runoff generally drains to sump inlet to Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E8 primarily consists of lots located along Street E and Y. Surface runoff generally drains to on-grade inlet at Design Point 5H. 100 Year discharge street flows to sump inlet at Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E9 primarily consists of lots located along Street R and X. Surface runoff generally drains to on-grade inlet at Design Point 5I. 100 Year discharge street flows to sump inlet at Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E10 primarily consists of lots located along Street R. Surface runoff generally drains to Design Point 5J. Flow from 5J street flows to sump inlet at Design Point 5B. Emergency flow from DP 5B overland flows to Pond C.

Sub-basin E11 primarily consists of open space along Street E. Surface runoff generally drains to Design Point 5K. Flow from 5K street flows to on-grade inlet at Design Point 5X. 100 year flows from 5X will street flow to Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E12 primarily consists of open space along Street E. Surface runoff generally drains to Design Point 5L. Flow from 5L street flows to on-grade inlet at Design Point 5X. 100 year flows from 5X will street flow to Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E13 primarily consists of open space along N. Pinery Parkway. Surface runoff generally drains to Design Point 5M. Flow from 5M street flows to on-grade inlet at Design Point 5Y. 100 year flows from 5Y will street flow to Design Point 6H via pipe and swale through basin F8. Emergency flow from DP 6H overland flows to Pond C.

Sub-basin E14 primarily consists of lots located along Street Q. Surface runoff generally drains to sump inlet at Design Point 5N. Emergency flow from DP 5N will street flow on Street E.

BASIN SUMMARY			
BASIN ID	AREA	Q_2	Q_{100}
	(AC)	(CFS)	(CFS)
E1	15.28	15.39	66.03
E2	5.27	5.36	20.26
E3	4.77	6.32	23.90
E4	3.14	4.32	16.38
E5	5.49	6.64	26.15
E6	5.54	7.76	29.46
E7	5.53	7.08	26.75
E8	4.73	6.65	24.86
E9	4.84	4.59	20.55
E10	0.70	1.04	3.68
E11	3.99	2.48	14.96
E12	7.51	4.67	28.17
E13	4.45	2.76	16.67
E14	11.27	13.66	53.43

Sub-basin E15 primarily consists of lots located along Street W and T. Surface runoff generally drains to Design Point 5W. Flow from 5W street flows to sump inlet at Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E16 is located along Street E. Surface runoff generally drains to on-grade inlet at Design Point 5X. Flow from 5X street flows to sump inlet at Design Point 5G. Emergency flow from DP 5G overland flows to Pond C.

Sub-basin E17 is located along N. Pinery Parkway. Surface runoff generally drains to on-grade inlet at Design Point 5Y. Flow from 5Y street flows to sump inlet at Design Point 6H. Emergency flow from DP 6H overland flows to Pond C.

Sub-basin F1 is located along Street Q and N. Pinery Parkway. Surface runoff generally drains to on-grade inlet at Design Point 6A. 100 Year discharge from 6A street flows to sump inlet at Design Point 6I. Emergency flow from DP 6I overland flows to East Channel.

Sub-basin F2 is located along Bayou Gulch Rd. Surface runoff generally drains to on-grade inlet at Design Point 6B. 100 Year discharge from 6B street flows to sump inlet at Design Point 6I. Emergency flow from DP 6I overland flows to East Channel.

Sub-basin F3 is located along Street E. Surface runoff generally drains to on-grade inlet at Design Point 6C. 100 Year discharge from 6C street flows to sump inlet at Design Point 6I. Emergency flow from DP 6I overland flows to East Channel.

Sub-basin F4 primarily consists of lots located along Street FF. Surface runoff generally drains to Design Point 6D. 2 Year discharge from 6D street flows Design Point 6C. 100 Year discharge from 6D street flows to sump inlet at Design Point 6I. Emergency flow from DP 6I overland flows to East Channel.

Sub-basin F5 primarily consists of lots located along N. Pinery Parkway. Surface runoff generally drains to on-grade inlet at Design Point 6E. 100 Year discharge from 6E street flows to sump inlet at Design Point 6I. Emergency flow from DP 6I overland flows to East Channel.

Sub-basin F6 primarily consists of lots located along Street Z. Surface runoff generally drains to sump inlet at Design Point 6F. Emergency flow from DP 6F overland flows to Pond C.

Sub-basin F7 is located along N. Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 6G. Emergency flow from DP 6G overland flows to East Channel.

Sub-basin F8 primarily consists of lots located along Street Y. Surface runoff generally drains to sump inlet at Design Point 6H. Emergency flow from DP 6H overland flows to Pond C.

Sub-basin F9 is located along street E and N. Pinery Parkway. Surface runoff generally drains to sump inlet at Design Point 6I. Emergency flow from DP 6I overland flows to East Channel.

BASIN SUMMARY			
BASIN ID	AREA	Q_2	Q_{100}
	(AC)	(CFS)	(CFS)
F1	5.16	12.76	36.88
F2	2.99	7.34	21.27
F3	3.60	1.95	13.13
F4	7.36	9.68	36.24
F5	4.58	5.34	21.28
F6	5.67	5.37	24.06
F7	2.98	2.25	11.77
F8	8.58	6.91	34.42
F9	1.28	2.47	7.48
E15	1.95	2.46	9.41
E16	1.57	4.11	11.53
E17	1.55	4.05	11.37

Sub-basin OS 1 is located south west of the site. Surface runoff overland flows to Design Point A. Flow from DP A enters West Channel.

Sub-basin OS 2 is located west of the site. Surface runoff overland flows to West Channel.

Sub-basin OS 3 is located north west of the site. Surface runoff overland flows to West Channel.

Sub-basin OS 4 is located south east of the site. Surface runoff overland flows to East Channel.

V. ENVIRONMENTAL PROTECTION CRITERIA

1. Erosion and Sediment Control Concept

Where possible native open space area will be maintained around the perimeter of the site and along/within the natural drainage ways. As planning areas develop, BMP erosion control plans shall be developed to control erosion and sediment at the construction site.

VI. CONCLUSIONS

1. Compliance with Standards

This Preliminary Drainage Report for Trails at Crowfoot is prepared in general conformance with the *"Storm Drainage and Environmental Criteria Manual"* [Ref. 2] and the *"Urban Storm Drainage Criteria Manual, Volumes 1, 2, and 3"* [Ref. 1].

2. Summary of Concept

The report addresses drainage concepts related to pond and storm system infrastructure sizing. The site drains to three major basins. Surface flow is intercepted by on-grade and sump inlets and are piped to three separate ponds. The three ponds will capture and detain developed runoff before exiting the site, respective to the drainage basin.

VI. REFERENCES

1. **Urban Storm Drainage Criteria Manual, Volumes 1,2,3**, Urban Drainage and Flood Control District, prepared by Urban Drainage and Flood Control District, Latest Revisions.

2. **Web Soil Survey**, http://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx, United States Department of Agriculture – Natural Resources Conservation Service.

3. **First Creek Major Drainageway Plan Conceptual Design Report**, Moser and Associates Engineering, August 2010.

4. **Hess Ranch Conceptual Drainage Report**, Manhard Consulting, June 23, 2015.

5. **FIRM, Flood Insurance Rate Map**, Douglas County, Colorado, map number 08035CO181G, 08035CO182G, 08035CO183G, and 08035CO184G Federal Emergency Management Agency, March 16, 2016.

Appendix

I. Vicinity Map

II. Hydrologic Computations

A. Land use assumptions
B. Minor and major storm runoff computations for historic and developed runoff conditions
C. Detention and water quality volume required and provided calculations

III. Hydraulic Computations

A. Detention pond capacity
B. Pipe Hydraulics

IV. Copies of graphs, tables, and nomographs

A. FIRM
B. Soils Report
C. 1-Hour Rainfall Graph
D. Excerpts from Adjacent Studies
    - Piney Creek MDP

V. Sub-basin Exhibit

Appendix

I. Vicinity Map

II. Hydrologic Computations

A. Land use assumptions
B. Minor and major storm runoff computations for historic and developed runoff conditions
C. Pond Calculations

III. Hydraulic Computations

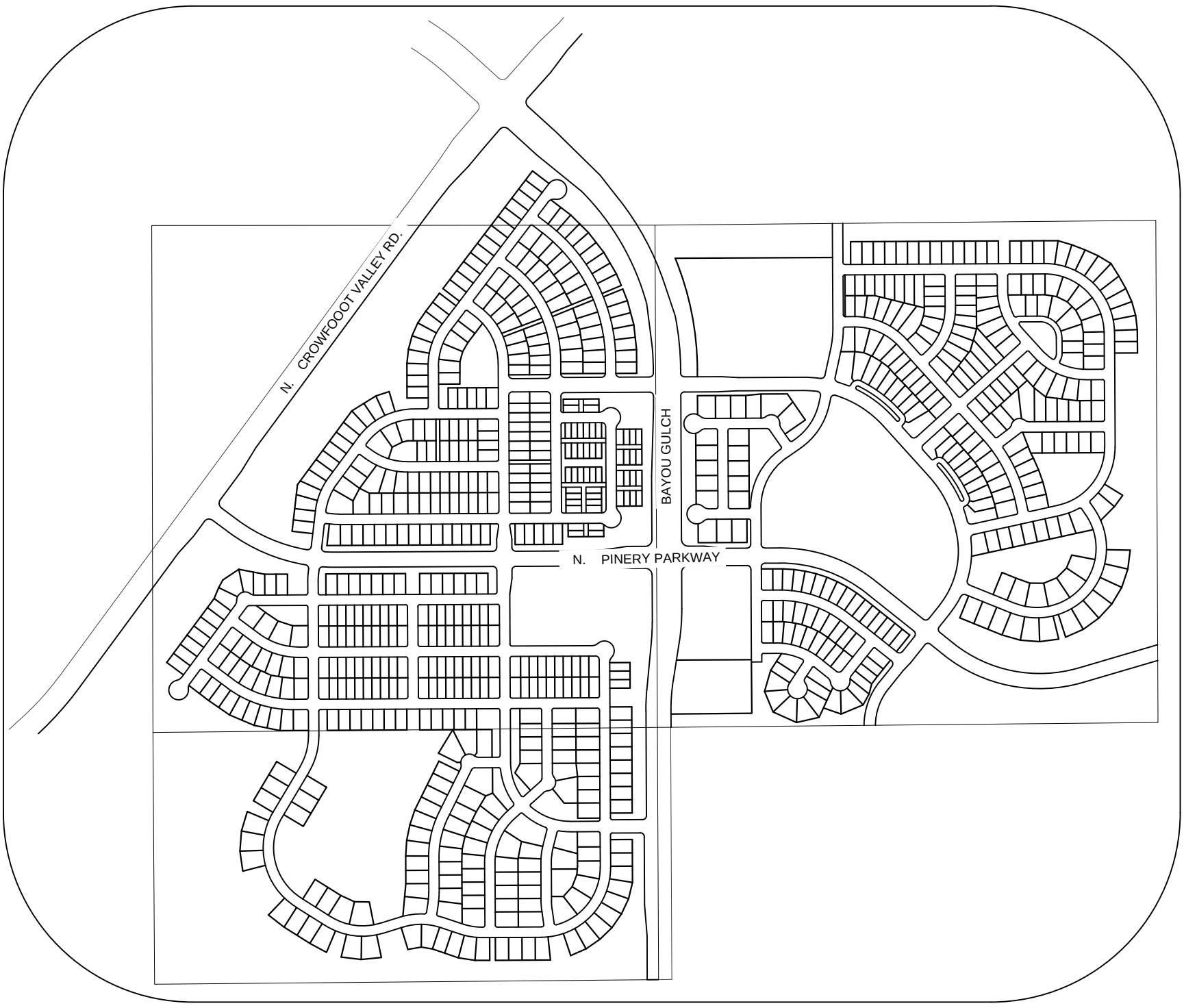
A. CUHP & SWMM
B. UD-Inlet
C. UD-Sewer
D. UD-Channel
E. UD-Culvert

IV. Copies of graphs, tables, and nomographs

A. FIRM
B. Soils Report
C. 1-Hour Rainfall Data
D. Excerpts from Adjacent Studies

V. Sub-basin Exhibit

I. Vicinity Map



VICINITY MAP

N.T.S.

II. Hydrologic Computations

A. Land use assumptions
    1. Imperviousness & Composite C Calculations
    2. SF-1-Time of Concentrations
B. Minor and major storm runoff computations for historic and developed runoff conditions
    1. SF-2 2-Year Sub-Basin Rational Method Calculations
    2. SF-2 100-Year Sub-Basin Ration Method Calculations
C. Pond Calculations

COMPOSITE BASIN COEFFICIENTS

Subdivision: Trails at Crowfoot

Project Name: Trails at Crowfoot

Project No. 254103

Soil Type B/C/D

Calculated By: MRS

Soil Type B **49.80%**

Land Use	Imp.	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.42	0.67
School	55%	0.49	0.51	0.72
Business	95%	0.85	0.88	0.92
Residential (Multi Family)	75%	0.67	0.70	0.82
Streets	100%	0.89	0.93	0.94
Parks	10%	0.09	0.09	0.50
Open Space / Lawns	2%	0.02	0.02	0.46
Mixed Use	30%	0.27	0.28	0.60

Soil Type C/D **50.20%**

Land Use	Imp.	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.44	0.71
School	55%	0.49	0.53	0.76
Business	95%	0.85	0.88	0.96
Residential (Multi Family)	75%	0.67	0.70	0.85
Streets	100%	0.89	0.92	0.96
Parks	10%	0.09	0.14	0.55
Open Space / Lawns	2%	0.02	0.07	0.52
Mixed Use	30%	0.27	0.31	0.64

Composite Runoff Co-eff

Land Use	Imp.	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69
School	55%	0.49	0.52	0.74
Business	95%	0.85	0.88	0.94
Residential (Multi Family)	75%	0.67	0.70	0.84
Streets	100%	0.89	0.92	0.95
Parks	10%	0.09	0.12	0.53
Open Space / Lawns	2%	0.02	0.05	0.49
Mixed Use	30%	0.27	0.30	0.62

A1

Total Area **4.86 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.22	20.5	0.18	0.20	0.32
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.77	36.4	0.32	0.34	0.35
Open Space / Lawns	2%	0.02	0.05	0.49	0.87	0.4	0.00	0.01	0.09
TOTAL					**4.86**	**57.3**	**0.51**	**0.54**	**0.75**

A2

Total Area **4.37 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.21	33.1	0.29	0.32	0.51
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.16	26.5	0.24	0.25	0.25
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**4.37**	**59.6**	**0.53**	**0.56**	**0.76**

A3

Total Area **3.23 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.28	31.8	0.28	0.30	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.74	23.0	0.20	0.21	0.22
Open Space / Lawns	2%	0.02	0.05	0.49	0.21	0.1	0.00	0.00	0.03
TOTAL					**3.23**	**54.9**	**0.49**	**0.52**	**0.74**

A4

Total Area **4.07 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	1.50	16.6	0.15	0.16	0.25
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.92	22.6	0.20	0.21	0.21
Open Space / Lawns	2%	0.02	0.05	0.49	1.65	0.8	0.01	0.02	0.20
TOTAL					**4.07**	**40.0**	**0.36**	**0.39**	**0.67**

A5

Total Area **4.10 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.80	30.7	0.27	0.29	0.47
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.10	26.9	0.24	0.25	0.26
Open Space / Lawns	2%	0.02	0.05	0.49	0.20	0.1	0.00	0.00	0.02
TOTAL					**4.10**	**57.7**	**0.51**	**0.54**	**0.75**

A6

Total Area **4.96 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.44	22.1	0.20	0.21	0.34
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.84	16.9	0.15	0.16	0.16
Open Space / Lawns	2%	0.02	0.05	0.49	1.68	0.7	0.01	0.02	0.17
TOTAL					**4.96**	**39.8**	**0.35**	**0.38**	**0.67**

A7

Total Area **6.37 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.78	26.7	0.24	0.26	0.41
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	2.08	32.6	0.29	0.30	0.31
Open Space / Lawns	2%	0.02	0.05	0.49	0.51	0.2	0.00	0.00	0.04
TOTAL					**6.37**	**59.5**	**0.53**	**0.56**	**0.76**

A8

Total Area **2.86 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.09	32.9	0.29	0.31	0.50
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.77	26.9	0.24	0.25	0.26
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**2.86**	**59.8**	**0.53**	**0.56**	**0.76**

A9

Total Area **3.44 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	2.02	58.7	0.52	0.54	0.56
Open Space / Lawns	2%	0.02	0.05	0.49	1.42	0.8	0.01	0.02	0.20
TOTAL					**3.44**	**59.5**	**0.53**	**0.56**	**0.76**

A10

Total Area **0.72 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.52	72.2	0.64	0.67	0.69
Open Space / Lawns	2%	0.02	0.05	0.49	0.20	0.6	0.01	0.01	0.14
TOTAL					**0.72**	**72.8**	**0.65**	**0.68**	**0.82**

A11

Total Area **6.12 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.77	35.0	0.31	0.33	0.54
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.35	22.1	0.20	0.20	0.21
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**6.12**	**57.2**	**0.51**	**0.54**	**0.75**

A12

Total Area **5.98 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.23	31.8	0.28	0.30	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.50	25.0	0.22	0.23	0.24
Open Space / Lawns	2%	0.02	0.05	0.49	0.26	0.1	0.00	0.00	0.02
TOTAL					**5.98**	**56.9**	**0.51**	**0.54**	**0.75**

A13

Total Area **7.08 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	2.45	26.0	0.23	0.24	0.29
Streets	100%	0.89	0.92	0.95	1.89	26.7	0.24	0.25	0.25
Open Space / Lawns	2%	0.02	0.05	0.49	2.74	0.8	0.01	0.02	0.19
TOTAL					**7.08**	**53.5**	**0.48**	**0.51**	**0.73**

A14

Total Area **1.43 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	1.07	33.7	0.30	0.32	0.52
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.36	25.1	0.22	0.23	0.24
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**1.43**	**58.8**	**0.52**	**0.55**	**0.76**

A15

Total Area **7.15 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	2.45	34.3	0.31	0.32	0.33
Open Space / Lawns	2%	0.02	0.05	0.49	4.70	1.3	0.01	0.03	0.32
TOTAL					**7.15**	**35.6**	**0.32**	**0.35**	**0.65**

B1

Total Area **21.00 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	5.62	12.0	0.11	0.12	0.18
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	2.88	13.7	0.12	0.13	0.13
Open Space / Lawns	2%	0.02	0.05	0.49	12.50	1.2	0.01	0.03	0.29
TOTAL					**21.00**	**26.9**	**0.24**	**0.27**	**0.61**

B2

Total Area **3.13 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.39	34.4	0.31	0.33	0.53
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.74	23.6	0.21	0.22	0.22
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**3.13**	**58.0**	**0.52**	**0.55**	**0.75**

B3

Total Area **4.92 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.93	26.8	0.24	0.26	0.41
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.56	31.7	0.28	0.29	0.30
Open Space / Lawns	2%	0.02	0.05	0.49	0.43	0.2	0.00	0.00	0.04
TOTAL					**4.92**	**58.7**	**0.52**	**0.55**	**0.76**

B4

Total Area **4.03 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	3.20	75.4	0.67	0.70	0.75
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.83	20.6	0.18	0.19	0.20
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**4.03**	**96.0**	**0.86**	**0.89**	**0.94**

B5

Total Area **6.38 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.55	32.1	0.29	0.31	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.83	28.7	0.26	0.27	0.27
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**6.38**	**60.8**	**0.54**	**0.57**	**0.76**

B6

Total Area **6.38 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.55	32.1	0.29	0.31	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.83	28.7	0.26	0.27	0.27
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**6.38**	**60.8**	**0.54**	**0.57**	**0.76**

B7

Total Area **5.76 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.57	27.9	0.25	0.27	0.43
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.69	29.3	0.26	0.27	0.28
Open Space / Lawns	2%	0.02	0.05	0.49	0.50	0.2	0.00	0.00	0.04
TOTAL					**5.76**	**57.4**	**0.51**	**0.54**	**0.75**

B8

Total Area **4.93 acres** Composite Calculations

Land Use	.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.13	19.4	0.17	0.19	0.30
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.74	35.3	0.31	0.33	0.34
Open Space / Lawns	2%	0.02	0.05	0.49	1.06	0.4	0.00	0.01	0.11
TOTAL					**4.93**	**55.2**	**0.49**	**0.52**	**0.74**

B9

Total Area **2.81 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.00	32.0	0.28	0.31	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.65	23.1	0.21	0.21	0.22
Open Space / Lawns	2%	0.02	0.05	0.49	0.16	0.1	0.00	0.00	0.03
TOTAL					**2.81**	**55.3**	**0.49**	**0.52**	**0.74**

B10

Total Area **0.65 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.65	100.0	0.89	0.92	0.95
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**0.65**	**100.0**	**0.89**	**0.92**	**0.95**

B11

Total Area **0.84 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.84	100.0	0.89	0.92	0.95
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**0.84**	**100.0**	**0.89**	**0.92**	**0.95**

C1

Total Area **7.47 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.41	26.6	0.24	0.25	0.41
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.68	22.5	0.20	0.21	0.21
Open Space / Lawns	2%	0.02	0.05	0.49	1.38	0.4	0.00	0.01	0.09
TOTAL					**7.47**	**49.4**	**0.44**	**0.47**	**0.71**

D1

Total Area **5.94 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.20	31.8	0.28	0.30	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.83	14.0	0.12	0.13	0.13
Open Space / Lawns	2%	0.02	0.05	0.49	0.91	0.3	0.00	0.01	0.08
TOTAL					**5.94**	**46.1**	**0.41**	**0.44**	**0.70**

D2

Total Area **5.33 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.34	28.2	0.25	0.27	0.43
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.28	24.0	0.21	0.22	0.23
Open Space / Lawns	2%	0.02	0.05	0.49	0.71	0.3	0.00	0.01	0.07
TOTAL					**5.33**	**52.5**	**0.47**	**0.50**	**0.73**

D3

Total Area **5.67 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.10	24.6	0.22	0.24	0.38
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.93	34.0	0.30	0.31	0.32
Open Space / Lawns	2%	0.02	0.05	0.49	0.64	0.2	0.00	0.01	0.06
TOTAL					**5.67**	**58.9**	**0.52**	**0.56**	**0.76**

D4

Total Area **2.91 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	1.83	28.3	0.25	0.27	0.43
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.54	18.6	0.17	0.17	0.18
Open Space / Lawns	2%	0.02	0.05	0.49	0.54	0.4	0.00	0.01	0.09
TOTAL					**2.91**	**47.2**	**0.42**	**0.45**	**0.70**

D5

Total Area **9.10 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	4.05	42.3	0.38	0.39	0.42
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	2.05	22.5	0.20	0.21	0.21
Open Space / Lawns	2%	0.02	0.05	0.49	3.00	0.7	0.01	0.01	0.16
TOTAL					**9.10**	**65.5**	**0.59**	**0.61**	**0.79**

D6

Total Area **2.57 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.28	49.8	0.44	0.46	0.47
Open Space / Lawns	2%	0.02	0.05	0.49	1.29	1.0	0.01	0.02	0.25
TOTAL					**2.57**	**50.8**	**0.45**	**0.48**	**0.72**

D7

Total Area **2.58 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	1.29	22.5	0.20	0.22	0.35
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.67	26.0	0.23	0.24	0.25
Open Space / Lawns	2%	0.02	0.05	0.49	0.62	0.5	0.00	0.01	0.12
TOTAL					**2.58**	**48.9**	**0.44**	**0.47**	**0.71**

D8

Total Area **0.85 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.65	34.4	0.31	0.33	0.53
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.20	23.5	0.21	0.22	0.22
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**0.85**	**57.9**	**0.52**	**0.55**	**0.75**

D9

Total Area **4.62 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.73	11.9	0.11	0.11	0.13
Streets	100%	0.89	0.92	0.95	1.77	38.3	0.34	0.35	0.36
Open Space / Lawns	2%	0.02	0.05	0.49	2.12	0.9	0.01	0.02	0.22
TOTAL					**4.62**	**51.1**	**0.46**	**0.49**	**0.72**

D10

Total Area **4.80 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.87	36.3	0.32	0.35	0.56
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.93	19.3	0.17	0.18	0.18
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**4.80**	**55.6**	**0.49**	**0.53**	**0.74**

D11

Total Area **3.29 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	3.29	100.0	0.89	0.92	0.95
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**3.29**	**100.0**	**0.89**	**0.92**	**0.95**

D12

Total Area **1.13 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.13	100.0	0.89	0.92	0.95
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**1.13**	**100.0**	**0.89**	**0.92**	**0.95**

E1

Total Area **15.28 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
School	55%	0.49	0.52	0.74	10.28	37.0	0.33	0.35	0.50
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.74	11.4	0.10	0.11	0.11
Open Space / Lawns	2%	0.02	0.05	0.49	3.27	0.4	0.00	0.01	0.10
TOTAL					**15.28**	**48.8**	**0.43**	**0.46**	**0.71**

E2

Total Area **5.27 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.75	32.0	0.28	0.31	0.49
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.43	27.1	0.24	0.25	0.26
Open Space / Lawns	2%	0.02	0.05	0.49	0.09	0.0	0.00	0.00	0.01
TOTAL					**5.27**	**59.2**	**0.53**	**0.56**	**0.76**

E3

Total Area **4.77 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.55	33.5	0.30	0.32	0.51
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.22	25.6	0.23	0.24	0.24
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**4.77**	**59.1**	**0.53**	**0.56**	**0.76**

E4

Total Area **3.14 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.35	33.7	0.30	0.32	0.52
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.79	25.2	0.22	0.23	0.24
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**3.14**	**58.8**	**0.52**	**0.55**	**0.76**

E5

Total Area **5.49 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.54	29.0	0.26	0.28	0.45
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.43	26.2	0.23	0.24	0.25
Open Space / Lawns	2%	0.02	0.05	0.49	0.51	0.2	0.00	0.00	0.05
TOTAL					**5.49**	**55.4**	**0.49**	**0.52**	**0.74**

E6

Total Area **5.54 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.78	30.7	0.27	0.29	0.47
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.55	28.0	0.25	0.26	0.27
Open Space / Lawns	2%	0.02	0.05	0.49	0.21	0.1	0.00	0.00	0.02
TOTAL					**5.54**	**58.8**	**0.52**	**0.55**	**0.76**

E7

Total Area **5.53 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	4.10	33.4	0.30	0.32	0.51
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.43	25.8	0.23	0.24	0.25
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**5.53**	**59.2**	**0.53**	**0.56**	**0.76**

E8

Total Area **4.73 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.13	29.8	0.27	0.28	0.46
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.44	30.4	0.27	0.28	0.29
Open Space / Lawns	2%	0.02	0.05	0.49	0.16	0.1	0.00	0.00	0.02
TOTAL					**4.73**	**60.2**	**0.54**	**0.57**	**0.76**

E9

Total Area **4.84 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.04	19.0	0.17	0.18	0.29
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.12	23.2	0.21	0.21	0.22
Parks	10%	0.09	0.12	0.53	1.67	3.5	0.03	0.04	0.18
TOTAL					**4.84**	**45.6**	**0.41**	**0.44**	**0.69**

E10

Total Area **0.70 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.43	27.6	0.25	0.26	0.42
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.27	38.6	0.34	0.36	0.37
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**0.70**	**66.2**	**0.59**	**0.62**	**0.79**

E11

Total Area **3.99 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Mixed Use	30%	0.27	0.30	0.62	3.99	30.0	0.27	0.30	0.62
TOTAL					**3.99**	**30.0**	**0.27**	**0.30**	**0.62**

E12

Total Area **7.51 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Mixed Use	30%	0.27	0.30	0.62	7.51	30.0	0.27	0.30	0.62
TOTAL					**7.51**	**30.0**	**0.27**	**0.30**	**0.62**

E13

Total Area **4.45 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Mixed Use	30%	0.27	0.30	0.62	4.45	30.0	0.27	0.30	0.62
TOTAL					**4.45**	**30.0**	**0.27**	**0.30**	**0.62**

E14

Total Area **11.27 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	4.45	29.6	0.26	0.28	0.33
Streets	100%	0.89	0.92	0.95	2.89	25.6	0.23	0.24	0.24
Open Space / Lawns	2%	0.02	0.05	0.49	3.93	0.7	0.01	0.02	0.17
TOTAL					**11.27**	**56.0**	**0.50**	**0.53**	**0.74**

F1

Total Area **5.16 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	4.46	82.0	0.73	0.76	0.81
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.71	13.7	0.12	0.13	0.13
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**5.16**	**95.7**	**0.86**	**0.89**	**0.94**

F2

Total Area **2.99 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	2.78	88.4	0.79	0.82	0.87
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.21	6.9	0.06	0.06	0.07
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**2.99**	**95.3**	**0.85**	**0.88**	**0.94**

F3

Total Area **3.60 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.89	24.8	0.22	0.23	0.24
Open Space / Lawns	2%	0.02	0.05	0.49	2.70	1.5	0.02	0.03	0.37
TOTAL					**3.60**	**26.3**	**0.24**	**0.26**	**0.60**

F4

Total Area **7.36 acres** Composite Calculations

Land Use	.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	5.35	32.7	0.29	0.31	0.50
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	2.02	27.4	0.24	0.25	0.26
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**7.36**	**60.1**	**0.53**	**0.57**	**0.76**

F5

Total Area **4.58 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.18	21.4	0.19	0.20	0.33
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.49	32.7	0.29	0.30	0.31
Open Space / Lawns	2%	0.02	0.05	0.49	0.90	0.4	0.00	0.01	0.10
TOTAL					**4.58**	**54.5**	**0.48**	**0.52**	**0.74**

F6

Total Area **5.67 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	2.43	19.3	0.17	0.18	0.30
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.46	25.8	0.23	0.24	0.25
Open Space / Lawns	2%	0.02	0.05	0.49	1.78	0.6	0.01	0.01	0.15
TOTAL					**5.67**	**45.7**	**0.41**	**0.44**	**0.69**

F7

Total Area **2.98 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.06	35.4	0.32	0.33	0.34
Open Space / Lawns	2%	0.02	0.05	0.49	1.93	1.3	0.01	0.03	0.32
TOTAL					**2.98**	**36.7**	**0.33**	**0.36**	**0.65**

F8

Total Area **8.58 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	3.33	17.4	0.16	0.17	0.27
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.80	21.0	0.19	0.19	0.20
Open Space / Lawns	2%	0.02	0.05	0.49	3.45	0.8	0.01	0.02	0.20
TOTAL					**8.58**	**39.3**	**0.35**	**0.38**	**0.66**

E15

Total Area **1.95 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	1.49	34.3	0.30	0.33	0.53
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.46	23.8	0.21	0.22	0.23
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**1.95**	**58.1**	**0.52**	**0.55**	**0.75**

E16

Total Area **1.57 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.57	100.0	0.89	0.92	0.95
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**1.57**	**100.0**	**0.89**	**0.92**	**0.95**

E17

Total Area **1.55 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.55	100.0	0.89	0.92	0.95
Open Space / Lawns	2%	0.02	0.05	0.49	0.00	0.0	0.00	0.00	0.00
TOTAL					**1.55**	**100.0**	**0.89**	**0.92**	**0.95**

F9

Total Area **1.28 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	1.10	86.0	0.77	0.80	0.82
Open Space / Lawns	2%	0.02	0.05	0.49	0.18	0.3	0.00	0.01	0.07
TOTAL					**1.28**	**86.3**	**0.77**	**0.80**	**0.89**

OS1

Total Area **4.98 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Open Space / Lawns	2%	0.02	0.05	0.49	4.98	2.0	0.02	0.05	0.49
TOTAL					**4.98**	**2.0**	**0.02**	**0.05**	**0.49**

OS2

Total Area **10.95 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Open Space / Lawns	2%	0.02	0.05	0.49	10.95	2.0	0.02	0.05	0.49
TOTAL					**10.95**	**2.0**	**0.02**	**0.05**	**0.49**

OS3

Total Area **7.81 acres** Composite Calculations

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Open Space / Lawns	2%	0.02	0.05	0.49	7.81	2.0	0.02	0.05	0.49
TOTAL					**7.81**	**2.0**	**0.02**	**0.05**	**0.49**

OS4

Total Area **2.22 acres**

Land Use	Imp.	C_2	C_5	C_{100}	Area	Imp%	C_2	C_5	C_{100}
					Composite Calculations				
Residential (Single Family)	45%	0.40	0.43	0.69	0.00	0.0	0.00	0.00	0.00
Business	95%	0.85	0.88	0.94	0.00	0.0	0.00	0.00	0.00
Residential (Multi Family)	75%	0.67	0.70	0.84	0.00	0.0	0.00	0.00	0.00
Streets	100%	0.89	0.92	0.95	0.00	0.0	0.00	0.00	0.00
Open Space / Lawns	2%	0.02	0.05	0.49	2.22	2.0	0.02	0.05	0.49
TOTAL					**2.22**	**2.0**	**0.02**	**0.05**	**0.49**

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Trails at Crowfoot

Project Name: Trails at Crowfoot

Project No. 254103

Calculated By: MRS

SUB-BASIN DATA			INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)				Tc CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (AC)	C_5	L (FT)	S (%)	T_i (MIN)	L (FT)	S (%)	VEL. (FPS)	T_t (MIN)	COMP. T_c (MIN)	TOTAL LENGTH(FT)	MIN. T_c (MIN)	T_c (MIN)
A1	4.86	0.54	50	1.0	7.1	1150	3.2	3.6	5.4	12.5	1200.0	16.7	12.5
A2	4.37	0.56	50	1.0	6.9	800	3.6	3.8	3.5	10.4	850.0	14.7	10.4
A3	3.23	0.52	50	1.0	7.4	810	1.2	2.2	6.2	13.6	860.0	14.8	13.6
A4	4.07	0.39	50	1.0	9.1	880	2.0	2.8	5.2	14.3	930.0	15.2	14.3
A5	4.10	0.54	50	1.0	7.1	900	2.7	3.2	4.7	11.7	950.0	15.3	11.7
A6	4.96	0.38	50	1.0	9.1	1178	4.5	4.2	4.7	13.8	1228.0	16.8	13.8
A7	6.37	0.56	50	1.0	6.9	870	4.0	4.0	3.6	10.5	920.0	15.1	10.5
A8	2.86	0.56	50	1.0	6.8	540	1.9	2.7	3.4	10.2	590.0	13.3	10.2
A9	3.44	0.56	50	1.0	6.9	1300	5.2	4.6	4.8	11.6	1350.0	17.5	11.6
A10	0.72	0.68	50	1.0	5.3	427	3.5	3.7	1.9	7.2	477.0	12.7	7.2
A11	6.12	0.54	50	1.0	7.1	1230	3.1	3.5	5.9	13.1	1280.0	17.1	13.1
A12	5.98	0.54	50	1.0	7.2	1090	3.7	3.8	4.8	11.9	1140.0	16.3	11.9
A13	7.08	0.51	50	1.0	7.5	890	1.7	2.5	5.9	13.4	940.0	15.2	13.4
A14	1.43	0.55	50	1.0	6.9	430	4.4	4.2	1.7	8.7	480.0	12.7	8.7
A15	7.15	0.35	50	1.0	9.6	1669	4.7	4.3	6.5	16.1	1719.0	19.6	16.1
B1	21.00	0.27	50	1.0	10.6	1925	3.0	3.5	9.3	19.8	1975.0	21.0	19.8
B2	3.13	0.55	50	1.0	7.0	650	4.8	4.3	2.5	9.5	700.0	13.9	9.5
B3	4.92	0.55	50	1.0	7.0	750	2.7	3.2	3.9	10.8	800.0	14.4	10.8
B4	4.03	0.89	50	1.0	2.7	900	3.0	3.5	4.3	7.0	950.0	15.3	7.0
B5	6.38	0.57	50	1.0	6.7	1200	3.3	3.6	5.5	12.2	1250.0	16.9	12.2
B6	6.38	0.57	50	1.0	6.7	1200	3.3	3.6	5.5	12.2	1250.0	16.9	12.2
B7	5.76	0.54	50	1.0	7.1	1280	2.7	3.2	6.6	13.7	1330.0	17.4	13.7
B8	4.93	0.52	50	1.0	7.4	1100	3.9	3.9	4.6	12.0	1150.0	16.4	12.0
B9	2.81	0.52	50	1.0	7.3	850	2.0	2.8	5.0	12.4	900.0	15.0	12.4
B10	0.65	0.92	50	1.0	2.2	630	2.5	3.2	3.3	5.5	680.0	13.8	5.5
B11	0.84	0.92	50	1.0	2.2	750	2.3	3.0	4.2	6.4	800.0	14.4	6.4
C1	7.47	0.47	50	1.0	8.0	2100	5.0	4.5	7.8	15.8	2150.0	21.9	15.8

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Trails at Crowfoot

Project Name: Trails at Crowfoot

Project No. 254103

Calculated By: MRS

SUB-BASIN DATA			INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)				Tc CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (AC)	C_5	L (FT)	S (%)	T_i (MIN)	L (FT)	S (%)	VEL. (FPS)	T_t (MIN)	COMP. T_c (MIN)	TOTAL LENGTH(FT)	MIN. T_c (MIN)	T_c (MIN)
D1	5.94	0.44	50	1.0	8.4	1150	4.9	4.4	4.4	12.8	1200.0	16.7	12.8
D2	5.33	0.50	50	1.0	7.7	800	5.3	4.6	2.9	10.6	850.0	14.7	10.6
D3	5.67	0.56	50	1.0	6.9	730	1.1	2.0	6.1	13.0	780.0	14.3	13.0
D4	2.91	0.45	50	1.0	8.3	890	1.0	2.0	7.4	15.7	940.0	15.2	15.2
D5	9.10	0.61	50	1.0	6.2	1570	2.4	3.0	8.6	14.8	1620.0	19.0	14.8
D6	2.57	0.48	50	1.0	7.8	950	2.4	3.1	5.1	13.0	1000.0	15.6	13.0
D7	2.58	0.47	50	1.0	8.1	620	5.0	4.5	2.3	10.4	670.0	13.7	10.4
D8	0.85	0.55	50	1.0	7.0	250	1.2	2.2	1.9	8.9	300.0	11.7	8.9
D9	4.62	0.49	50	1.0	7.8	700	1.3	2.2	5.3	13.1	750.0	14.2	13.1
D10	4.80	0.53	50	1.0	7.3	680	3.8	3.9	2.9	10.2	730.0	14.1	10.2
D11	3.29	0.92	50	1.0	2.2	1375	2.3	3.0	7.7	10.0	1425.0	17.9	10.0
D12	1.13	0.92	50	1.0	2.2	1376	2.3	3.0	7.7	10.0	1426.0	17.9	10.0
E1	15.28	0.46	50	1.0	8.1	1380	3.8	3.8	6.0	14.1	1430.0	17.9	14.1
E2	5.27	0.56	50	1.0	6.9	1800	0.5	1.4	21.2	28.1	1850.0	20.3	20.3
E3	4.77	0.56	50	1.0	6.9	900	2.7	3.2	4.7	11.6	950.0	15.3	11.6
E4	3.14	0.55	50	1.0	6.9	710	2.8	3.3	3.5	10.5	760.0	14.2	10.5
E5	5.49	0.52	50	1.0	7.3	850	2.0	2.8	5.0	12.3	900.0	15.0	12.3
E6	5.54	0.55	50	1.0	6.9	750	4.4	4.1	3.0	10.0	800.0	14.4	10.0
E7	5.53	0.56	50	1.0	6.9	1080	2.5	3.2	5.7	12.6	1130.0	16.3	12.6
E8	4.73	0.57	50	1.0	6.8	750	2.8	3.3	3.7	10.5	800.0	14.4	10.5
E9	4.84	0.44	50	1.0	8.5	709	1.3	2.2	5.4	13.8	759.0	14.2	13.8
E10	0.70	0.62	50	1.0	6.1	710	1.3	2.2	5.4	11.5	760.0	14.2	11.5
E11	3.99	0.30	50	1.0	10.2	711	1.3	2.2	5.4	15.7	761.0	14.2	14.2
E12	7.51	0.30	50	1.0	10.2	712	1.3	2.2	5.4	15.7	762.0	14.2	14.2
E13	4.45	0.30	50	1.0	10.2	713	1.3	2.2	5.4	15.7	763.0	14.2	14.2
E14	11.27	0.53	50	1.0	7.3	714	1.3	2.2	5.4	12.7	764.0	14.2	12.7

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Trails at Crowfoot

Project Name: Trails at Crowfoot

Project No. 254103

Calculated By: MRS

SUB-BASIN DATA			INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)				Tc CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (AC)	C_5	L (FT)	S (%)	T_i (MIN)	L (FT)	S (%)	VEL. (FPS)	T_t (MIN)	COMP. T_c (MIN)	TOTAL LENGTH(FT)	MIN. T_c (MIN)	T_c (MIN)
F1	5.16	0.89	50	1.0	2.7	715	1.3	2.2	5.4	8.2	765.0	14.3	8.2
F2	2.99	0.88	50	1.0	2.8	716	1.3	2.2	5.4	8.2	766.0	14.3	8.2
F3	3.60	0.26	50	1.0	10.6	717	1.3	2.2	5.5	16.1	767.0	14.3	14.3
F4	7.36	0.57	50	1.0	6.8	718	1.3	2.2	5.5	12.3	768.0	14.3	12.3
F5	4.58	0.52	50	1.0	7.4	719	1.3	2.2	5.5	12.9	769.0	14.3	12.9
F6	5.67	0.44	50	1.0	8.4	720	1.3	2.2	5.5	13.9	770.0	14.3	13.9
F7	2.98	0.36	50	1.0	9.5	721	1.2	2.2	5.5	14.9	771.0	14.3	14.3
F8	8.58	0.38	50	1.0	9.2	722	1.2	2.2	5.5	14.7	772.0	14.3	14.3
F9	1.28	0.80	50	1.0	3.8	1708	3.2	3.6	8.0	11.7	1758.0	19.8	11.7
E15	1.95	0.55	50	1.0	7.0	723	1.2	2.2	5.5	12.5	773.0	14.3	12.5
E16	1.57	0.92	50	1.0	2.2	724	1.2	2.2	5.5	7.7	774.0	14.3	7.7
E17	1.55	0.92	50	1.0	2.2	725	1.2	2.2	5.5	7.7	775.0	14.3	7.7
OS1	4.98	0.05	300	3.1	22.7	515	3.1	3.5	2.5	25.1	815.0	14.5	14.5
OS2	10.95	0.05	300	1.2	30.8	1334	1.2	2.2	10.1	40.9	1634.0	19.1	19.1
OS3	7.81	0.05	300	2.9	23.2	1037	2.9	3.3	5.2	28.4	1337.0	17.4	17.4
OS4	2.22	0.05	300	3.5	21.7	730	3.5	3.7	3.3	24.9	1030.0	15.7	15.7

NOTES:

$T_i = (1.8*(1.1 - C_5)*(L)^0.5)/(S^0.33)$

T_t=L/60V (Velocity From Fig. 3-2)

T_c Check = 10+L/180

II-B. MINOR AND MAJOR STORM RUNOFF COMPUTATIONS FOR DEVELOPED RUNOFF CONDITIONS

1. SF-2 2-Year Sub-Basin Rational Method Calculations
2. SF-2 100-Year Sub-Basin Rational Method Calculations
3. Pond Calculations

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

Subdivision Trails at Crowfoot

Design Storm 2 Yr
2-Year P1 = 0.99 in.

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
A1	1A	A1	4.86	0.51	12.5	2.48	2.4	6.1				12.5	2.48	2.4	6.1									
A1,11	1K	A11	6.12	0.51	13.1	3.11	2.4	7.5	2@ 10' Type R On-Grade Inlet	12.8	0.0	14.4	5.59	2.3	12.8	3.0	6.1				410.0	3.46	2.0	Street Flow to 1K
A,2,11	1B	A2	4.37	0.53	10.4	2.32	2.6	6.1	1 @ 15' Type R Sump Inlet	6.1	0.0	15.2	7.91	2.2	17.7			12.8	2.00	18.00	340.0	7.60	0.7	Piped to 1B
A,2,3,11	1C	A3	3.23	0.49	13.6	1.58	2.4	3.7	1 @ 15' Type R Sump Inlet	3.7	0.0	15.3	9.49	2.2	21.1			17.7	2.00	24.00	40.0	8.30	0.1	Piped to 1C
A13	1M	A13	7.08	0.48	13.4	3.38	2.4	8.0	1 @ 15' Type R On-Grade Inlet	8.0	0.0	13.4	3.38	2.4	8.0			21.1	2.00	36.00	500.0	8.30	1.0	Piped to Channel
A7	1G	A7	6.37	0.53	10.5	3.37	2.6	8.9				10.5	3.37	2.6	8.9			8.0	4.00	18.00	820.0	8.30	1.6	Piped to 1E
A5,7,13	1E	A5	4.10	0.51	11.7	2.10	2.5	5.3	2 @ 15' Type R Sump Inlet	13.4	0.0	12.4 15.1	5.47 8.86	2.5 2.2	13.4 19.9	5.0	8.9				500.0	4.47	1.9	Street Flow to 1E
A4,5,7,13	1D	A4	4.07	0.36	14.3	1.45	2.3	3.3	1 @ 10' Type R Sump Inlet	3.3	0.0	15.4	10.31	2.2	22.9			19.9	1.00	24.00	80.0	4.00	0.3	Piped to 1D
A4,5,7,8,13	1H	A8	2.86	0.53	10.2	1.52	2.7	4.0	1 @ 10' Type R Sump Inlet	4.0	0.0	15.5	11.83	2.2	26.2			22.9	1.00	24.00	40.0	6.00	0.1	Piped to 1H
A12	1L	A12	5.98	0.51	11.9	3.03	2.5	7.5	1 @ 15' Type R On-Grade Inlet	7.5	0.0	11.9	3.03	2.5	7.5			26.2	1.50	42.00	150.0	6.30	0.4	Piped to Channel
A6,12	1F	A6	4.96	0.35	13.8	1.76	2.3	4.1	1 @ 15' Type R On-Grade Inlet	4.1	0.0	13.8	4.79	2.3	11.2			7.5	1.00	18.00	80.0	6.30	0.2	Piped to 1F
A6,12,14	1N	A14	1.43	0.52	8.7	0.75	2.8	2.1	2 @ 10' Type R Sump Inlet	2.1	0.0	14.1	5.53	2.3	12.8			11.2	1.00	24.00	160.0	9.10	0.3	Piped to 1N
A15	1O	A15	7.15	0.32	16.1	2.27	2.2	4.9	1 @ 10' Type R Sump Inlet	4.9	0.0	16.1	2.27	2.2	4.9			12.8	2.50	36.00	405.0	9.10	0.7	Piped to Channel
A9	1I	A9	3.44	0.53	11.6	1.83	2.5	4.6				11.6	1.83	2.5	4.6			4.9	0.50	24.00	100.0	9.10	0.2	Piped to 1J
A9,10,15	1J	A10	0.72	0.65	7.2	0.47	3.0	1.4	1 @ 10' Type R Sump Inlet	5.5	0.0	13.143 16.3	2.29 4.57	2.4 2.2	5.5 9.9	4.0	4.6				370.0	4.00	1.5	Street Flow to 1J
B3	2C	B3	4.92	0.52	10.8	2.57	2.6	6.7				10.8	2.57	2.6	6.7			9.9	1.50	24.00	10.0	9.10	0.0	Piped to Channel
B3,4	2D	B4	4.03	0.86	7.0	3.46	3.0	10.5	2 @ 15' Type R On-Grade Inlet	15.6	0.0	11.0	6.03	2.6	15.6	5.0	6.7				35.0	4.47	0.1	Street Flow to 2D
B2	2B	B2	3.13	0.52	9.5	1.61	2.7	4.4				9.5	1.61	2.7	4.4			15.6	3.00	18.00	1037.0	9.10	1.9	Piped to 2H
B1	2A	B1	21.00	0.24	19.8	5.06	2.0	9.9	2 @ 15' Type R On-Grade Inlet	13.1	0.0	19.8	6.68	2.0	13.1	4.0	4.4				1230.0	4.00	5.1	Street Flow to 2A
B1,2.5	2E	B5	6.38	0.54	12.2	3.45	2.5	8.5	2 @ 15' Type R On-Grade Inlet	8.5	0.0	20.7	10.12	1.9	19.4			13.1	3.50	30.00	245.0	4.70	0.9	Piped to 2E
B1,2,5,6	2F	B6	6.38	0.54	12.2	3.45	2.5	8.5	2 @ 15' Type R On-Grade Inlet	8.5	0.0	21.6	13.57	1.9	25.4			19.4	3.50	30.00	245.0	4.70	0.9	Piped to 2F
B7	2G	B7	5.76	0.51	13.7	2.94	2.3	6.9				13.7	2.94	2.3	6.9			25.4	3.50	30.00	263.0	4.70	0.9	Piped to 2H
B7,9	2I	B9	2.81	0.49	12.4	1.38	2.5	3.4	2 @ 15' Type R Sump Inlet	9.9	0.0	14.4	4.32	2.3	9.9	6.0	6.9				200.0	4.90	0.7	Street Flow to 2I
B1,2,5,6,7,8,9	2H	B8	4.93	0.49	12.0	2.42	2.5	6.0	2 @ 10' Type R Sump Inlet	6.0	0.0	22.5	21.93	1.8	40.1			9.9	0.50	24.00	130.0	5.70	0.4	Piped to 2H
B1,2,3,4,5,6,7,8,9,10	2J	B10	0.65	0.89	5.5	0.58	3.3	1.9	1 @ 15' Type R Sump Inlet	1.9	0.0	24.4	22.51	1.8	39.4			40.1	2.20	42.00	635.0	5.70	1.9	Piped to 2J
B1,2,3,4,5,6,7,8,9,10,11	2K	B11	0.84	0.89	6.4	0.75	3.1	2.3	1 @ 5' Type R Sump Inlet	2.3	0.0	24.5	23.26	1.7	40.6			39.4	2.20	42.00	30.0	4.70	0.1	Piped to 2K
C1	3A	C1	7.47	0.44	15.8	3.29	2.2	7.2	1 @ 15' Type R On-Grade Inlet	2.0	0.0	15.8	3.29	2.2	7.2			40.6	2.00	48.00	50.0	4.70	0.2	Piped to Channel
																		7.2	0.50	18.00	952.0	4.70	3.4	Piped to Channel through Swale

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

Subdivision Trails at Crowfoot

Design Storm 2 Yr
2-Year P1 = 0.99 in.

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
D2	4B	D2	5.33	0.47	10.6	2.49	2.6	6.5				10.6	2.49	2.6	6.5									
D2, D10	4J	D10	4.80	0.49	10.2	2.37	2.7	6.3	2 @ 10' Type R Sump Inlet	12.1	0.0	11.9	4.86	2.5	12.1	6.00	6.5				375.0	4.90	1.3	Street Flow to 4J
D7	4G	D7	2.58	0.44	10.4	1.12	2.6	3.0	2 @ 10' Type R On-Grade Inlet	3.0	0.0	10.4	1.12	2.6	3.0			12.1	1.00	36.00	20.0	5.80	0.1	Piped to DP 1
D2,7,10	1											11.9	5.99	2.5	14.9			3.0	2.00	18.00	630.0	9.10	1.2	Piped to DP 1
D1,2,7,10	4A	D1	5.94	0.41	12.8	2.44	2.4	5.9	1 @ 15' Type R Sump Inlet	5.9	0.0	12.8	8.42	2.4	20.4			14.9	2.00	24.00	20.0	9.10	0.0	Piped to 4A
D8	4H	D8	0.85	0.52	8.9	0.44	2.8	1.2				8.9	0.44	2.8	1.2			20.4	2.00	36.00	30.0	4.50	0.1	Piped to Pond B
D12	4L	D12	1.13	0.89	10.0	1.01	2.7	2.7	1 @ 15' Type R On-Grade Inlet	2.7	0.0	10.0	1.01	2.7	2.7	5.00	1.2				980.0	4.47	3.7	Street Flow to 4F
D6,8,12	4F	D6	2.57	0.45	13.0	1.17	2.4	2.8	2 @ 10' Type R On-Grade Inlet	3.0	0.0	13.0	2.61	2.4	6.3			2.7	1.50	18.00	100.0	4.50	0.4	Piped to 4F
																		6.3	0.50	18.00	200.0	5.20	0.6	Piped to Pond B

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Project Name: Trails at Crowfoot
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Subdivision Trails at Crowfoot

Design Storm 2 Yr
2-Year P1 = 0.99 in.

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
D3	4C	D3	5.67	0.52	13.0	2.97	2.4	7.1				13.0	2.97	2.4	7.1									
																5.0	7.1				50.0	4.47	0.2	Street Flow to 4D
D3,4	4D	D4	2.91	0.42	15.2	1.22	2.2	2.7				15.2	4.19	2.2	9.4									
																3.0	9.4				1668.0	3.46	8.0	Street Flow to 4E
D3,4,5	4E	D5	9.10	0.59	14.8	5.33	2.3	12.0	2 @ 15' Type R Sump Inlet	17.1	0.0	23.2	9.52	1.8	17.1									
																		17.1	1.00	24.00	30.0	5.20	0.1	Piped to DP 2
D9	4I	D9	4.62	0.46	13.1	2.11	2.4	5.0	1 @ 10' Type R Sump Inlet	5.0	0.0	13.1	2.11	2.4	5.0									
																		5.0	1.00	24.00	30.0	5.20	0.1	Piped to DP 2
D3,4,5,9	2											23.3	11.63	1.8	20.8									
																		20.8	2.50	36.00	204.0	5.20	0.7	Piped to DP 3
D11	4K	D11	3.29	0.89	10.0	2.93	2.7	7.9	1 @ 10' Type R Sump Inlet	7.9	0.0	10.0	2.93	2.7	7.9									
																		7.9	1.00	24.00	30.0	5.20	0.1	Piped to DP 3
D3,4,5,9,11	3											24.0	14.56	1.8	25.7									
																		25.7	2.50	36.00	400.0	5.20	1.3	Piped to DP 6
E1	5A	E1	15.28	0.43	14.1	6.65	2.3	15.4	2 @ 15' Type R On-Grade Inlet	14.8	0.6	14.1	0.26	2.3	0.6									
													6.39	2.3	14.8	2.74	0.6				573.0	3.29	2.9	Street Flow to 5C
		50% of carryover flow from 5A ~ 0.6/2 = 0.3 CFS							1 @ 15' Type R									14.8	1.00	24.00	573.0	7.60	1.3	Piped to 5C
E3, E1	5C	E3	4.77	0.53	11.6	2.50	2.5	6.3	On-Grade Inlet	6.6	0.0	15.3	9.15	2.2	20.4									
																		20.4	1.00	24.00	1197.0	8.18	2.4	Piped to 5E
E4	5D	E4	3.14	0.52	10.5	1.64	2.6	4.3				10.5	1.64	2.6	4.3									
												15.3	4.35	2.2	9.7						956.0	3.29	4.8	Street Flow to 5E
E1,3,4,5	5E	E5	5.49	0.49	12.3	2.70	2.5	6.6	2 @ 15' Type R Sump Inlet	9.7	0.0	17.8	13.49	2.1	27.9	2.74	4.3							
																		27.9	0.50	24.00	30.0	5.30	0.1	Piped to DP 4
E10,E2	5B	E2	5.27	0.53	20.3	2.77	1.9	5.4	1 @ 10' Type R Sump Inlet	6.6	0.0	20.3	3.19	1.9	6.2									
																		6.2	0.50	24.00	30.0	5.30	0.1	Piped to DP 4
E14	5N	E14	11.27	0.50	12.7	5.63	2.4	13.7	2 @ 10' Type R Sump Inlet	13.7	0.0	12.7	5.63	2.4	13.7									
																		13.7	4.50	30.00	970.0	14.70	1.1	Piped to 5X
E11	5K	E11	3.99	0.27	14.2	1.08	2.3	2.5				14.2	1.08	2.3	2.5									
																1.0	2.5				10.0	2.00	0.1	Overland to 5X
E12	5L	E12	7.51	0.27	14.2	2.03	2.3	4.7				14.2	2.03	2.3	4.7									
												14.317	4.50	2.3	10.3	1.0	4.7				10.0	2.00	0.1	Overland to 5X
E11,12,14,16	5X	E16	1.57	0.89	7.7	1.40	2.9	4.1	1 @ 15' Type R On-Grade Inlet	10.3	0.0	14.3	10.14	2.3	23.3									
																		23.3	2.50	30.00	329.0	11.30	0.5	Piped to 5F
E6,11,12,14,16	5F	E6	5.54	0.52	10.0	2.89	2.7	7.8	1 @ 15' Type R On-Grade Inlet	7.8	0.0	14.8	13.03	2.3	29.5									
																		29.5	2.50	30.00	288.0	12.30	0.4	Piped to 5H
E6,8,11,12,14,16	5H	E8	4.73	0.54	10.5	2.53	2.6	6.7	1 @ 15' Type R On-Grade Inlet	6.7	0.0	15.2	15.57	2.2	34.8									
																		34.8	1.00	30.00	660.0	12.90	0.9	Piped to DP 4
E9	5I	E9	4.84	0.41	13.8	1.97	2.3	4.6	1 @ 15' Type R On-Grade Inlet	4.6	0.0	13.8	1.97	2.3	4.6									
																		4.6	1.00	18.00	30.0	5.70	0.1	Piped to DP 4
E10	5J	E10	0.70	0.59	11.5	0.41	2.5	1.0				11.5	0.41	2.5	1.0									
																1.0	1.0				300.0	2.00	2.5	Street Flow to 5B
E15	5W	E15	1.95	0.52	12.5	1.01	2.4	2.5				12.5	1.01	2.4	2.5									
																1.0	2.5				500.0	2.00	4.2	Overland to 5G
E7,15	5G	E7	5.53	0.53	12.6	2.91	2.4	7.1	2 @ 15' Type R Sump Inlet	8.4	0.0	16.7	3.92	2.1	8.4									
																		8.4	1.00	42.00	30.0	6.60	0.1	Piped to DP 5
E6,8,9,10,11,12,14,16	4											16.0	17.94	2.2	39.1									
																		39.1	1.00	30.00	280.0	9.40	0.5	Piped to DP 5
E1,2,3,4,5,6,7,8,9,10,11 E12,14,15,16	5											20.4	38.54	1.9	74.3									
																		74.3	0.50	24.00	280.0	5.30	0.9	Piped to Pond

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

Subdivision Trails at Crowfoot

Design Storm 2 Yr
2-Year P1 = 0.99 in.

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
F2	6B	F2	2.99	0.85	8.2	2.55	2.9	7.3	1 @ 15' Type R On-Grade Inlet	7.3		8.2	2.55	2.9	7.3									
F1,F2	6A	F1	5.16	0.86	8.2	4.42	2.9	12.8	2 @ 15' Type R On-Grade Inlet	12.8	0.0	9.4	6.96	2.7	19.1			7.3	1.50	18.00	530.0	7.46	1.2	Piped to 6A
D3,4,5,9,11,F1,F2	6											25.3	21.52	1.7	36.9			19.1	1.50	24.00	100.0	6.50	0.3	Piped to DP 6
F4	6D	F4	7.36	0.53	12.3	3.93	2.5	9.7				12.3	3.93	2.5	9.7			36.9	1.50	42.00	1025.0	6.50	2.6	Piped to 6E
F3,4	6C	F3	3.60	0.24	14.3	0.85	2.3	2.0	2 @ 15' Type R On-Grade Inlet	11.0	0.0	14.3	4.78	2.3	11.0	1.0	9.7				50.0	2.00	0.4	Street Flow to 6C
F1,2,3,4,5	6E	F5	4.58	0.48	12.9	2.22	2.4	5.3	1 @ 15' Type R On-Grade Inlet	5.3	0.0	27.9	28.52	1.6	46.2			11.0	1.50	18.00	200.0	8.10	0.4	Piped to 6E
F1,2,3,4,5,9	6I	F9	1.28	0.77	11.7	0.99	2.5	2.5	2 @ 15' Type R Sump Inlet	2.5	0.0	31.0	29.51	1.5	45.0			46.2	1.50	42.00	1200.0	6.50	3.1	Piped to 6I
F1,2,3,4,5,7,9	6G	F7	2.98	0.33	14.3	0.98	2.3	2.3	1 @ 10' Type R Sump Inlet	2.3	0.0	31.5	30.49	1.5	46.0			45.0	0.50	18.00	200.0	6.50	0.5	Piped to 6G
E13	5M	E13	4.45	0.27	14.2	1.20	2.3	2.8				14.2	1.20	2.3	2.8			46.0	0.50	18.00	200.0	6.50	0.5	Piped to Pond
E17	5Y	E17	1.55	0.89	7.7	1.38	2.9	4.1	1 @ 10' Type R On-Grade Inlet	5.8	0.0	15.1	2.58	2.2	5.8	1.0	2.8				100.0	2.00	0.8	Street Flow to 5Y
F8	6H	F8	8.58	0.35	14.3	3.00	2.3	6.9	2 @ 10' Type R Sump Inlet	9.8	0.0	15.3	5.58	2.2	12.4			5.8	0.50	18.00	70.0	6.50	0.2	Piped/Overland Flow to 6H
F6	6F	F6	5.67	0.41	13.9	2.31	2.3	5.4	1 @ 10' Type R Sump Inlet	4.9	0.0	15.3	7.89	2.2	17.6			12.4	0.50	48.00	30.0	9.10	0.1	Piped to 6F
																		17.6	0.50	48.00	102.0	9.10	0.2	Piped to Channel

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision Trails at Crowfoot

Design Storm 100 Yr
100-Year P1 = 2.6 in.

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
A1	1A	A1	4.86	0.75	12.5	3.64	6.4	23.4				12.5	3.64	6.4	23.4									
A1,11	1K	A11	6.12	0.75	13.1	4.58	6.3	28.8				14.4	8.22	6.0	49.4	3.0	23.4				410.0	3.46	2.0	Street Flow to 1K
A,2,11	1B	A2	4.37	0.76	10.4	3.32	6.9	23.0	1 @ 15' Type R Sump Inlet	42.1	23.8	16.1	11.53	5.7	65.9	3.0	49.4				340.0	3.46	1.6	Street Flow to 1B
A,2,3,11	1C	A3	3.23	0.74	13.6	2.38	6.2	14.7	1 @ 15' Type R Sump Inlet	37.1	0.0	16.2	13.91	5.7	79.3			65.9	2.00	24.00	40.0	8.30	0.1	Piped to 1C
A13	1M	A13	7.08	0.73	13.4	5.19	6.2	32.2				13.4	5.19	6.2	32.2			79.3	2.00	36.00	500.0	14.90	0.6	Piped to Channel
A7	1G	A7	6.37	0.76	10.5	4.83	6.9	33.3				14.4	10.02	6.0	60.3	5.0	32.2				263.0	4.47	1.0	Street Flow to 1E
A5,7,13	1E	A5	4.10	0.75	11.7	3.08	6.6	20.3	2 @ 15' Type R Sump Inlet	74.4	0.0	16.3	13.10	5.7	74.4	5.0	60.3				500.0	4.47	1.9	Street Flow to 1E
A4,5,7,13	1D	A4	4.07	0.67	14.3	2.72	6.0	16.4	1 @ 10' Type R Sump Inlet	16.4	0.0	16.6	15.81	5.6	88.9			74.4	1.00	24.00	80.0	4.00	0.3	Piped to 1D
A4,5,7,8,13	1H	A8	2.86	0.76	10.2	2.17	7.0	15.2	1 @ 10' Type R Sump Inlet	15.2	0.0	16.7	17.99	5.6	100.8			88.9	1.00	24.00	40.0	6.00	0.1	Piped to 1H
A12	1L	A12	5.98	0.75	11.9	4.46	6.5	29.2				11.9	4.46	6.5	29.2			100.8	1.50	42.00	330.0	12.00	0.5	Piped to Channel
A6,12	1F	A6	4.96	0.67	13.8	3.31	6.1	20.3				13.8	7.77	6.1	47.7	1.5	29.2				88.0	2.45	0.6	Street Flow to 1F
A6,12,14	1N	A14	1.43	0.76	8.7	1.08	7.4	8.0	2 @ 10' Type R Sump Inlet	52.5	0.0	14.8	8.85	5.9	52.5	1.5	47.7				150.0	2.45	1.0	Street Flow to 1N
A15	1O	A15	7.15	0.65	16.1	4.63	5.7	26.4	1 @ 10' Type R Sump Inlet	26.4	0.0	16.1	4.63	5.7	26.4			52.5	2.50	30.00	330.0	13.37	0.4	Piped to Channel
A9	1I	A9	3.44	0.76	11.6	2.62	6.6	17.3				11.6	2.62	6.6	17.3			26.4	0.50	24.00	100.0	9.10	0.2	Piped to 1J
A9,10,15	1J	A10	0.72	0.82	7.2	0.59	7.9	4.7	1 @ 10' Type R Sump Inlet	20.1	0.0	13.143 16.3	3.21 7.84	6.3 5.7	20.1 44.5	4.0	17.3				370.0	4.00	1.5	Street Flow to 1J
B3	2C	B3	4.92	0.76	10.8	3.71	6.8	25.3				10.8	3.71	6.8	25.3			44.5	1.50	24.00	10.0	9.10	0.0	Piped to Channel
B3,4	2D	B4	4.03	0.94	7.0	3.80	8.0	30.3				11.0	7.51	6.8	50.9	5.0	25.3				35.0	4.47	0.1	Street Flow to 2D
B2	2B	B2	3.13	0.75	9.5	2.35	7.2	16.9				9.5	2.35	7.2	16.9	5.0	50.9				1605.0	4.47	6.0	Street Flow to 2J
B1	2A	B1	21.00	0.61	19.8	12.74	5.1	65.4	2 @ 15' Type R On-Grade Inlet	48.6	28.9	19.8	15.09	5.1	77.5	4.0	16.9				1230.0	4.00	5.1	Street Flow to 2A
B1,2.5	*50% of carryover flow from 2A ~ 28.9/2 = 14.5 CFS* 2E	B5	6.38	0.76	12.2	4.88	6.5	31.6	2 @ 15' Type R On-Grade Inlet	37.0	9.0	20.7	19.97	5.0	100.3			77.5	3.50	30.00	245.0	4.70	0.9	Piped to 2E
B1,2,5,6	*50% of carryover flow from 2E ~ 9/2 = 4.5 CFS* 2F	B6	6.38	0.76	12.2	4.88	6.5	31.6	2 @ 15' Type R On-Grade Inlet	31.8	4.3	21.6	24.85	4.9	122.1			100.3	3.50	30.00	245.0	4.70	0.9	Piped to 2F
B7	2G	B7	5.76	0.75	13.7	4.31	6.2	26.5				13.7	4.31	6.2	26.5			122.1	3.50	30.00	263.0	4.70	0.9	Piped to 2H
B7,9	2I	B9	2.81	0.74	12.4	2.08	6.4	13.4	2 @ 15' Type R Sump Inlet	38.4	0.0	14.4	6.39	6.0	38.4	6.0	26.5				200.0	4.90	0.7	Street Flow to 2I
B1,2,5,6,7,8,9	*50% of carryover flow from 2F ~ 4.3/2 = 2.15 CFS* 2H	B8	4.93	0.74	12.0	3.64	6.5	23.8	2 @ 10' Type R Sump Inlet	15.8	0.0	22.5	34.88	4.8	167.5			38.4	0.50	24.00	130.0	5.70	0.4	Piped to 2H
B1,2,3,4,5,6,7,8,9,10	2J	B10	0.65	0.95	5.5	0.62	8.6	5.3	1 @ 15' Type R Sump Inlet	45.2	0.0	16.946 24.4	8.13 43.01	5.6 4.6	45.2 197.7			167.5	2.20	42.00	635.0	5.70	1.9	Piped to 2J
B1,2,3,4,5,6,7,8,9,10,11	2K	B11	0.84	0.95	6.4	0.80	8.2	6.5	1 @ 5' Type R Sump Inlet	6.5	0.0	24.5	43.81	4.6	200.9			197.7	2.20	42.00	30.0	4.70	0.1	Piped to 2K
C1	3A	C1	7.47	0.71	15.8	5.32	5.8	30.6	1 @ 15' Type R On-Grade Inlet	2.0	0.0	15.8	5.32	5.8	30.6			200.9	2.00	48.00	50.0	4.70	0.2	Piped to Channel
																		30.6	0.50	18.00	952.0	4.70	3.4	Piped to Channel through Swale

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision Trails at Crowfoot

Design Storm 100 Yr
100-Year P1 = 2.6 in.

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
D2	4B	D2	135.0 5.33	0.73	10.6	3.87	6.9	26.6				10.6	3.87	6.9	26.6									
D2, D10	4J	D10	4.80	0.74	10.2	3.55	7.0	24.8	2 @ 10' Type R Sump Inlet	48.7	0.0	11.9	7.42	6.6	48.7	6.00	26.6				375.0	4.90	1.3	Street Flow to 4J
D7	4G	D7	2.58	0.71	10.4	1.83	6.9	12.7	2 @ 10' Type R On-Grade Inlet	12.7	0.0	10.4	1.83	6.9	12.7			48.7	1.00	36.00	20.0	5.80	0.1	Piped to DP 1
D2,7,10	1											11.9	9.25	6.5	60.5			12.7	2.00	18.00	630.0	9.10	1.2	Piped to DP 1
D1,2,7,10	4A	D1	5.94	0.70	12.8	4.13	6.4	26.3	1 @ 15' Type R Sump Inlet	5.9	0.0	12.8	13.39	6.4	85.0			60.5	2.00	24.00	20.0	9.10	0.0	Piped to 4A
D8	4H	D8	0.85	0.75	8.9	0.64	7.3	4.7				8.9	0.64	7.3	4.7			85.0	2.00	36.00	30.0	4.50	0.1	Piped to Pond B
D12	4L	D12	1.13	0.95	10.0	1.08	7.0	7.6	1 @ 15' Type R On-Grade Inlet	3.0	0.0	10.0	1.08	7.0	7.6	5.00	4.7				980.0	4.47	3.7	Street Flow to 4F
D6,8,12	4F	D6	2.57	0.72	13.0	1.85	6.3	11.7	2 @ 10' Type R On-Grade Inlet	3.0	0.0	13.0	3.56	6.3	22.5			7.6	1.50	18.00	100.0	4.50	0.4	Piped to 4F
																		22.5	0.50	18.00	200.0	5.20	0.6	Piped to Pond B

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
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Subdivision Trails at Crowfoot

Design Storm 100 Yr
100-Year P1 = 2.6 in.

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
D3	4C	D3	5.67	0.76	13.0	4.29	6.3	27.0				13.0	4.29	6.3	27.0									
																5.0	27.0				50.0	4.47	0.2	Street Flow to 4D
D3,4	4D	D4	2.91	0.70	15.2	2.04	5.9	12.0				15.2	6.33	5.9	37.1									
																3.0	37.1				1668.0	3.46	8.0	Street Flow to 4E
D3,4,5	4E	D5	9.10	0.79	14.8	7.23	5.9	42.9	2 @ 15' Type R Sump Inlet	63.9	0.0	23.2	13.55	4.7	63.9									
																		63.9	1.00	24.00	30.0	5.20	0.1	Piped to DP 2
D9	4I	D9	4.62	0.72	13.1	3.33	6.3	20.9	1 @ 10' Type R Sump Inlet	20.9	0.0	13.1	3.33	6.3	20.9									
																		20.9	1.00	24.00	30.0	5.20	0.1	Piped to DP 2
D3,4,5,9	2											23.3	16.88	4.7	79.5									
																		79.5	2.50	36.00	204.0	5.20	0.7	Piped to DP 3
D11	4K	D11	3.29	0.95	10.0	3.13	7.0	22.0	1 @ 10' Type R Sump Inlet	22.0	0.0	10.0	3.13	7.0	22.0									
																		22.0	1.00	24.00	30.0	5.20	0.1	Piped to DP 3
D3,4,5,9,11	3											24.0	20.01	4.6	92.8									
																		92.8	2.50	36.00	2621.0	5.20	8.4	Piped to DP 5
E1	5A	E1	15.28	0.71	14.1	10.86	6.1	66.0				14.1	10.86	6.1	66.0									
																2.7	66.0				573.0	3.29	2.9	Street Flow to 5C
E3, E1	5C	E3	4.77	0.76	11.6	3.60	6.6	23.9				17.0	14.46	5.6	80.4									
																2.7	80.4				290.0	3.29	1.5	Street Flow to 5D
E1,3,4	5D	E4	3.14	0.76	10.5	2.37	6.9	16.4				18.4	16.84	5.3	89.8									
																1.0	89.8				920.0	2.00	7.7	Piped to 5E
E1,3,4,5	5E	E5	5.49	0.74	12.3	4.06	6.4	26.2	2 @ 15' Type R Sump Inlet	86.1	6.3	26.1	20.89	4.4	92.4									
																		86.1	1.00	42.00	30.0	11.74	0.0	Piped to DP 5
E2, E10	*50% of carryover flow from 5E & 5G ~ 6.3/2+29.6/2 = 17.95 CFS* 5B	E2	5.27	0.76	20.3	3.99	5.1	20.3	1 @ 15' Type R Sump Inlet	37.3	0.0	26.5	8.50	4.4	37.3									
																		37.3	1.00	30.00	30.0	9.40	0.1	Piped to DP 5
E14	5N	E14	11.27	0.74	12.7	8.39	6.4	53.4	2 @ 10' Type R Sump Inlet	53.4	0.0	12.7	8.39	6.4	53.4									
																		53.4	1.50	30.00	2465.0	14.70	2.8	Piped to DP 5
E11	5K	E11	3.99	0.62	14.2	2.47	6.0	15.0				14.2	2.47	6.0	15.0									
																1.0	15.0				10.0	2.00	0.1	Street Flow to 5X
E12	5L	E12	7.51	0.62	14.2	4.66	6.0	28.2				14.2	4.66	6.0	28.2									
																1.0	28.2				10.0	2.00	0.1	Street Flow to 5X
E11,12,16	5X	E16	1.57	0.95	7.7	1.49	7.7	11.5				14.3	8.62	6.0	52.0									
																1.0	52.0				230.0	2.00	1.9	Street Flow to 5F
E6,11,12,16	5F	E6	5.54	0.76	10.0	4.18	7.0	29.5				16.2	12.80	5.7	72.8									
																1.0	72.8				288.0	2.00	2.4	Street Flow to 5H
E6,8,11,12,16	5H	E8	4.73	0.76	10.5	3.61	6.9	24.9				18.6	16.41	5.3	87.1									
																1.0	87.1				660.0	2.00	5.5	Street Flow DP 4
E9	5I	E9	4.84	0.69	13.8	3.35	6.1	20.5				13.8	3.35	6.1	20.5									
																1.0	20.5				30.0	2.00	0.3	Street Flow DP 4
E10	5J	E10	0.70	0.79	11.5	0.55	6.6	3.7				11.5	0.55	6.6	3.7									
																1.0	3.7				30.0	2.00	0.3	Street Flow DP 4
E6,8,9,10,11,12,14,16	4											24.1	20.32	4.6	93.9									
																1.0	93.9				280.0	2.00	2.3	Street Flow to 5G
E15	5W	E15	1.95	0.75	12.5	1.47	6.4	9.4				12.5	1.47	6.4	9.4									
																1.0	9.4				500.0	2.00	4.2	Street Flow to 5G
E7,15	5G	E7	5.53	0.76	12.6	4.18	6.4	26.7	2 @ 15' Type R Sump Inlet	86.1	27.8	26.5	19.63	4.4	86.1									
												26.5	25.97	4.4	113.9									
E1,2,3,4,5,6,7,8,9,10,11 E12,14,15,16	5											26.5	63.75	4.4	279.4									
																		86.1	1.00	42.00	30.0	11.70	0.0	Piped to DP 5
																		279.4	1.00	60.00	280.0	15.00	0.3	Piped to Pond

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision Trails at Crowfoot

Design Storm 100 Yr
100-Year P1 = 2.6 in.

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
F2	6B	F2	2.99	0.94	8.2	2.81	7.6	21.3				8.2	2.81	7.6	21.3									
																1.0	21.3				530.0	2.50	3.5	Street Flow to 6A
F1,F2	6A	F1	5.16	0.94	8.2	4.86	7.6	36.9				11.7	7.67	6.6	50.5									
																1.0	50.5				1025.0	2.50	6.8	Street Flow to 6E
F4	6D	F4	7.36	0.76	12.3	5.60	6.5	36.2				12.3	5.60	6.5	36.2									
																1.0	36.2				50.0	3.00	0.3	Street Flow to 6C
F3,4	6C	F3	3.60	0.60	14.3	2.17	6.0	13.1				14.3	7.78	6.0	47.0									
																1.0	47.0				200.0	2.00	1.7	Street Flow to 6E
F1,2,3,4,5	6E	F5	4.58	0.74	12.9	3.37	6.3	21.3				18.6	18.81	5.3	100.0									
																5.0	100.0				1200.0	4.47	4.5	Street Flow to 6I
F1,2,3,4,5,9	6I	F9	1.28	0.89	11.7	1.14	6.6	7.5	2 @ 15' Type R Sump Inlet	86.1	8.5	23.0	19.95	4.7	94.6									
F1,2,3,4,5,9 D3,4,5,9,11	6											32.4	39.96	3.9	155.7			94.6	0.50	18.00	200.0	6.50	0.5	Piped to 6G
D3,4,5,9,11	*50% of carryover flow from 6I= 8.5/2 = 4.25 CFS*								1 @ 10' Type R Sump Inlet	16.0	0.0							155.7	1.00	54.00	75.0	13.67	0.1	Piped to 6G
F1,2,3,4,5,7,9	6G	F7	2.98	0.65	14.3	1.95	6.0	11.8				32.5	41.91	3.9	163.0									
																		163.0	1.00	54.00	150.0	6.50	0.4	Piped to Channel
E13	5M	E13	4.45	0.62	14.2	2.76	6.0	16.7				14.2	2.76	6.0	16.7									
																1.0	16.7				100.0	2.00	0.8	Street Flow to 5Y
E17	5Y	E17	1.55	0.95	7.7	1.47	7.7	11.4				15.1	4.23	5.9	24.9									
																1.0	24.9				1124.0	2.00	9.4	Street Flow to 6H
F8	6H	F8	8.58	0.66	14.3	5.70	6.0	34.4	2 @ 10' Type R Sump Inlet	34.4	0.0	24.4	9.93	4.6	45.6									
F6	6F	F6	5.67	0.69	13.9	3.94	6.1	24.1	1 @ 10' Type R Sump Inlet	24.1	0.0	24.5	13.87	4.6	63.5			45.6	0.50	48.00	30.0	9.10	0.1	Piped to 6F
																		63.5	1.00	36.00	34.0	9.10	0.1	Piped to Channel

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision Trails at Crowfoot

Design Storm 100 Yr
100-Year P1 = 2.6 in.

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

	DIRECT RUNOFF								TOTAL RUNOFF							STREET		PIPE			TRAVEL TIME			
COMBINED BASINS	Design Point	Area Design.	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Inlet Type	Q (Intercept)	Q (Carry-On)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
WEST CHANNEL																								
C1,OS1	A	OS1	4.98	0.49	14.5	2.44	6.0	14.6				19.2	7.76	5.2	40.5			40.5	1.00	36.00	530.0	9.10	1.0	Piped to Pond
C1,OS1,B1-B11	B											24.7	51.57	4.6	235.5	0.5	235.5				1360.0	1.41	16.0	Channel Flow to C
C1,OS1-OS2,B1-B11,A1,2,3,11	C	OS2	10.95	0.49	19.1	5.37	5.2	28.1				40.7	70.85	3.4	240.0			240.0	1.00	36.00	530.0	6.00	1.5	Channel Flow to D
A4,5,7,8,13 C1,OS1-OS2,B1-B11,A1,2,3,11	D											42.2	88.84	3.3	294.2			294.2	1.00	36.00	800.0	6.00	2.2	Channel Flow to E
A1-A15,B1-B11,C1 OS1-OS3	E	OS3	7.81	0.49	17.4	3.83	5.5	21.0				61.8	109.35	2.6	281.7			281.7	1.00	36.00	530.0	9.10	1.0	Piped to Pond
EAST CHANNEL																								
D3,4,5,9,11 F1,2,3,4,5,7,9	F											32.9	41.91	3.9	161.9			161.9	1.00	36.00	530.0	9.10	1.0	Channel Flow to G
D3,4,5,9,11 F1-9	G											33.8	55.77	3.8	211.7			211.7	1.00	36.00	300.0	9.10	0.5	Channel Flow to F
D3,4,5,9,11 F1-9, OS 4	H	OS4	2.22	0.49	15.7	1.09	5.8	6.3				34.4	56.86	3.8	213.7			213.7	1.00	36.00	300.0	9.10	0.5	Channel Flow to Pond

POND RELEASE RATES

Subdivision: Trails at Crowfoot

Project Name: Trails at Crowfoot
Project No. 254103
Calculated By: MRS

POND A

	Area (a)	158.78	acres			
	Slope	3.1	%			
	L^2 / Area	4.75				
	Unit Discharge @ 3% Slope for Type B Soil	1.24	/acre	Unit Discharge @ 3% Slope for Type C/D Soil	1.35	/acre
	Unit Discharge @ 4% Slope for Type B Soil	1.31	/acre	Unit Discharge @ 4% Slope for Type C/D Soil	1.42	/acre
	Interpolated Unit Discharge	1.25	/acre	Interpolated Unit Discharge	1.36	/acre
	Total Interpolated Unit Discharge	1.30				
POND A	Release Rate	186.06	CFS			

POND B

	Area (a)	23.2	acres			
	Slope	5.7	%			
	L^2 / Area	2.27				
	Unit Discharge > 4% Slope for Type B Soil	1.45	/acre	Unit Discharge > 4% Slope for Type C/D Soil	1.57	/acre
	Total Interpolated Unit Discharge	1.51				
POND B	Release Rate	31.53	CFS			

POND C:

	Area (a)	160.87	acres			
	Slope	2.8	%			
	L^2 / Area	5.75				
	Unit Discharge @ 2% Slope for Type B Soil	1.16	/acre	Unit Discharge @ 2% Slope for Type C/D Soil	1.26	/acre
	Unit Discharge @ 3% Slope for Type B Soil	1.24	/acre	Unit Discharge @ 3% Slope for Type C/D Soil	1.35	/acre
	Interpolated Unit Discharge	1.22	/acre	Interpolated Unit Discharge	1.33	/acre
	Total Interpolated Unit Discharge	1.28				
POND C:	Release Rate	177.21	CFS			

DETENTION POND SIZING

Subdivision: Trails at Crowfoot

POND A

BASIN ID	AREA AC	% IMPERVIOUSNESS
A1	4.86	57.35
A2	4.37	59.60
A3	3.23	54.87
A4	4.07	40.00
A5	4.10	57.70
A6	4.96	39.75
A7	6.37	59.46
A8	2.86	59.77
A9	3.44	59.55
A10	0.72	72.78
A11	6.12	57.17
A12	5.98	56.93
A13	7.08	53.48
A14	1.43	58.80
A15	7.15	35.60
B1	21.00	26.95
B2	3.13	58.00
B3	4.92	58.68
B4	4.03	96.03
B5	6.38	60.78
B6	6.38	60.78
B7	5.76	57.40
B8	4.93	55.17
B9	2.81	55.27
B10	0.65	100.00
B11	0.84	100.00
C1	7.47	49.43
OS1	4.98	2.00
OS2	10.95	2.00
OS3	7.81	2.00
TOTAL	**158.78**	**44.10**

POND B

BASIN ID	AREA AC	% IMPERVIOUSNESS
D1	5.94	46.10
D2	5.33	52.47
D6	2.57	50.81
D7	2.58	48.95
D8	0.85	57.94
D10	4.80	55.62
D12	1.13	100.00
TOTAL	**23.20**	**53.43**

DETENTION POND SIZING

Subdivision: Trails at Crowfoot

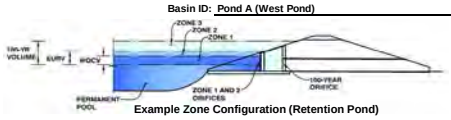
POND C

BASIN ID	AREA AC	% IMPERVIOUSNESS
D3	5.67	58.87
D4	2.91	47.23
D5	9.10	65.47
D9	4.62	51.08
D11	3.29	100.00
E1	15.28	48.76
E2	5.27	59.18
E3	4.77	59.05
E4	3.14	58.84
E5	5.49	55.38
E6	5.54	58.77
E7	5.53	59.21
E8	4.73	60.24
E9	4.84	45.65
E10	0.70	66.21
E11	3.99	30.00
E12	7.51	30.00
E13	4.45	30.00
E14	11.27	55.97
E15	1.95	58.08
E16	1.57	100.00
E17	1.55	100.00
F1	5.16	95.69
F2	2.99	95.35
F3	3.60	26.33
F4	7.36	60.07
F5	4.58	54.47
F6	5.67	45.69
F7	2.98	36.71
F8	8.58	39.26
F9	1.28	86.32
OS4	2.22	2.00
TOTAL	**157.58**	**54.47**

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

Project: Trails at Crowfoot

Basin ID: Pond A (West Pond)



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	158.78	acres
Watershed Length =	5,730	ft
Watershed Slope =	0.031	ft/ft
Watershed Imperviousness =	44.40%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	49.8%	percent
Percentage Hydrologic Soil Groups C/D =	50.2%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	Parker - Parker Town Court	
Water Quality Capture Volume (WQCV) =	2.532	acre-feet
Excess Urban Runoff Volume (EURV) =	7.034	acre-feet
2-yr Runoff Volume (P1 = 0.96 in.) =	5.104	acre-feet
5-yr Runoff Volume (P1 = 1.4 in.) =	9.590	acre-feet
10-yr Runoff Volume (P1 = 1.65 in.) =	12.803	acre-feet
25-yr Runoff Volume (P1 = 1.98 in.) =	18.037	acre-feet
50-yr Runoff Volume (P1 = 2.3 in.) =	22.332	acre-feet
100-yr Runoff Volume (P1 = 2.61 in.) =	27.139	acre-feet
500-yr Runoff Volume (P1 = 3.25 in.) =	36.318	acre-feet
Approximate 2-yr Detention Volume =	4.822	acre-feet
Approximate 5-yr Detention Volume =	8.049	acre-feet
Approximate 10-yr Detention Volume =	8.852	acre-feet
Approximate 25-yr Detention Volume =	10.027	acre-feet
Approximate 50-yr Detention Volume =	11.991	acre-feet
Approximate 100-yr Detention Volume =	14.853	acre-feet

Optional User Input 1-hr Precipitation

	inches
	inches
	inches
	inches
	inches
	inches

Stage-Storage Calculation

Zone 1 Volume (WQCV) =	2.532	acre-feet
Zone 2 Volume (EURV - Zone 1) =	4.502	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	7.819	acre-feet
Total Detention Basin Volume =	14.853	acre-feet
Initial Surcharge Volume (ISV) =	331	ft^3
Initial Surcharge Depth (ISD) =		ft
Total Available Detention Depth (H_{total}) =		ft
Depth of Trickle Channel (H_{TC}) =		ft
Slope of Trickle Channel (S_{TC}) =		ft/ft
Slopes of Main Basin Sides (S_{main}) =		H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =		
Initial Surcharge Area (A_{ISV}) =		ft^2
Surcharge Volume Length (L_{ISV}) =		ft
Surcharge Volume Width (W_{ISV}) =		ft
Depth of Basin Floor (H_{FLOOR}) =		ft
Length of Basin Floor (L_{FLOOR}) =		ft
Width of Basin Floor (W_{FLOOR}) =		ft
Area of Basin Floor (A_{FLOOR}) =		ft^2
Volume of Basin Floor (V_{FLOOR}) =		ft^3
Depth of Main Basin (H_{MAIN}) =		ft
Length of Main Basin (L_{MAIN}) =		ft
Width of Main Basin (W_{MAIN}) =		ft
Area of Main Basin (A_{MAIN}) =		ft^2
Volume of Main Basin (V_{MAIN}) =		ft^3
Calculated Total Basin Volume (V_{total}) =		acre-feet

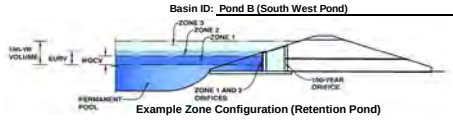
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Micropool									

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

Project: Trails at Crowfoot

Basin ID: Pond B (South West Pond)



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	23.20	acres
Watershed Length =	1,513	ft
Watershed Slope =	0.057	ft/ft
Watershed Imperviousness =	53.40%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	49.8%	percent
Percentage Hydrologic Soil Groups C/D =	50.2%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	Parker - Parker Town Court	
Water Quality Capture Volume (WQCV) =	0.417	acre-feet
Excess Urban Runoff Volume (EURV) =	1.255	acre-feet
2-yr Runoff Volume (P1 = 0.96 in.) =	0.919	acre-feet
5-yr Runoff Volume (P1 = 1.4 in.) =	1.616	acre-feet
10-yr Runoff Volume (P1 = 1.65 in.) =	2.095	acre-feet
25-yr Runoff Volume (P1 = 1.98 in.) =	2.848	acre-feet
50-yr Runoff Volume (P1 = 2.3 in.) =	3.486	acre-feet
100-yr Runoff Volume (P1 = 2.61 in.) =	4.186	acre-feet
500-yr Runoff Volume (P1 = 3.25 in.) =	5.534	acre-feet
Approximate 2-yr Detention Volume =	0.870	acre-feet
Approximate 5-yr Detention Volume =	1.398	acre-feet
Approximate 10-yr Detention Volume =	1.533	acre-feet
Approximate 25-yr Detention Volume =	1.713	acre-feet
Approximate 50-yr Detention Volume =	2.000	acre-feet
Approximate 100-yr Detention Volume =	2.413	acre-feet

Optional User Input 1-hr Precipitation

	inches
	inches
	inches
	inches
	inches
	inches

Stage-Storage Calculation

Zone 1 Volume (WQCV) =	0.417	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.837	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	1.159	acre-feet
Total Detention Basin Volume =	2.413	acre-feet
Initial Surcharge Volume (ISV) =	55	ft^3
Initial Surcharge Depth (ISD) =		ft
Total Available Detention Depth (H_{total}) =		ft
Depth of Trickle Channel (H_{TC}) =		ft
Slope of Trickle Channel (S_{TC}) =		ft/ft
Slopes of Main Basin Sides (S_{main}) =		H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =		
Initial Surcharge Area (A_{ISV}) =		ft^2
Surcharge Volume Length (L_{ISV}) =		ft
Surcharge Volume Width (W_{ISV}) =		ft
Depth of Basin Floor (H_{FLOOR}) =		ft
Length of Basin Floor (L_{FLOOR}) =		ft
Width of Basin Floor (W_{FLOOR}) =		ft
Area of Basin Floor (A_{FLOOR}) =		ft^2
Volume of Basin Floor (V_{FLOOR}) =		ft^3
Depth of Main Basin (H_{MAIN}) =		ft
Length of Main Basin (L_{MAIN}) =		ft
Width of Main Basin (W_{MAIN}) =		ft
Area of Main Basin (A_{MAIN}) =		ft^2
Volume of Main Basin (V_{MAIN}) =		ft^3
Calculated Total Basin Volume (V_{total}) =		acre-feet

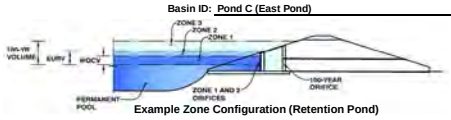
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Micropool									

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

Project: Trails at Crowfoot

Basin ID: Pond C (East Pond)



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	160.87	acres
Watershed Length =	6,347	ft
Watershed Slope =	0.028	ft/ft
Watershed Imperviousness =	53.43%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	49.8%	percent
Percentage Hydrologic Soil Groups C/D =	50.2%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	Parker - Parker Town Court	
Water Quality Capture Volume (WQCV) =	2.893	acre-feet
Excess Urban Runoff Volume (EURV) =	8.704	acre-feet
2-yr Runoff Volume (P1 = 0.96 in.) =	6.377	acre-feet
5-yr Runoff Volume (P1 = 1.4 in.) =	11.210	acre-feet
10-yr Runoff Volume (P1 = 1.65 in.) =	14.529	acre-feet
25-yr Runoff Volume (P1 = 1.98 in.) =	19.755	acre-feet
50-yr Runoff Volume (P1 = 2.3 in.) =	24.179	acre-feet
100-yr Runoff Volume (P1 = 2.61 in.) =	29.031	acre-feet
500-yr Runoff Volume (P1 = 3.25 in.) =	38.377	acre-feet
Approximate 2-yr Detention Volume =	6.036	acre-feet
Approximate 5-yr Detention Volume =	9.701	acre-feet
Approximate 10-yr Detention Volume =	10.637	acre-feet
Approximate 25-yr Detention Volume =	11.881	acre-feet
Approximate 50-yr Detention Volume =	13.873	acre-feet
Approximate 100-yr Detention Volume =	16.738	acre-feet

Optional User Input 1-hr Precipitation

	inches
	inches
	inches
	inches
	inches
	inches

Stage-Storage Calculation

Zone 1 Volume (WQCV) =	2.893	acre-feet
Zone 2 Volume (EURV - Zone 1) =	5.810	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	8.034	acre-feet
Total Detention Basin Volume =	16.738	acre-feet
Initial Surcharge Volume (ISV) =	378	ft^3
Initial Surcharge Depth (ISD) =		ft
Total Available Detention Depth (H_{total}) =		ft
Depth of Trickle Channel (H_{TC}) =		ft
Slope of Trickle Channel (S_{TC}) =		ft/ft
Slopes of Main Basin Sides (S_{main}) =		H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =		

Initial Surcharge Area (A_{ISV}) =		ft^2
Surcharge Volume Length (L_{ISV}) =		ft
Surcharge Volume Width (W_{ISV}) =		ft
Depth of Basin Floor (H_{FLOOR}) =		ft
Length of Basin Floor (L_{FLOOR}) =		ft
Width of Basin Floor (W_{FLOOR}) =		ft
Area of Basin Floor (A_{FLOOR}) =		ft^2
Volume of Basin Floor (V_{FLOOR}) =		ft^3
Depth of Main Basin (H_{MAIN}) =		ft
Length of Main Basin (L_{MAIN}) =		ft
Width of Main Basin (W_{MAIN}) =		ft
Area of Main Basin (A_{MAIN}) =		ft^2
Volume of Main Basin (V_{MAIN}) =		ft^3
Calculated Total Basin Volume (V_{total}) =		acre-feet

Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Micropool									

III. Hydraulic Computations

A. CUHP & SWMM
B. UD-INLET
C. UD-SEWER
D. UD-CHANNEL
E. UD-CULVERT

III. Hydraulic Computations

A. CUHP & SWMM

Summary of CUHP Input Parameters (Version 1.4.4)

								Depression		Horton's Infiltration			DCIA Level and Fractions			
Catchment Name/ID	SWMM Node/ID	Raingage Name/ID	Area (sq.mi.)	Dist. to Centroid (miles)	Length (miles)	Slope (ft./ft.)	Percent Imperv.	Pervious (inches)	Imperv. (inches)	Initial Rate (in./hr.)	Final Rate (in.hr.)	Decay Coeff. (1/sec.)	DCIA Level	Dir. Con'ct Imperv. Fraction	Receiv. Perv. Fraction	Percent Eff. Imperv.
A	A	100 YEAR	0.248	0.673	1.085	0.031	44.4	0.35	0.10	3.75	0.55	0.0018	0.00	0.82	0.21	43.32
B	B	100 YEAR	0.036	0.128	0.287	0.057	53.4	0.35	0.10	3.75	0.55	0.0018	0.00	0.87	0.24	52.41
C	C	100 YEAR	0.251	0.553	1.202	0.028	53.4	0.35	0.10	3.75	0.55	0.0018	0.00	0.87	0.24	52.44

Summary of Unit Hydrograph Parameters Used By Program and Calculated Results (Version 1.4.4)

	Unit Hydrograph Parameters and Results									Excess Precip.		Storm Hydrograph			
Catchment Name/ID	Ct	Cp	W50 (min.)	W50 Before Peak	W75 (min.)	W75 Before Peak	Time to Peak (min.)	Peak (cfs)	Volume (c.f)	Excess (inches)	Excess (c.f.)	Time to Peak (min.)	Peak Flow (cfs)	Total Volume (c.f.)	Runoff per Unit Area (cfs/acre)
A	0.092	0.453	18.8	6.56	9.8	4.39	13.4	397	576,371	2.05	1,178,980	40.0	447	1,177,638	2.81
B	0.158	0.391	7.7	2.70	4.0	1.81	6.4	141	84,216	2.16	181,851	35.0	106	185,481	4.58
C	0.087	0.523	15.1	5.29	7.9	3.54	12.6	499	583,958	2.16	1,261,186	40.0	542	1,268,430	3.37

```
EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.010)
--------------------------------------------------------------

WARNING 04: minimum elevation drop used for Conduit A_DL

***********************************************************
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
***********************************************************

****************
Analysis Options
****************
Flow Units ............... CFS
Process Models:
  Rainfall/Runoff ........ NO
  RDII ................... NO
  Snowmelt ............... NO
  Groundwater ............ NO
  Flow Routing ........... YES
  Ponding Allowed ........ NO
  Water Quality .......... NO
Flow Routing Method ...... KINWAVE
Starting Date ............ JAN-01-2005 00:00:00
Ending Date .............. JAN-02-2005 06:00:00
Antecedent Dry Days ...... 0.0
Report Time Step ......... 00:15:00
Routing Time Step ........ 30.00 sec
```

Flow Routing Continuity	Volume acre-feet	Volume 10^6 gal
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.000	0.000
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	60.410	19.685
External Outflow	60.387	19.678
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.038	

Highest Flow Instability Indexes

All links are stable.

Routing Time Step Summary

Minimum Time Step	:	30.00 sec
Average Time Step	:	30.00 sec
Maximum Time Step	:	30.00 sec
Percent in Steady State	:	0.00
Average Iterations per Step	:	1.00
Percent Not Converging	:	0.00

****
Node Depth Summary
****

Node	Type	Average Depth Feet	Maximum Depth Feet	Maximum HGL Feet	Time of Max Occurrence days hr:min	Reported Max Depth Feet
A	JUNCTION	0.00	0.00	6069.00	0 00:00	0.00
B	JUNCTION	0.00	0.00	6115.00	0 00:00	0.00
C	JUNCTION	0.00	0.00	5986.00	0 00:00	0.00
OUT-A	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OUT-B	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OUT-C	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
POND-A	STORAGE	0.62	8.84	6077.84	0 01:17	8.82
POND-B	STORAGE	0.32	6.10	6101.10	0 00:56	6.07
POND-C	STORAGE	0.69	8.30	5979.30	0 01:16	8.29

****
Node Inflow Summary
****

Node	Type	Maximum Lateral Inflow CFS	Maximum Total Inflow CFS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 gal	Total Inflow Volume 10^6 gal	Flow Balance Error Percent
A	JUNCTION	446.90	446.90	0 00:40	8.81	8.81	0.000
B	JUNCTION	106.28	106.28	0 00:35	1.39	1.39	0.000
C	JUNCTION	541.71	541.71	0 00:40	9.49	9.49	0.000
OUT-A	OUTFALL	0.00	178.79	0 01:17	0	8.81	0.000
OUT-B	OUTFALL	0.00	31.39	0 00:56	0	1.39	0.000
OUT-C	OUTFALL	0.00	170.34	0 01:16	0	9.48	0.000
POND-A	STORAGE	0.00	446.90	0 00:40	0	8.81	0.035
POND-B	STORAGE	0.00	106.28	0 00:35	0	1.39	0.045

POND-C	STORAGE	0.00	541.71	0	00:40	0	9.49	0.040	

****
Node Surcharge Summary
****

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Feet	Min. Depth Below Rim Feet
A	JUNCTION	30.00	0.000	0.000
B	JUNCTION	30.00	0.000	0.000
C	JUNCTION	30.00	0.000	0.000
POND-A	STORAGE	30.00	8.839	0.161
POND-B	STORAGE	30.00	6.095	2.905
POND-C	STORAGE	30.00	8.297	0.703

****
Node Flooding Summary
****

No nodes were flooded.

****
Storage Volume Summary
****

Storage Unit	Average Volume 1000 ft3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 ft3	Max Pcnt Full	Time of Max Occurrence days	Time of Max Occurrence hr:min	Maximum Outflow CFS
POND-A	37.295	6	0	0	595.546	98	0	01:17	178.79
POND-B	4.578	3	0	0	98.684	56	0	00:56	31.39
POND-C	55.123	7	0	0	733.812	90	0	01:16	170.34

****
Outfall Loading Summary
****

Outfall Node	Flow Freq Pcnt	Avg Flow CFS	Max Flow CFS	Total Volume 10^6 gal
OUT-A	74.64	14.60	178.79	8.806
OUT-B	26.89	6.38	31.39	1.387
OUT-C	99.53	11.80	170.34	9.484
System	67.02	32.78	378.67	19.676

****
Link Flow Summary
****

Link	Type	Maximum |Flow| CFS	Time of Max Occurrence days	hr:min	Maximum |Veloc| ft/sec	Max/ Full Flow	Max/ Full Depth
A_DL	DUMMY	446.90	0	00:40			
B_DL	DUMMY	106.28	0	00:35			
C_DL	DUMMY	541.71	0	00:40			
OUT_A	DUMMY	178.79	0	01:17			
OUT_B	DUMMY	31.39	0	00:56			
OUT_C	DUMMY	170.34	0	01:16			

****
Conduit Surcharge Summary
****

No conduits were surcharged.

Analysis begun on: Tue Oct 25 09:07:38 2016
Analysis ended on: Tue Oct 25 09:07:38 2016
Total elapsed time: < 1 sec

III. Hydraulic Computations

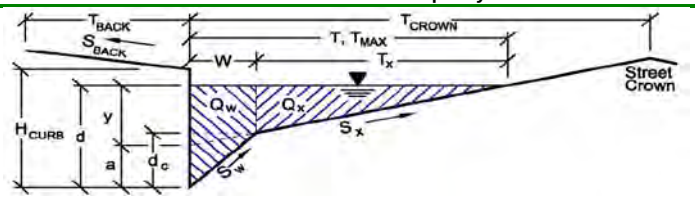
B. UD-Inlet

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 100 Year Arterial Capacity



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	19.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.50	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.030	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	5.6	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**15.0**	**112.0**	**cfs**

Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

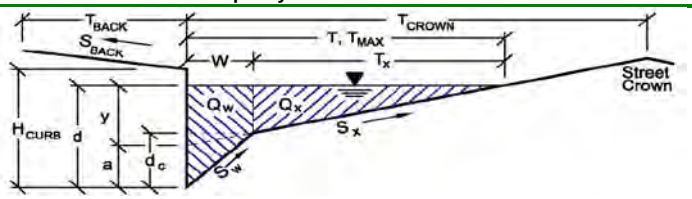
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 100 Year Capacity for Residential Boulevard Collector



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	13.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.50	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.030	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	5.6	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**15.0**	**109.3**	**cfs**

Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

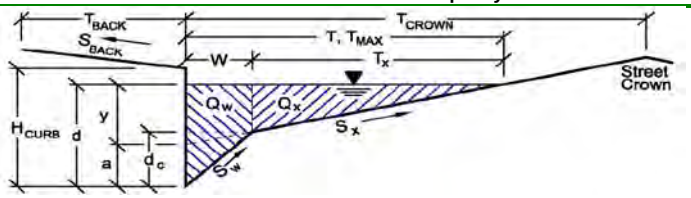
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 100 Year Local Street Capacity



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

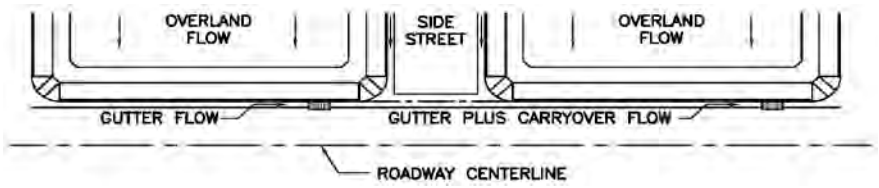
WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1H



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	6.1	65.9	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

Geographic Information: (Enter data in the blue cells):

FILL IN THE SECTIONS BELOW. <---

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

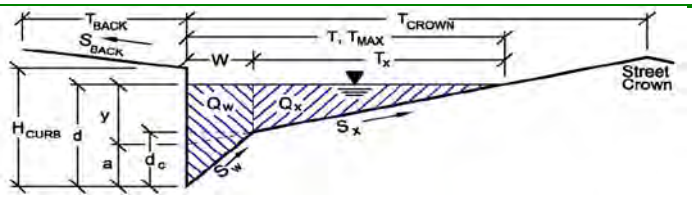
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	6.1	65.9	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1H



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

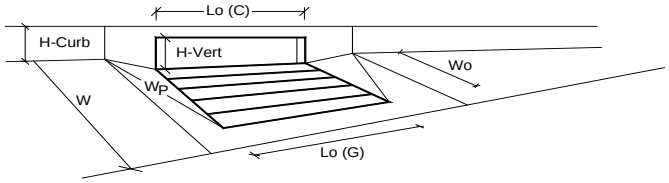
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1H

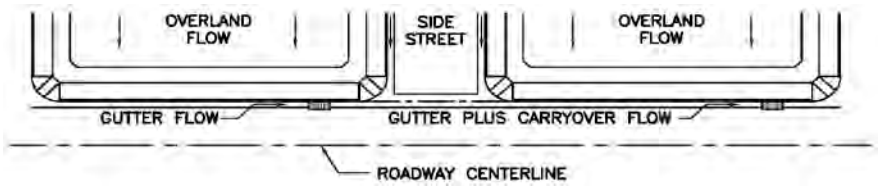


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**42.1**	**cfs**
WARNING: Inlet Capacity less than Q Peak for MAJOR Storm	$Q_{PEAK\ REQUIRED}$ =	6.1	65.9	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1C



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	3.7	14.7	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

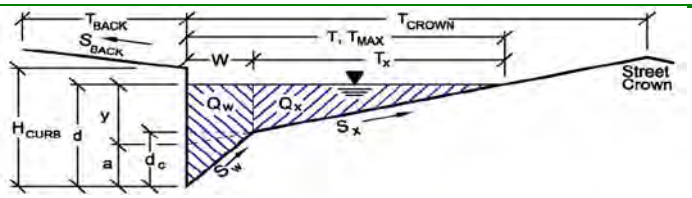
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	23.8	cfs
Total Design Peak Flow, Q =	3.7	38.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1C



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

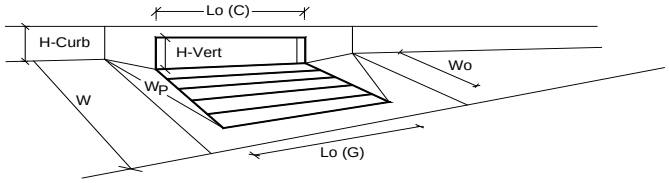
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1C

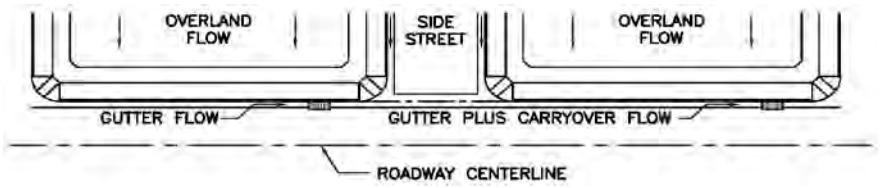


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**42.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	3.7	38.5	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1D



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	3.3	16.4	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

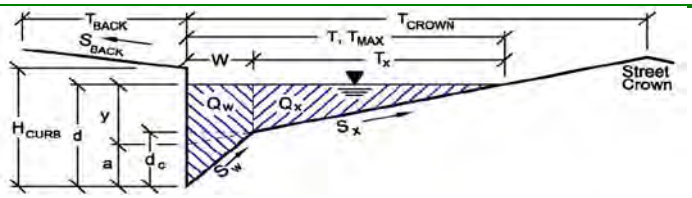
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	3.3	16.4	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1D



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

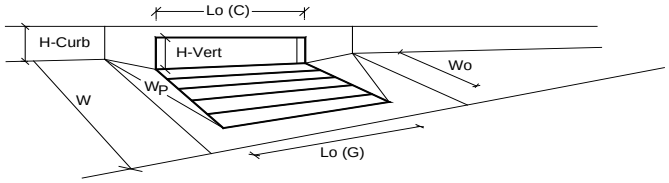
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1D

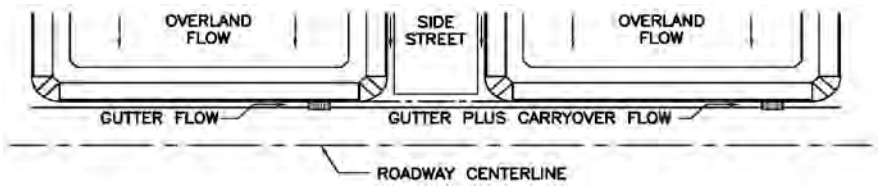


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	3.3	16.4	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1E



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	13.4	74.4	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

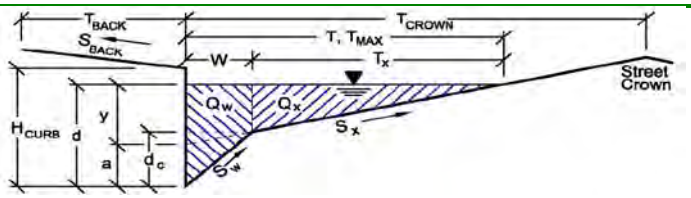
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	13.4	74.4	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1E



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

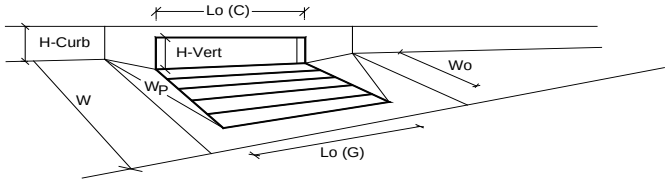
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1E

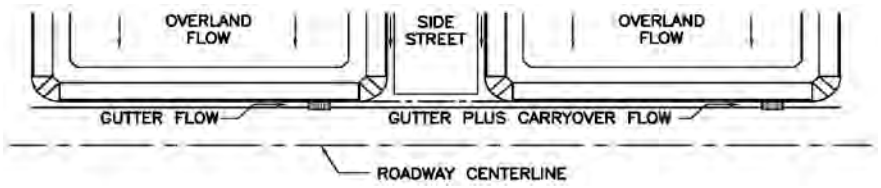


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**19.9**	**86.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	13.4	74.4	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1F



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	4.1	20.3	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

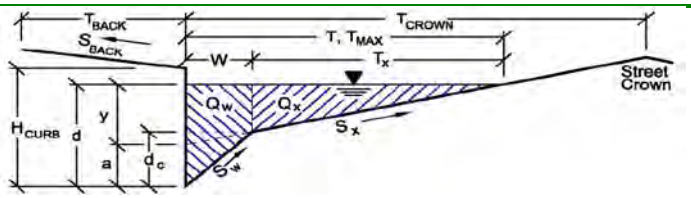
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	4.1	20.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1F



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

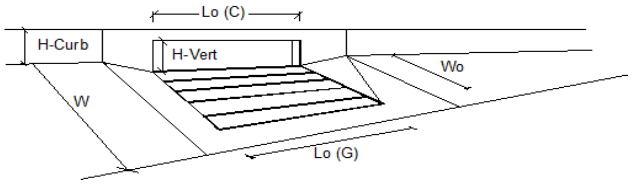
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 1F



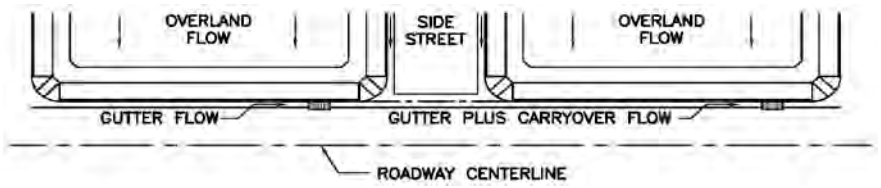
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < maximum allowable from sheet 'Q-Allow'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**4.10**	**10.63**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**9.7**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**52**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1H



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	4.0	15.2	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

FILL IN THE SECTIONS BELOW. <---

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

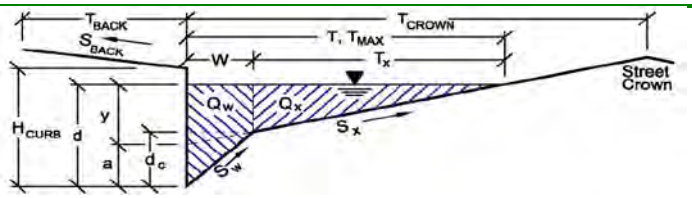
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	4.0	15.2	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1H



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

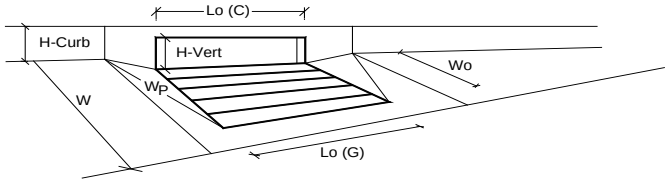
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1H

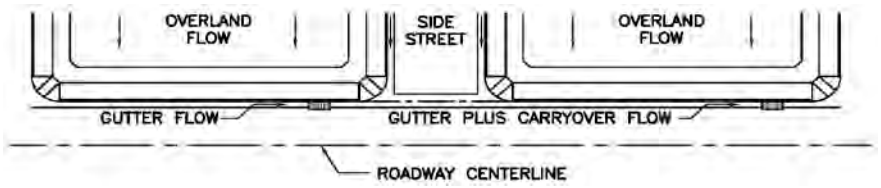


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	4.0	15.2	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 1J



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	5.5	20.1	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

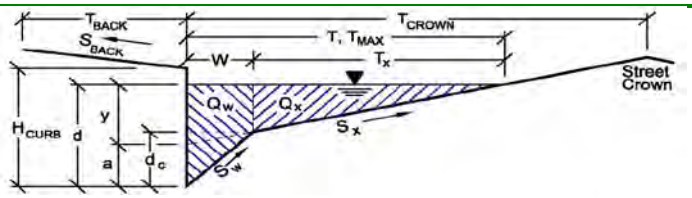
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	5.5	20.1	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 1J



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	19.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.50	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.040	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**13.8**	**102.8**	**cfs**

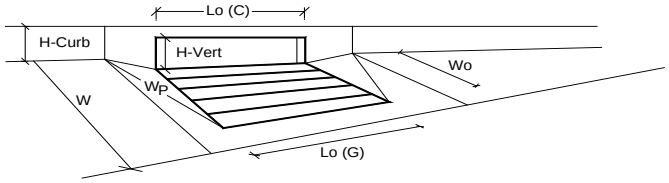
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 1J



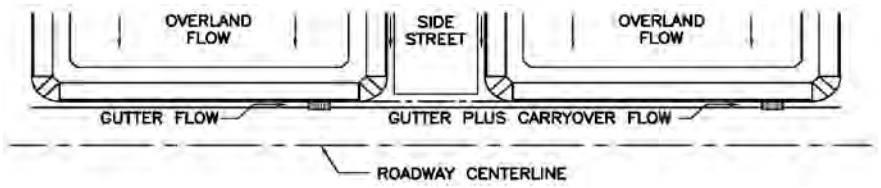
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.6	9.4	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Warning 1 Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.20	0.20	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**6.4**	**26.6**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	5.5	20.1	cfs

Warning 1: Dimension entered is not a typical dimension for inlet type specified.

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1K



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	12.8	28.8	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

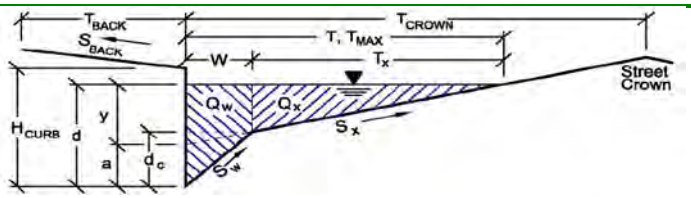
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	12.8	28.8	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1K



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.030	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

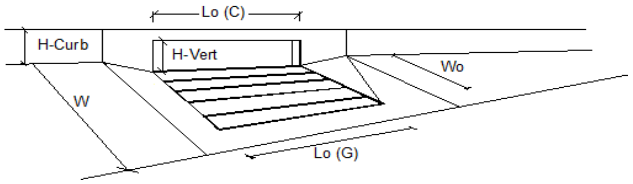
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**5.9**	**138.6**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 1K



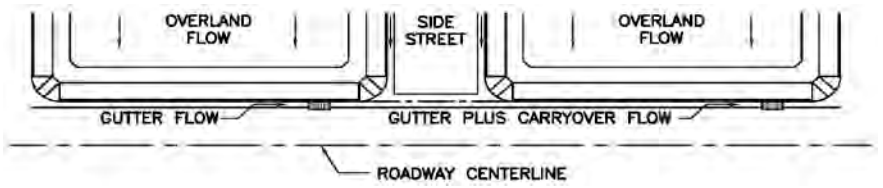
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**12.51**	**21.84**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.3**	**7.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**98**	**76**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1L



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	7.5	29.2	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

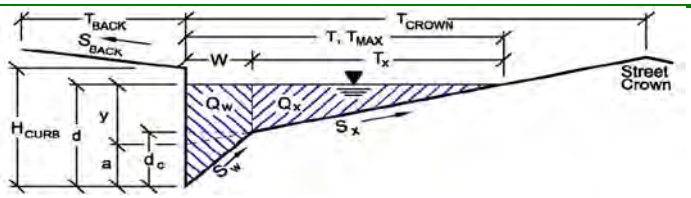
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	7.5	29.2	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1L



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

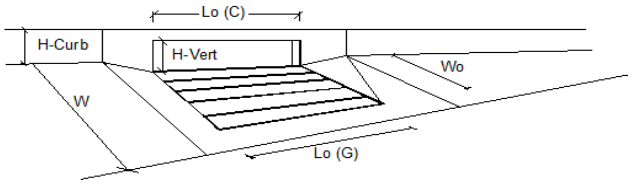
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 1L



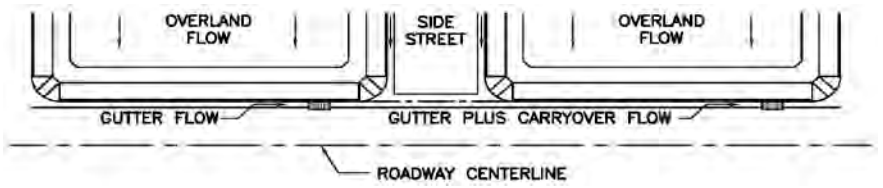
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.41**	**17.33**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.1**	**11.9**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**99**	**59**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1M



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	8.0	32.2	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

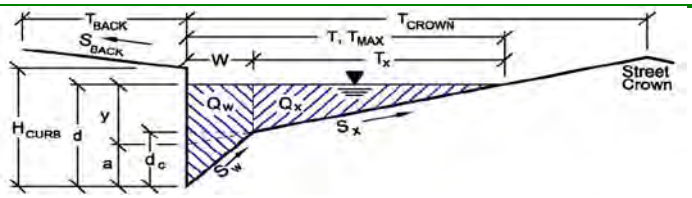
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	8.0	32.2	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1M



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.040	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

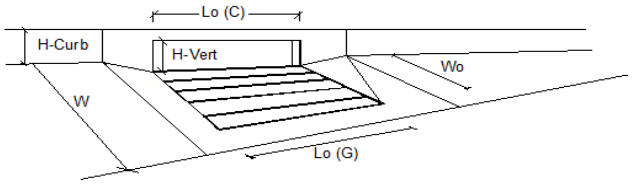
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**6.8**	**127.1**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 1M



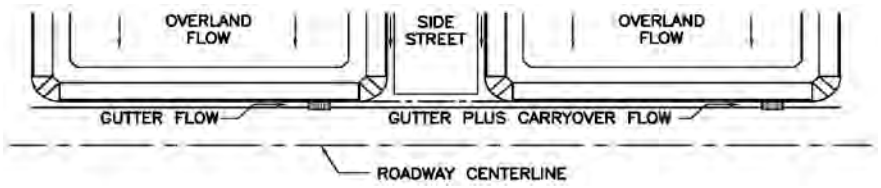
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**8.00**	**18.79**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**13.4**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**58**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1N



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	2.1	52.5	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

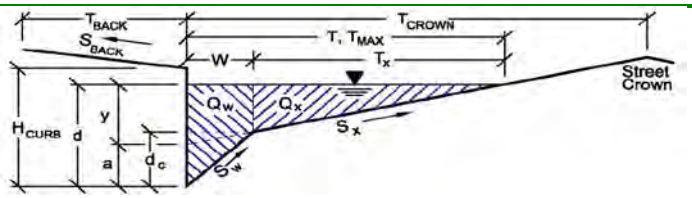
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	2.1	52.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1N



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

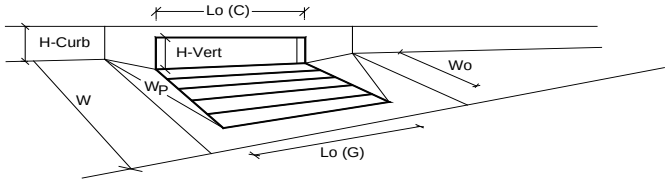
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1N

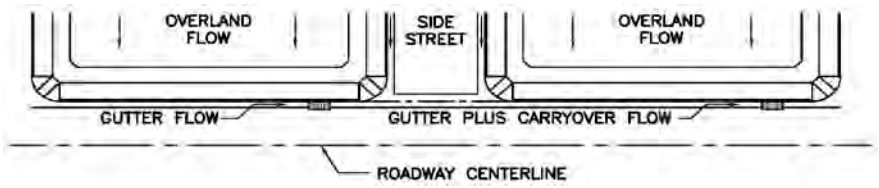


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**14.4**	**56.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	2.1	52.5	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 1O



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	4.9	26.4	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

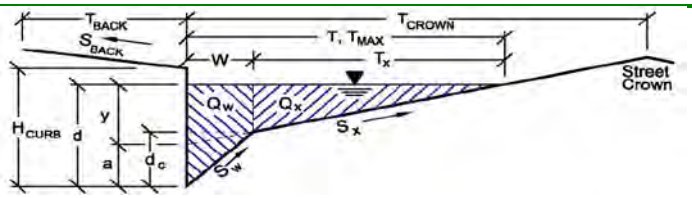
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	4.9	26.4	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 1O



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

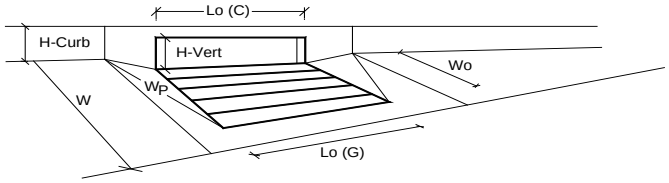
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = DP 1O

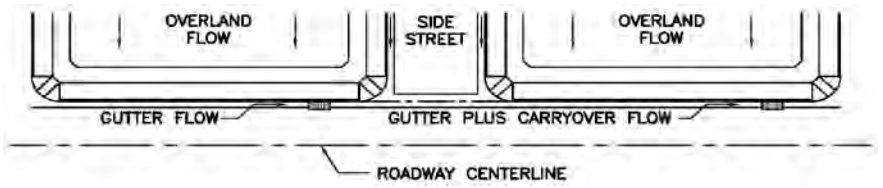


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	4.9	26.4	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2A



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	13.1	77.5	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

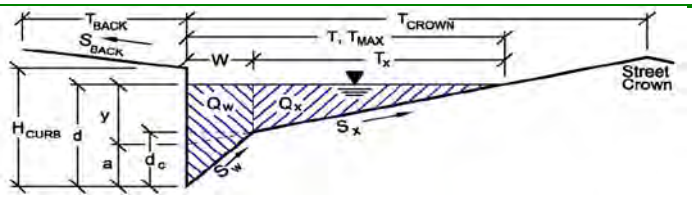
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	13.1	77.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2A



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.060	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

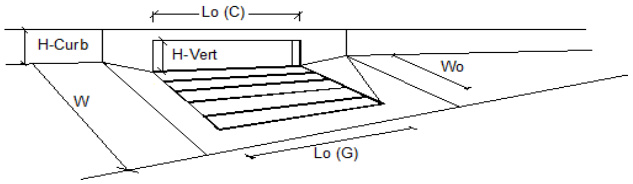
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**8.3**	**112.6**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: 2A



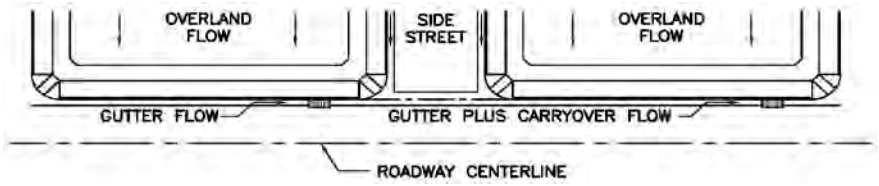
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**13.01**	**48.61**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.1**	**28.9**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**99**	**63**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2D



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	15.6	50.9	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

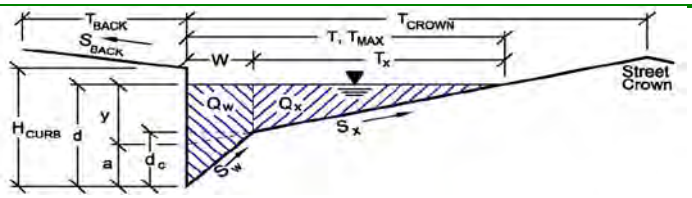
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	15.6	50.9	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2D



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.050	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

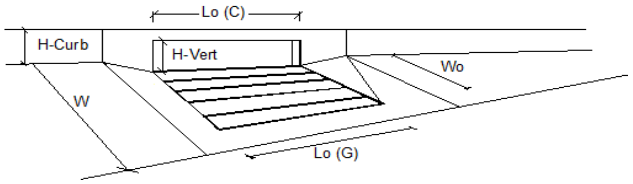
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**7.6**	**118.9**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: 2D



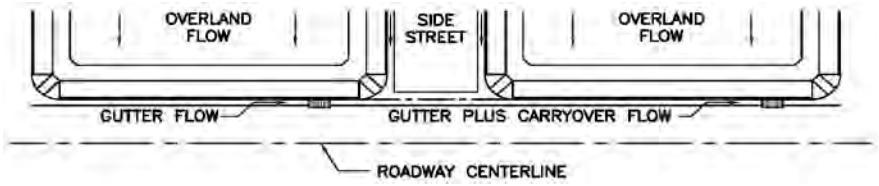
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**15.30**	**38.83**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.3**	**12.1**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**98**	**76**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2E



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	8.5	31.6	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

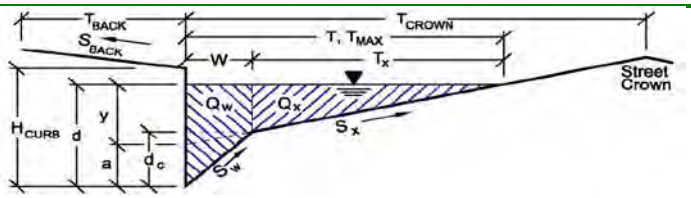
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	14.5	cfs
Total Design Peak Flow, Q =	8.5	46.1	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2E



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.060	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

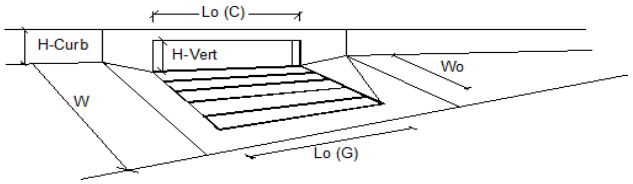
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**8.3**	**112.6**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: 2E



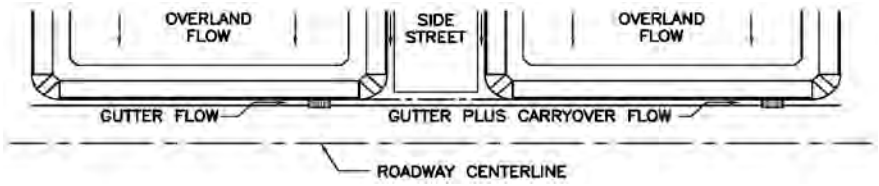
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**8.50**	**36.97**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**9.1**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**80**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2F



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	8.5	31.6	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

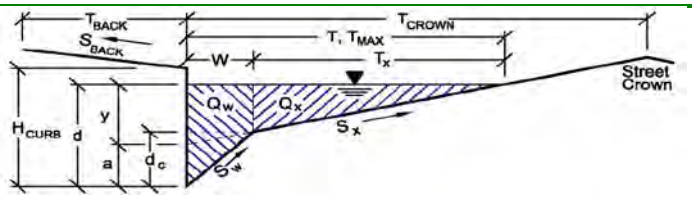
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	4.5	cfs
Total Design Peak Flow, Q =	8.5	36.1	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2F



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.060	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

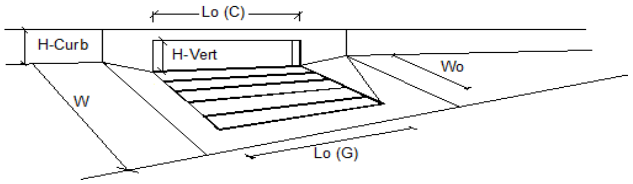
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**8.3**	**112.6**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: 2F



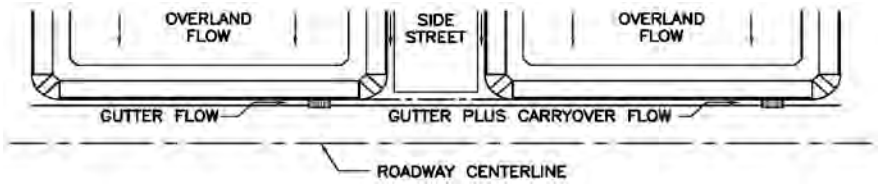
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**8.50**	**31.78**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**4.3**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**88**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2H



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	6.0	23.8	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

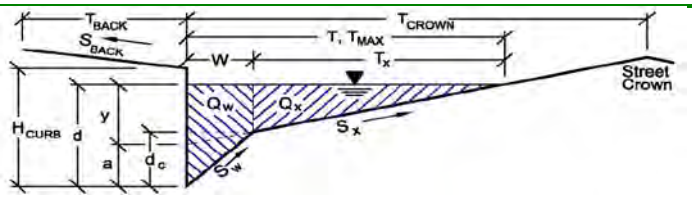
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	2.2	cfs
Total Design Peak Flow, Q =	6.0	26.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2H



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

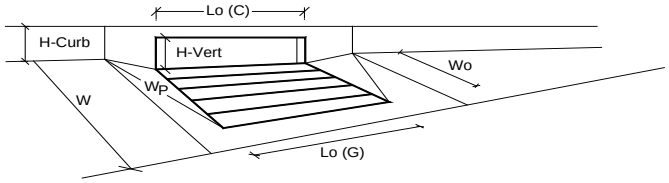
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 2H

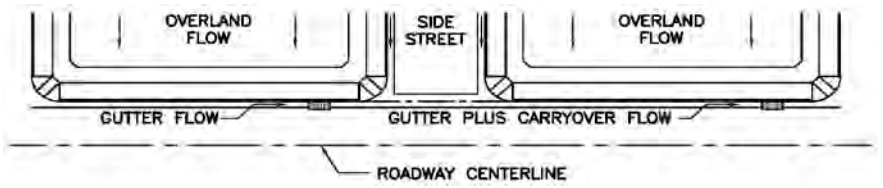


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	8.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**14.4**	**29.9**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	6.0	26.0	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2I



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	9.9	38.4	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

FILL IN THE SECTIONS BELOW. <---

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

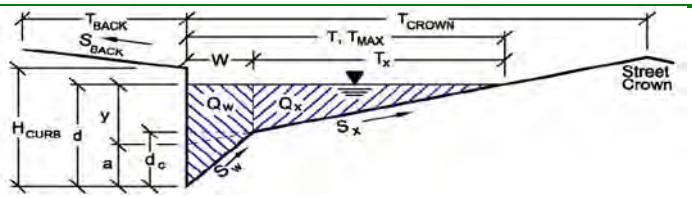
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	9.9	38.4	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2I



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

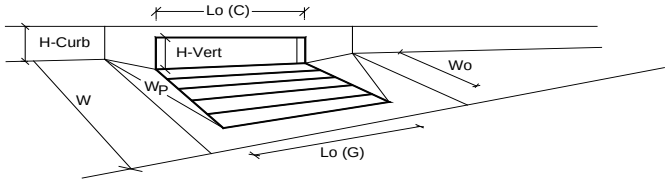
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 2I

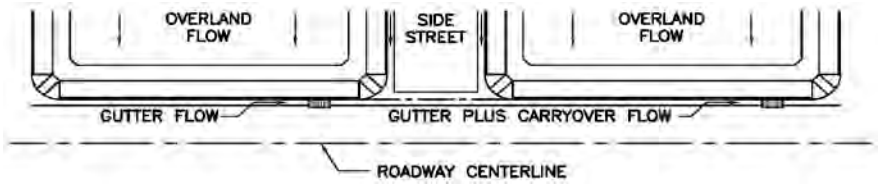


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	8.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**19.9**	**41.4**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	9.9	38.4	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2J



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	1.9	45.2	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

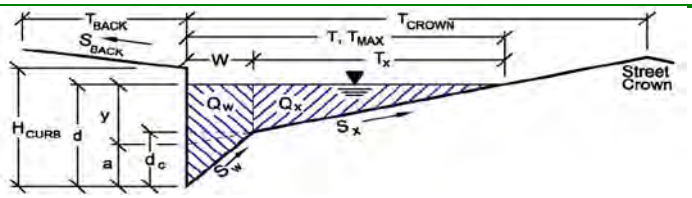
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	1.9	45.2	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2J



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

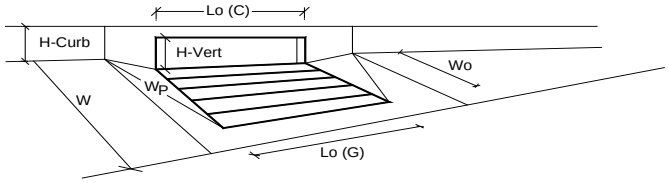
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 2J

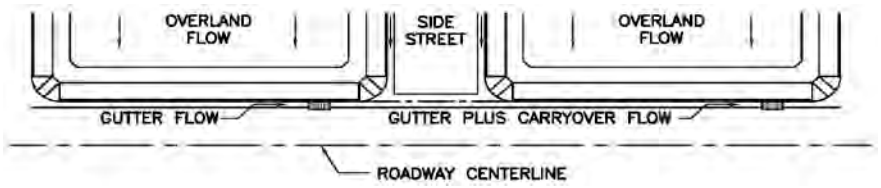


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**42.1**	**cfs**
WARNING: Inlet Capacity less than Q Peak for MAJOR Storm	$Q_{PEAK\ REQUIRED}$ =	1.9	45.2	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 2K



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^*Q_{Known}$ =	2.3	6.5	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

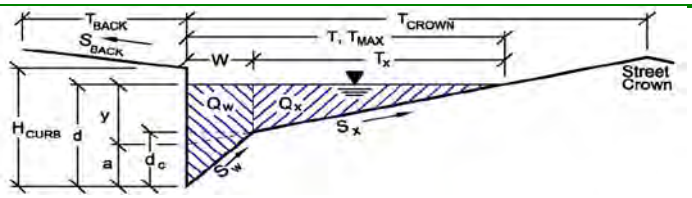
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	2.3	6.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 2K



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

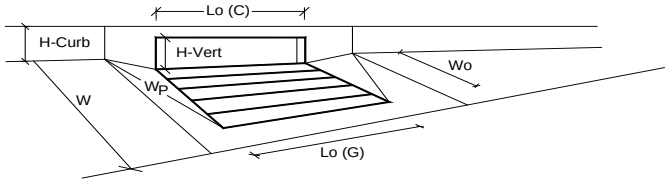
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 2K

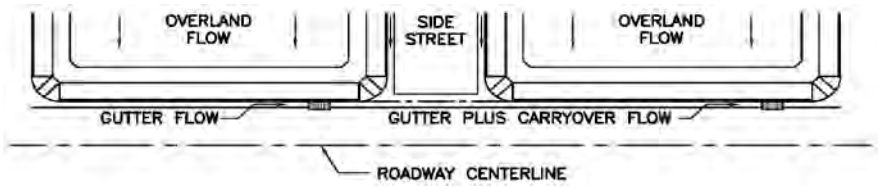


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**5.4**	**13.2**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	2.3	6.5	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 3A



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	7.2	30.6	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

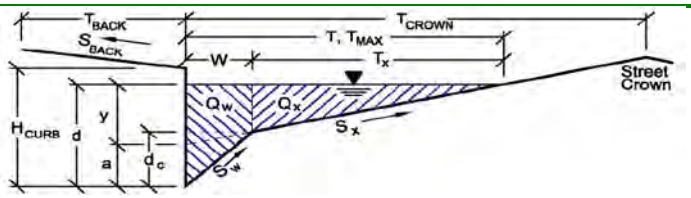
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	7.2	30.6	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 3A



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

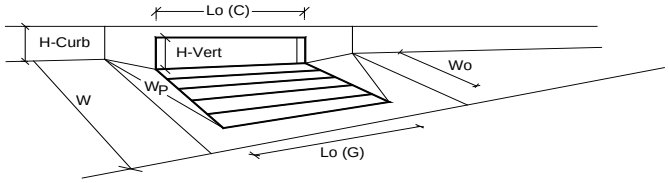
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 3A

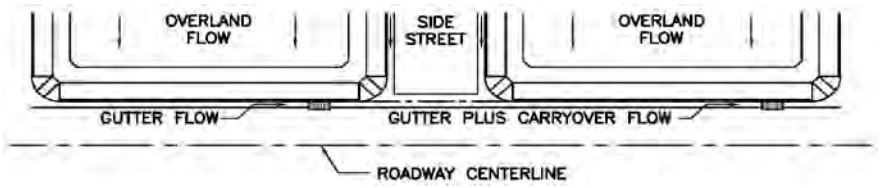


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**42.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	7.2	30.6	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4A



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	5.9	26.3	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

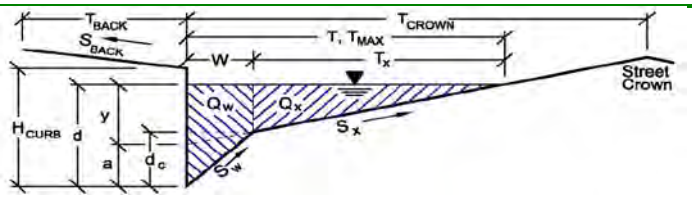
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	5.9	26.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4A



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

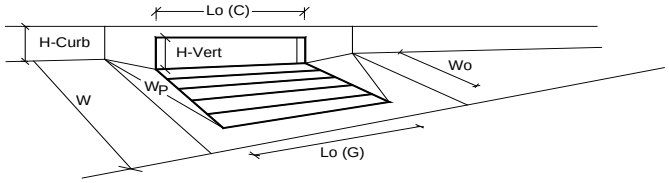
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 4A

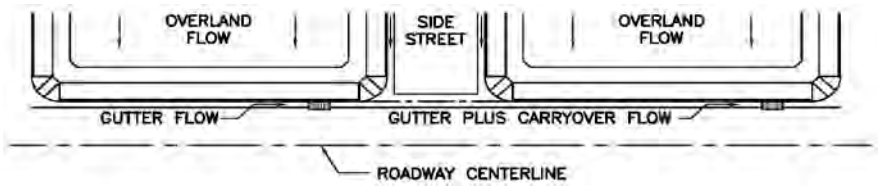


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**42.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	5.9	26.3	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4E



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	17.1	63.9	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

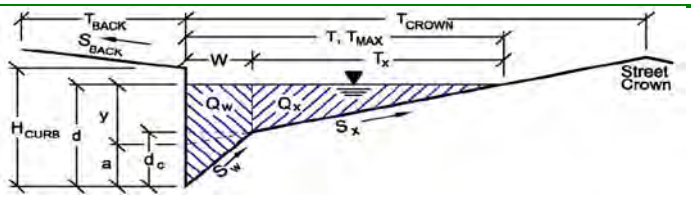
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	17.1	63.9	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4E



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

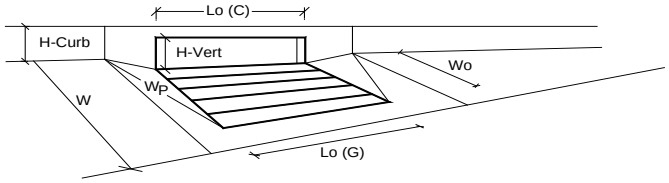
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 4E

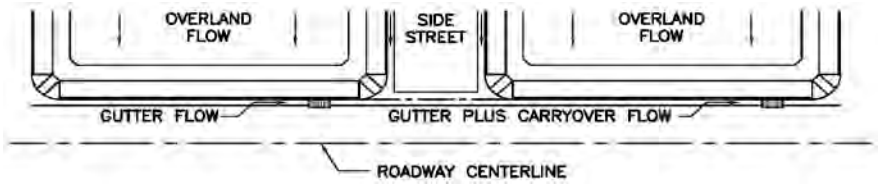


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**19.9**	**86.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	17.1	63.9	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4F



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	2.8	11.7	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

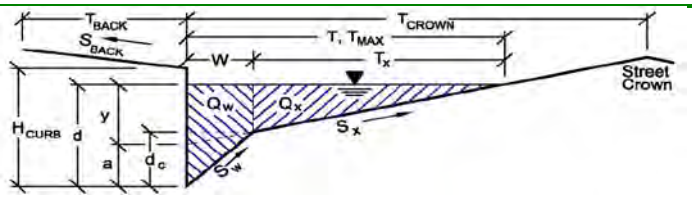
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	2.8	11.7	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4F



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	19.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.50	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.060	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**12.2**	**91.0**	**cfs**

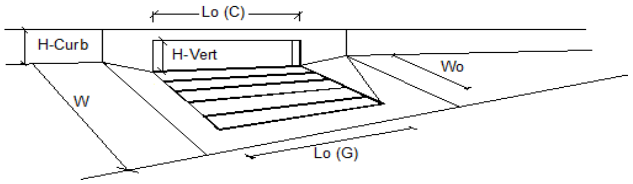
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: 4F



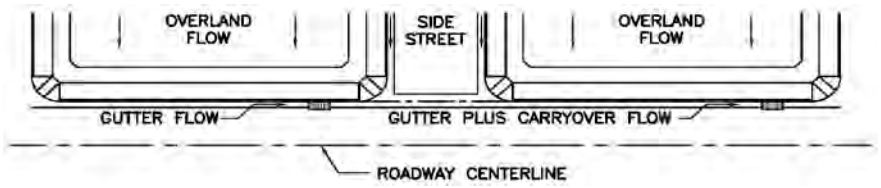
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < maximum allowable from sheet 'Q-Allow'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**2.80**	**11.69**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**100**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 4G



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	3.0	12.7	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

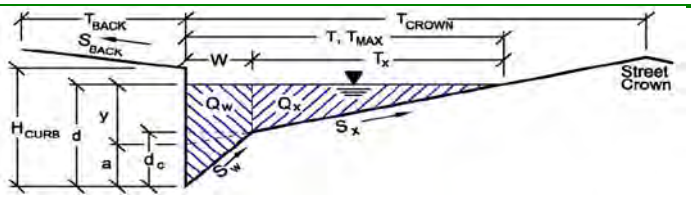
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	3.0	12.7	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 4G



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.040	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**6.8**	**127.1**	**cfs**

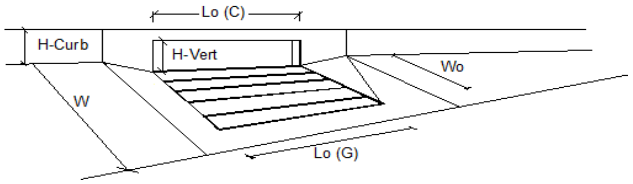
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 4G



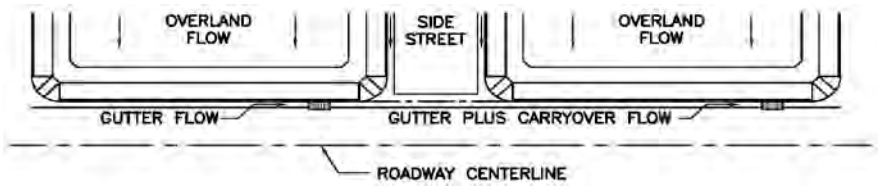
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < maximum allowable from sheet 'Q-Allow'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**3.00**	**12.52**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.2**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**99**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4I



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	5.0	21.9	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

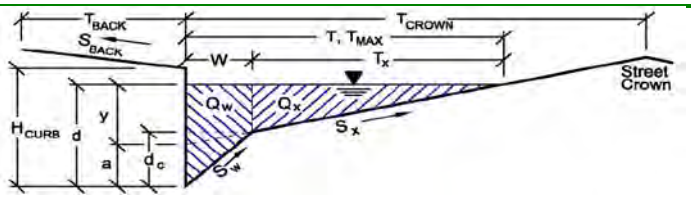
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	5.0	21.9	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4I



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

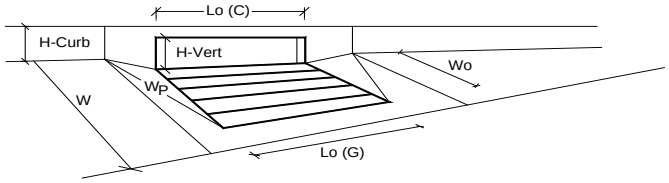
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 4I

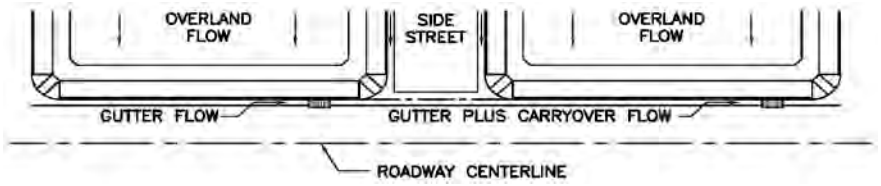


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	5.0	21.9	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4J



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	12.1	48.7	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

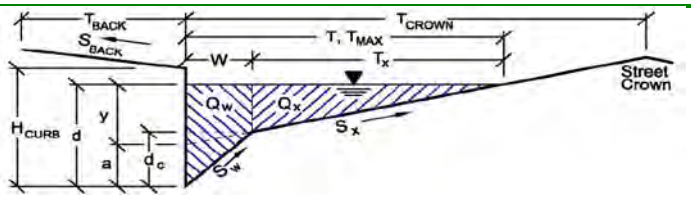
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	12.1	48.7	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4J



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

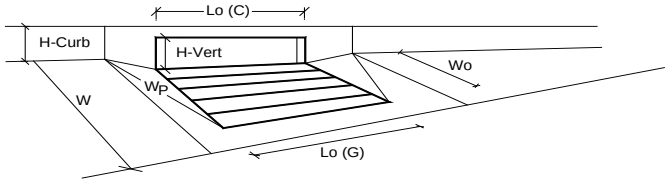
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 4J

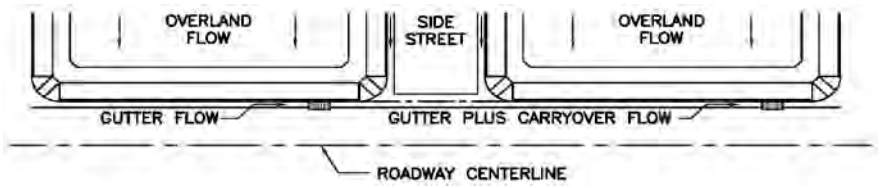


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**14.4**	**56.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	12.1	48.7	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4K



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	7.9	22.0	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

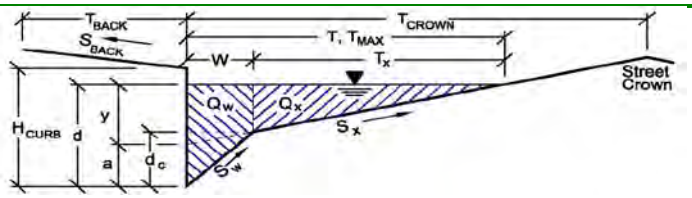
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	7.9	22.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4K



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

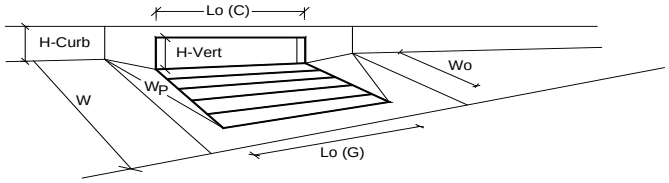
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 4K

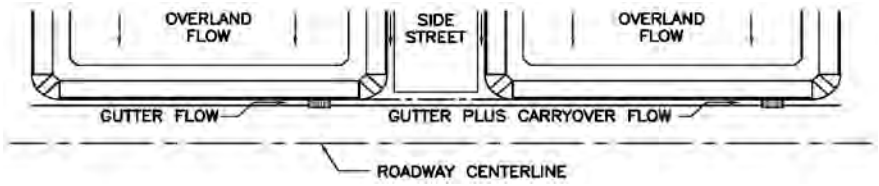


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	7.9	22.0	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 4L



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	2.7	7.6	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

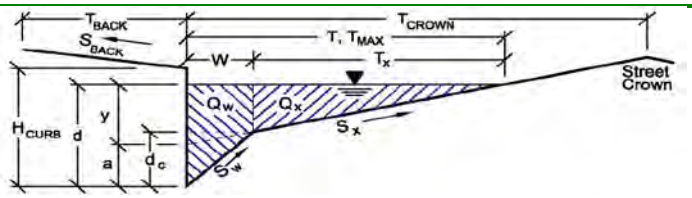
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	2.7	7.6	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 4L



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

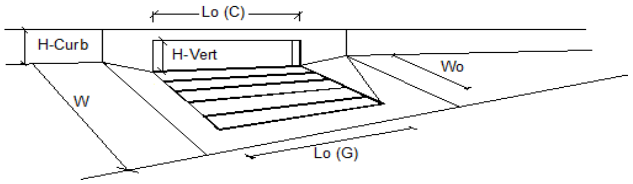
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: 4L



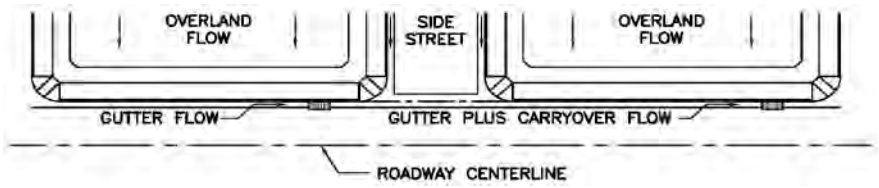
	Design Information (Input)		MINOR	MAJOR	
	Type of Inlet	Type =	CDOT Type R Curb Opening		
	Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	3.0	3.0	inches
	Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
	Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
	Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
	Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Warning 1	Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.20	0.20	

	Street Hydraulics: OK - Q < maximum allowable from sheet 'Q-Allow'		MINOR	MAJOR	
	Total Inlet Interception Capacity	**Q =**	**2.70**	**7.34**	**cfs**
	Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.3**	**cfs**
	Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**97**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5A



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	15.4	66.0	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

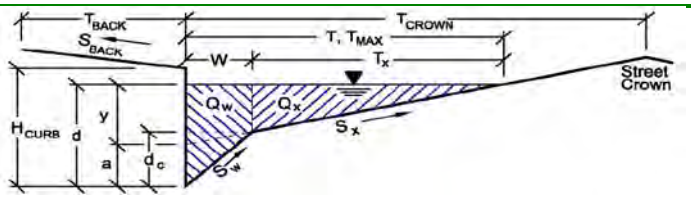
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	15.4	66.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5A



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.027	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

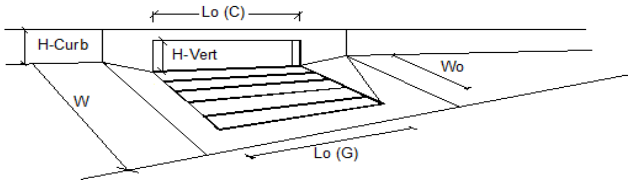
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**5.6**	**142.4**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5A



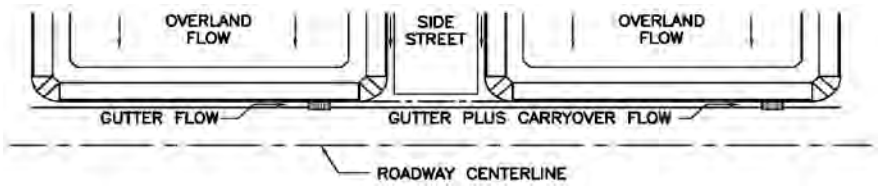
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**14.80**	**43.47**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.6**	**22.5**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**96**	**66**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 5B



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	6.6	20.3	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

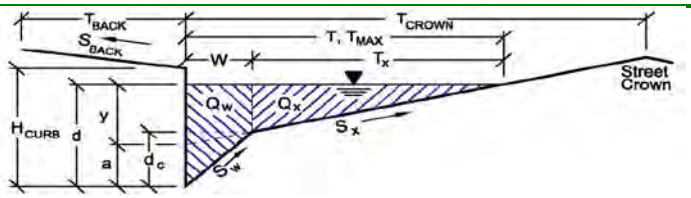
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	18.0	cfs
Total Design Peak Flow, Q =	6.6	38.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 5B



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

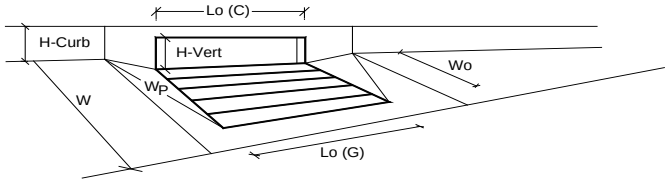
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 5B

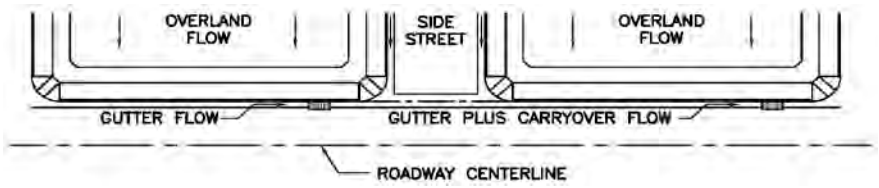


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**9.7**	**42.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	6.6	38.3	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5C



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	6.6	23.9	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

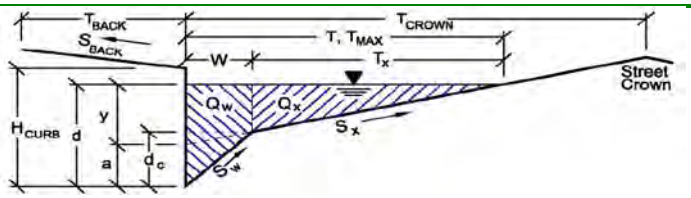
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	6.6	23.9	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5C



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.027	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

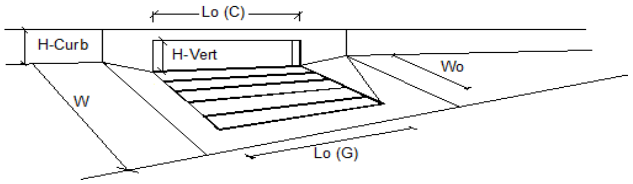
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**5.6**	**142.4**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5C



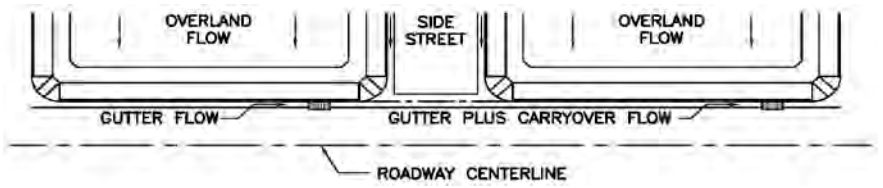
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**6.60**	**16.16**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**7.7**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**68**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 5E



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	9.7	92.4	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

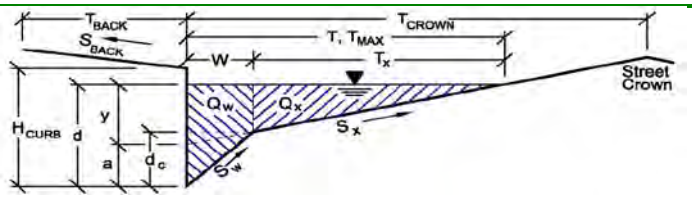
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	9.7	92.4	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 5E



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

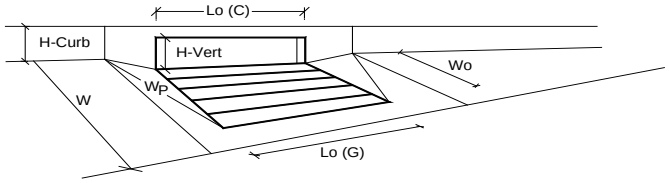
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 5E

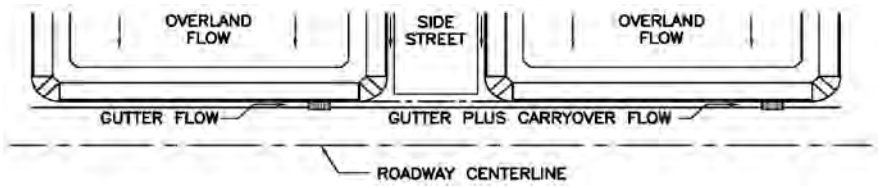


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**19.9**	**86.1**	**cfs**
WARNING: Inlet Capacity less than Q Peak for MAJOR Storm	$Q_{PEAK\ REQUIRED}$ =	9.7	92.4	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5F



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	7.8	45.4	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

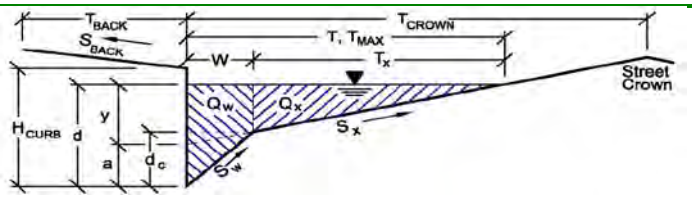
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	7.8	45.4	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5F



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.039	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

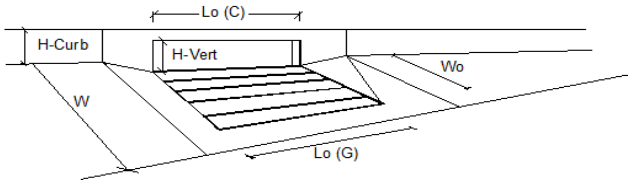
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**6.7**	**128.1**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5F



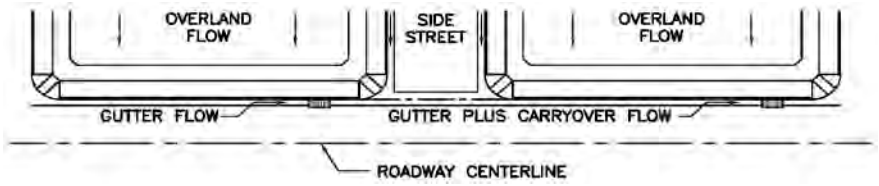
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.80**	**21.73**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**23.7**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**48**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 5G



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	8.4	115.7	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

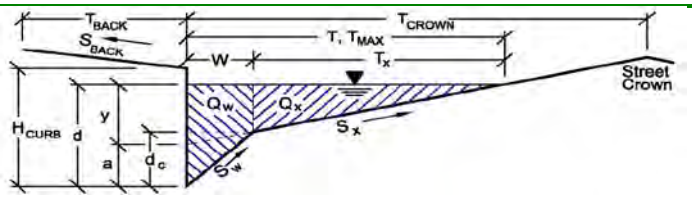
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	8.4	115.7	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 5G



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

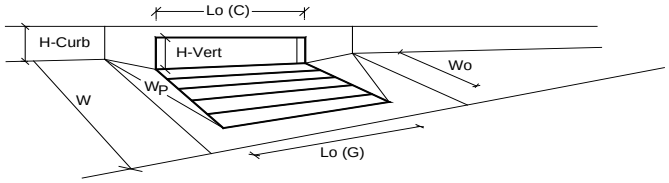
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 5G

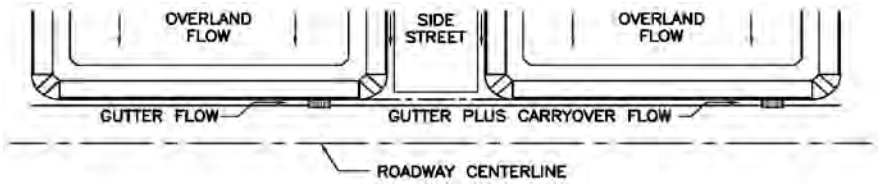


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**19.9**	**86.1**	**cfs**
WARNING: Inlet Capacity less than Q Peak for MAJOR Storm	$Q_{PEAK\ REQUIRED}$ =	8.4	115.7	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5H



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	6.7	81.3	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

FILL IN THE SECTIONS BELOW. <---

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

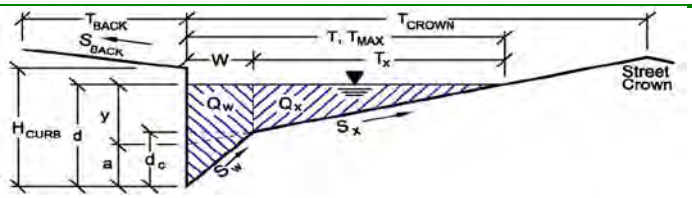
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	6.7	81.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5H



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.039	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

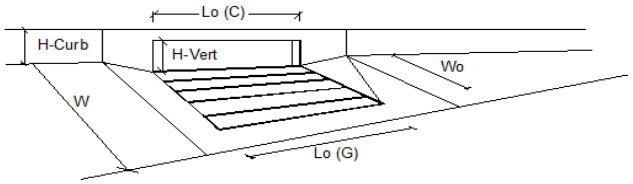
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**6.7**	**128.1**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5H



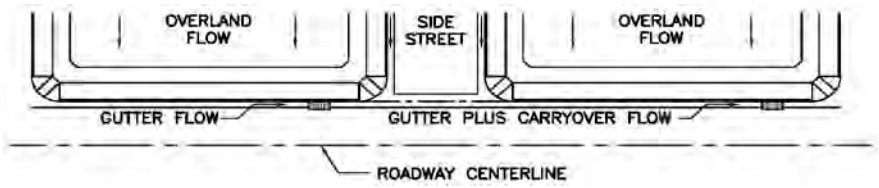
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**6.70**	**27.41**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**53.9**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**34**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5C



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	4.6	20.5	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

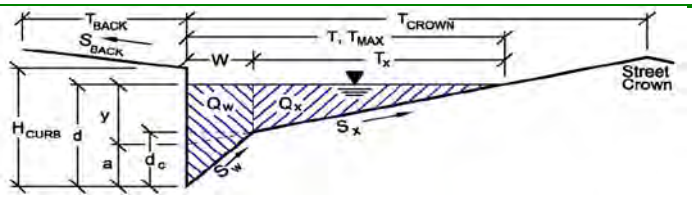
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	4.6	20.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5C



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.027	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**5.6**	**142.4**	**cfs**

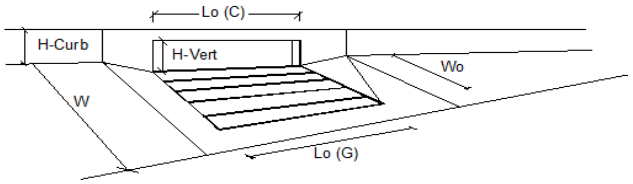
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5C



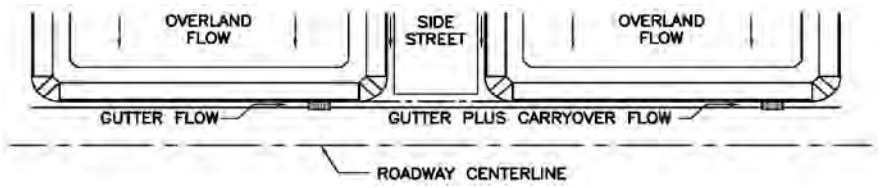
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < maximum allowable from sheet 'Q-Allow'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**4.60**	**14.99**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**5.5**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**73**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 5N



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	13.7	53.4	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

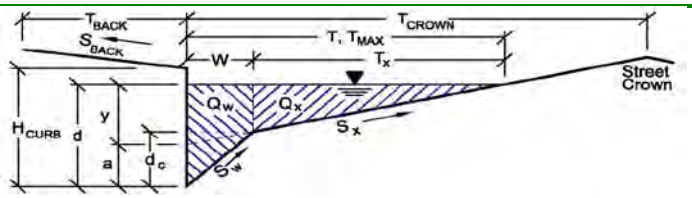
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	3.2	cfs
Total Design Peak Flow, Q =	13.7	56.6	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 5N



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

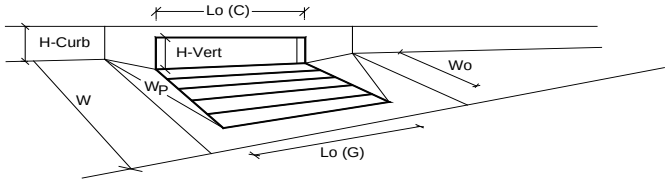
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 5N

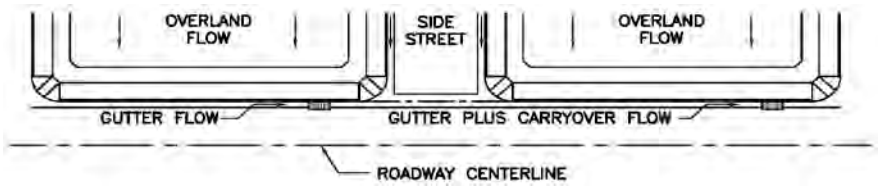


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**14.4**	**56.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	13.7	56.6	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5X



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	10.3	52.0	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

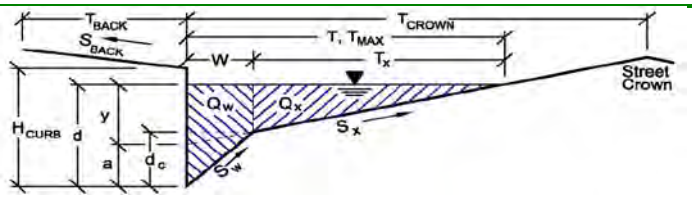
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	10.3	52.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5X



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.039	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

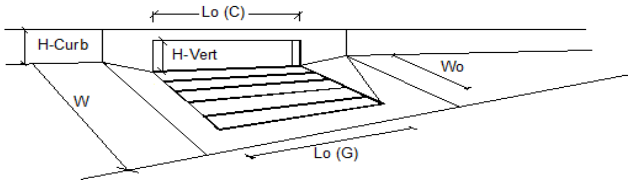
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**6.7**	**128.1**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5X



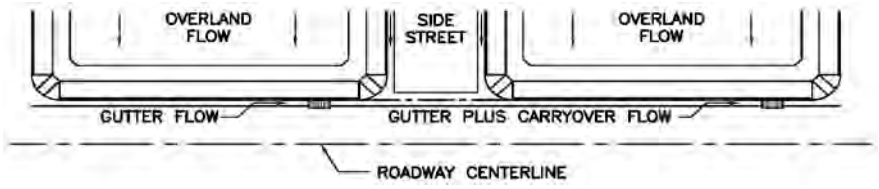
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**9.99**	**22.97**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.3**	**29.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**97**	**44**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 5Y



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	5.8	24.9	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

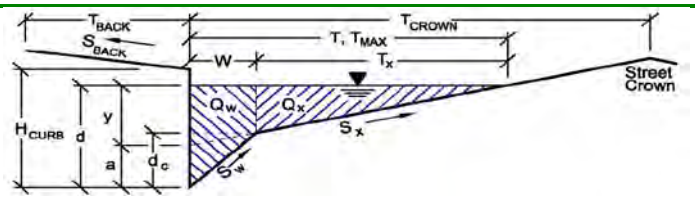
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	5.8	24.9	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 5Y



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.039	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**6.7**	**128.1**	**cfs**

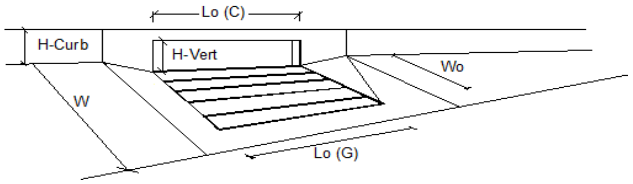
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 5Y



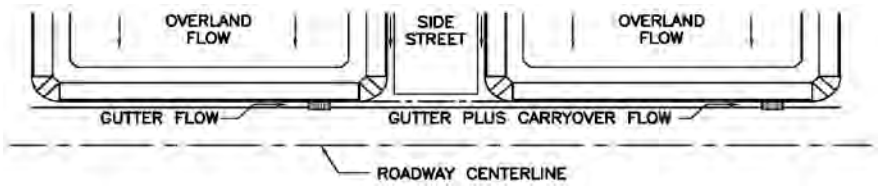
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: OK - Q < maximum allowable from sheet 'Q-Allow'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**5.58**	**11.97**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.2**	**12.9**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**96**	**48**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 6A



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	12.8	50.5	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

FILL IN THE SECTIONS BELOW. <---

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

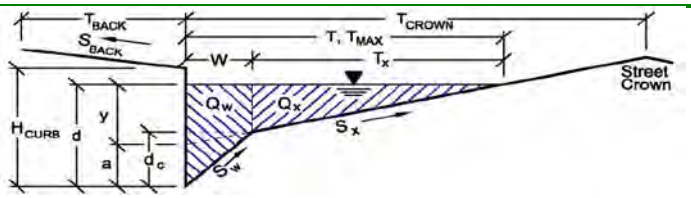
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	12.8	50.5	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 6A



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

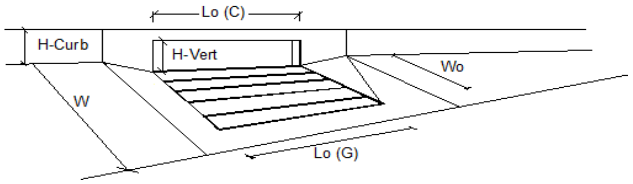
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 6A



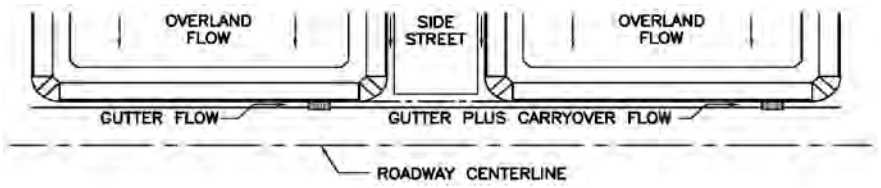
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**12.19**	**36.61**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.6**	**13.9**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**95**	**72**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 6B



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	7.3	21.3	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

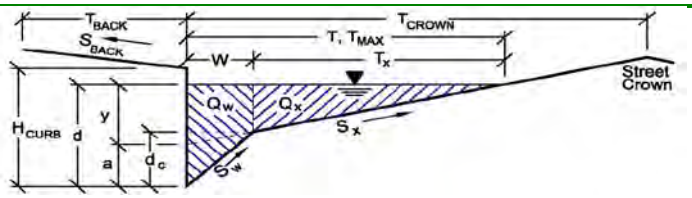
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	7.3	21.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 6B



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

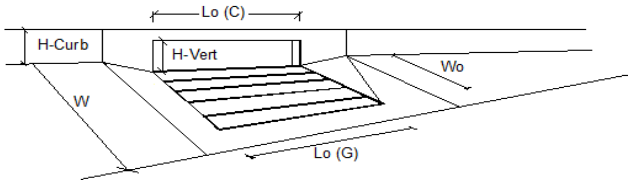
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 6B



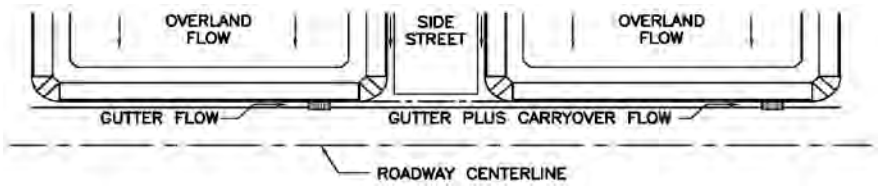
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**7.22**	**14.92**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.1**	**6.4**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**99**	**70**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 6C



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	11.0	47.0	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

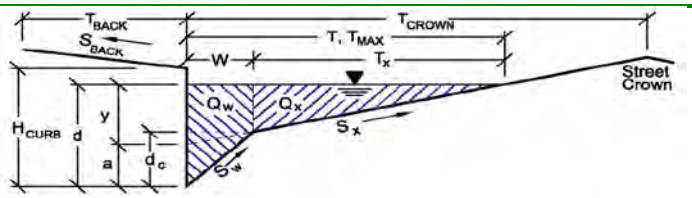
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	11.0	47.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 6C



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.020	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

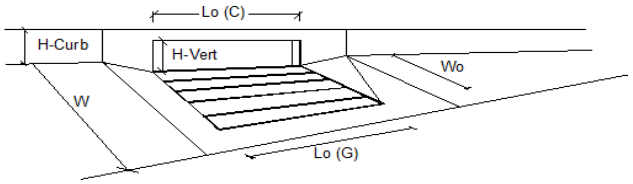
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.8**	**156.5**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 6C



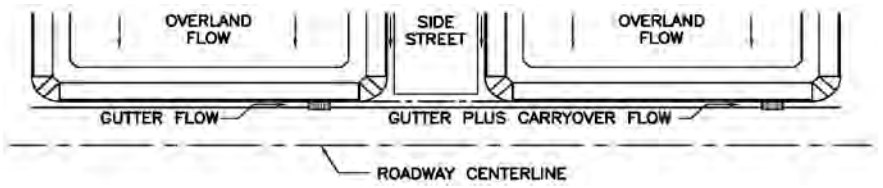
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**10.71**	**35.61**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.3**	**11.4**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**97**	**76**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: DP 6E



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^*Q_{Known}$ =	5.3	100.0	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

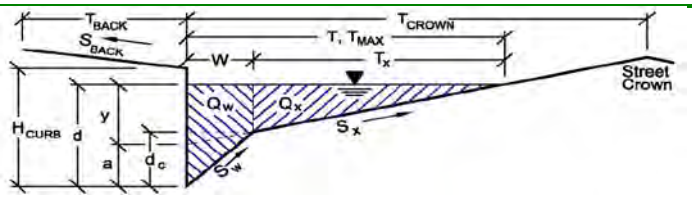
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	5.3	100.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: DP 6E



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

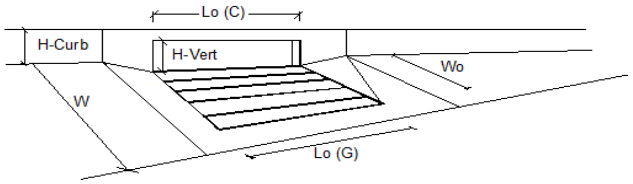
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**4.1**	**162.5**	**cfs**

WARNING: MINOR STORM max. allowable capacity is less than flow given on sheet 'Q-Peak'
Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET ON A CONTINUOUS GRADE

Project: Trails at Crowfoot

Inlet ID: DP 6E



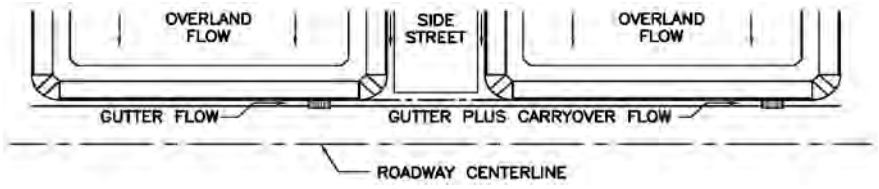
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{LOCAL} =	5.0	5.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W from Q-Allow)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**5.29**	**28.99**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**71.0**	**cfs**
Capture Percentage = Q_a/Q_o =	**C% =**	**100**	**29**	**%**

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 6F



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	4.9	24.1	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW.
<---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

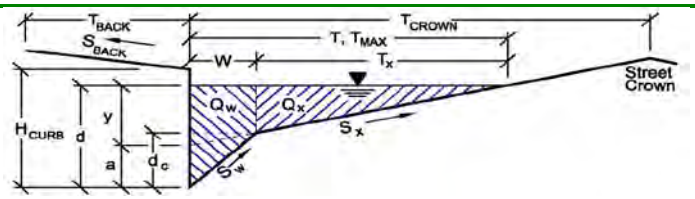
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	4.9	24.1	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 6F



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

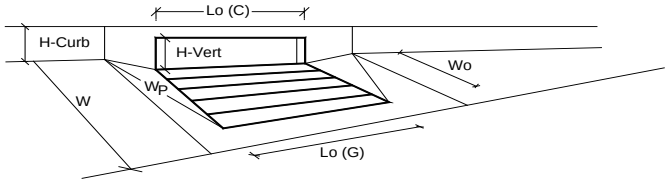
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 6F

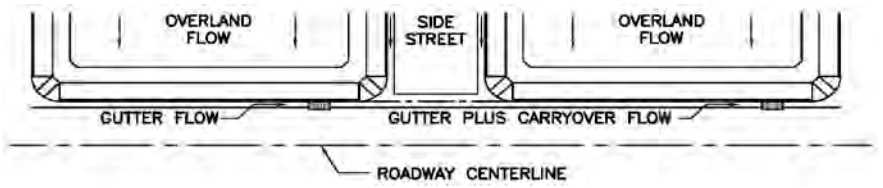


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	4.9	24.1	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 6G



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
$^{*}Q_{Known}$ =	2.3	16.0	cfs

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

<--- FILL IN THIS SECTION OR...

FILL IN THE SECTIONS BELOW. <---

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =	Acres
Percent Imperviousness =	%
NRCS Soil Type =	A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{*} C_3$

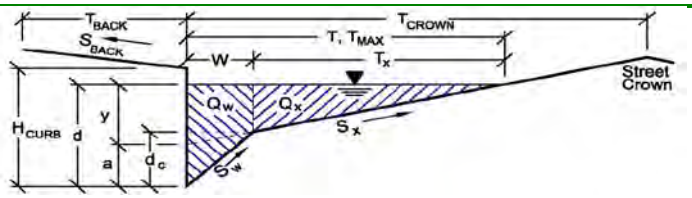
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	2.3	16.0	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 6G



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

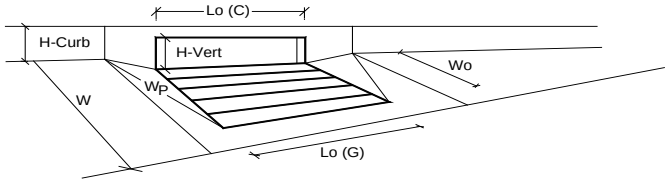
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 6G

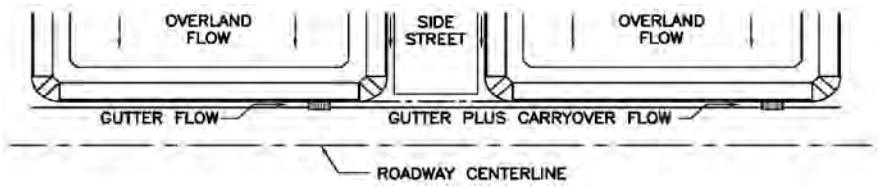


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**8.3**	**27.5**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	2.3	16.0	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 6H



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	9.8	45.6	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

FILL IN THE SECTIONS BELOW. <---

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- Site is Urban
- Site is Non-Urban

Flows Developed For:
- Street Inlets
- Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

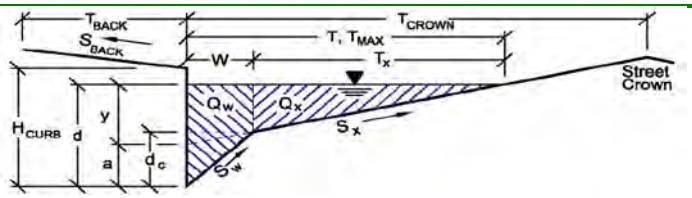
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	9.8	45.6	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 6H



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

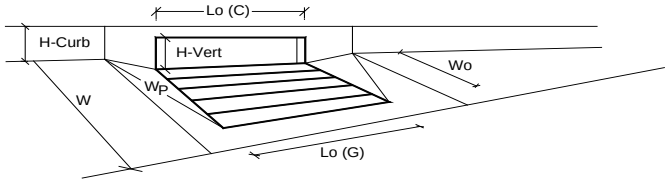
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 6H

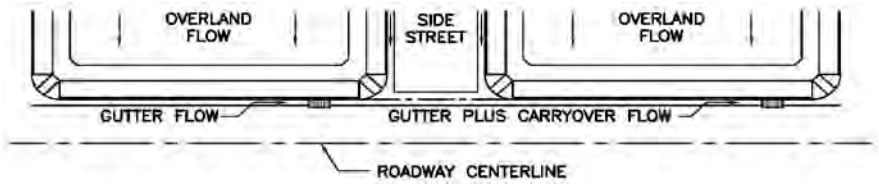


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**14.4**	**56.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	9.8	45.6	cfs

Worksheet Protected

DESIGN PEAK FLOW FOR ONE-HALF OF STREET OR GRASS-LINED CHANNEL BY THE RATIONAL METHOD

Project: Trails at Crowfoot
Inlet ID: 6I



Show Details

Design Flow: ONLY if already determined through other methods:
(local peak flow for 1/2 of street OR grass-lined channel):

	Minor Storm	Major Storm	
*Q_{Known} =	2.5	94.6	cfs

<--- FILL IN THIS SECTION OR...

*** If you enter values in Row 14, skip the rest of this sheet and proceed to sheet Q-Allow or Area Inlet.**

Geographic Information: (Enter data in the blue cells):

Subcatchment Area =		Acres
Percent Imperviousness =		%
NRCS Soil Type =		A, B, C, or D

Site Type:
- ○ Site is Urban
- ○ Site is Non-Urban

Flows Developed For:
- ○ Street Inlets
- ○ Area Inlets in a Median

	Slope (ft/ft)	Length (ft)
Overland Flow =		
Channel Flow =		

FILL IN THE SECTIONS BELOW. <---

Rainfall Information: Intensity I (inch/hr) = $C_1 * P_1 / (C_2 + T_c)^{**} C_3$

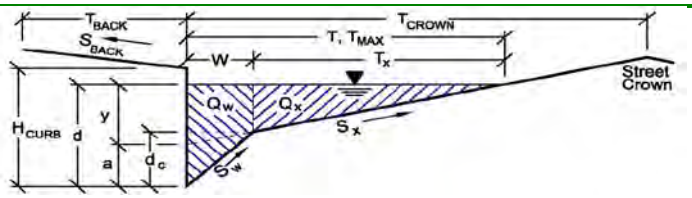
	Minor Storm	Major Storm	
Design Storm Return Period, T_r =			years
Return Period One-Hour Precipitation, P_1 =			inches
C_1 =			
C_2 =			
C_3 =			
User-Defined Storm Runoff Coefficient (leave this blank to accept a calculated value), C =			
User-Defined 5-yr. Runoff Coefficient (leave this blank to accept a calculated value), C_5 =			
Bypass (Carry-Over) Flow from upstream Subcatchments, Q_b =	0.0	0.0	cfs
Total Design Peak Flow, Q =	2.5	94.6	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Trails at Crowfoot

Inlet ID: 6I



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	18.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	4.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	17.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.0	12.0	inches
Allow Flow Depth at Street Crown (leave blank for no)		☐	☑	check = yes

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

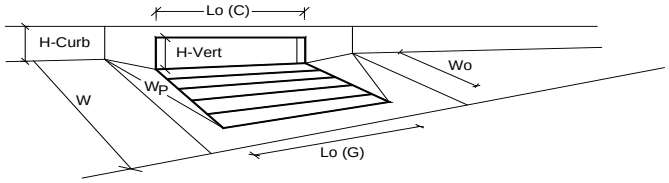
Minor storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

Major storm max. allowable capacity GOOD - greater than flow given on sheet 'Q-Peak'

INLET IN A SUMP OR SAG LOCATION

Project = Trails at Crowfoot

Inlet ID = 6I



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Inlet Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from 'Q-Allow')	a_{local} =	5.00	5.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	12.0	inches
				☑Override Depths
Grate Information		MINOR	MAJOR	
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**19.9**	**86.1**	**cfs**
WARNING: Inlet Capacity less than Q Peak for MAJOR Storm	$Q_{PEAK\ REQUIRED}$ =	2.5	94.6	cfs

III. Hydraulic Computations

C. UD-SEWER (To be provided with Final Drainage Report)

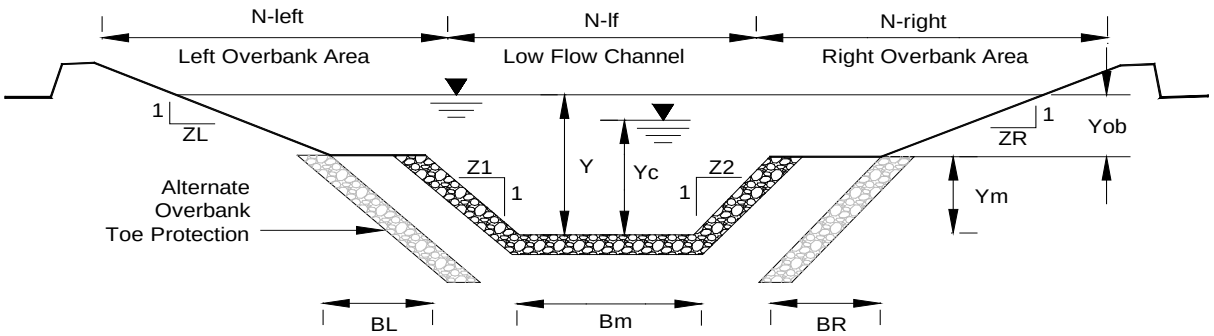
III. Hydraulic Computations

D. UD-CHANNEL

Capacity Analysis of Composite Channel

Project: Trails at Crowfoot

Channel ID:



Design Information (Input)

Channel Invert Slope	So =	0.00200 ft/ft	Left Overbank Bottom Width	BL =	6.00 ft
			Left Overbank Side Slope	ZL =	4.00 ft/ft
Low Flow Channel Bottom Width	Bm =	6.00 ft	Left Overbank Manning's n	n-left =	0.0400
Low Flow Channel Left Side Slope	Z1 =	2.50 ft/ft	Right Overbank Bottom Width	BR =	6.00 ft
Low Flow Channel Right Side Slope	Z2 =	2.50 ft/ft	Right Overbank Side Slope	ZR =	4.00 ft/ft
Low Flow Channel Manning's Nn for Qd	n-lf =	0.0650	Right Overbank Manning's n	n-right =	0.0400
Low Flow Channel Manning's Nn for Q100	n-m-Q100 =	0.0400			
(See USDCM Vol. II, n vs. Depth Graph)					
Low Flow Channel Bank-full depth	**Ym =**	**0.50 ft**	**Overbank Flow Depth Yob (Y - Ym)**	**Yob =**	**3.00 ft**

Low Flow Channel Condition for Qd			**Low Flow Channel Flow Condition for Q100**		
Top width	Tlf =	8.5 ft	Top width	Tm =	8.5 ft
Flow area	Alf =	3.6 sq ft	Flow area	Am =	29.1 sq ft
Wetted perimeter	Plf =	8.7 ft	Wetted perimeter	Pm =	8.7 ft
Discharge (Calculated)	**Qlf =**	**2.1 cfs**	**Discharge**	**Qm =**	**108.6 cfs**
Velocity	**Vlf=**	**0.6 fps**	**Velocity**	**Vm =**	**3.7 fps**
Froude number	**Fr-lf =**	**0.15**	**Froude number**	**Frm =**	**0.36**
Qd Critical Velocity	Vlfc =	2.15 fps	100-Yr. Critical Velocity	Vmc =	7.5 fps
Qd Critical Depth	Ylfc =	0.15 ft	100-Yr. Critical Depth	Ymc =	1.8 ft
Left Overbank Flow Condition for Q100			**Right Overbank Flow Condition for Q100**		
Top width	TL =	18.0 ft	Top width	TR =	18.0 ft
Flow area	AL =	36.0000 sq ft	Flow area	AR =	36.0000 sq ft
Wetted perimeter	PL =	18.3700 ft	Wetted perimeter	PR =	18.3700 ft
Discharge	**QL =**	**93.9 cfs**	**Discharge**	**QR =**	**93.9 cfs**
Velocity	**VL =**	**2.6 fps**	**Velocity**	**VR =**	**2.6 fps**
Froude number	**FrL =**	**0.33**	**Froude number**	**FrR =**	**0.33**
100-Yr. Critical Velocity	VLc =	6.2 fps	100-Yr. Critical Velocity	VRc =	6.2 fps
100-Yr. Critical Depth in Overbanks	YLc =	1.6 ft	100-Yr. Critical Depth in Overbanks	YRc =	1.6 ft

Composite Cross-Section Flow Condition for Q100

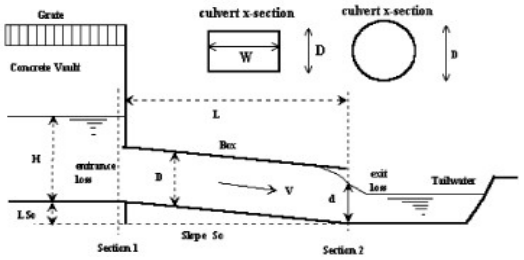
Top width	**T =**	**44.5 ft**	**Discharge**	**Q =**	**296.5 cfs**
Channel Depth Y	**Y =**	**3.50 ft**	**Velocity**	**V =**	**2.9 fps**
Flow area	A =	101.1 sq ft	**Froude number**	**Fr =**	**0.34**
Wetted perimeter	P =	45.4 ft	100-Yr. Critical Velocity	Vc =	6.6 fps
Cross-Sectional Manning's n (Calculated)	n =	0.0387	100-Yr. Critical Depth in Overbanks	Yc =	1.54 ft

III. Hydraulic Computations

E. UD-CULVERT

CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfooot
Basin ID: Channel West Culvert
Status:



Design Information (Input):

Circular Culvert: Barrel Diameter in Inches — D = [ ] inches
Inlet Edge Type (choose from pull-down list): Grooved End with Headwall

OR:

Box Culvert: Barrel Height (Rise) in Feet — Height (Rise) = 4.00 ft.
Barrel Width (Span) in Feet — Width (Span) = 6.00 ft.
Inlet Edge Type (choose from pull-down list): 1.5 : 1 Bevel w/ 90 Deg. Headwall

Number of Barrels	No =	2
Inlet Elevation at Culvert Invert	Inlet Elev =	6004 ft. elev.
Outlet Elevation at Culvert Invert **OR** Slope of Culvert (ft v./ft h.)	Outlet Elev =	6000.5 ft. elev.
Culvert Length in Feet	L =	350 ft.
Manning's Roughness	n =	0.012
Bend Loss Coefficient	K_b =	0
Exit Loss Coefficient	K_x =	1

Design Information (calculated):

Entrance Loss Coefficient	K_e =	0.20
Friction Loss Coefficient	K_f =	1.46
Sum of All Loss Coefficients	K_s =	2.66
Orifice Inlet Condition Coefficient	C_d =	1.03
Minimum Energy Condition Coefficient	KE_{low} =	0.0356

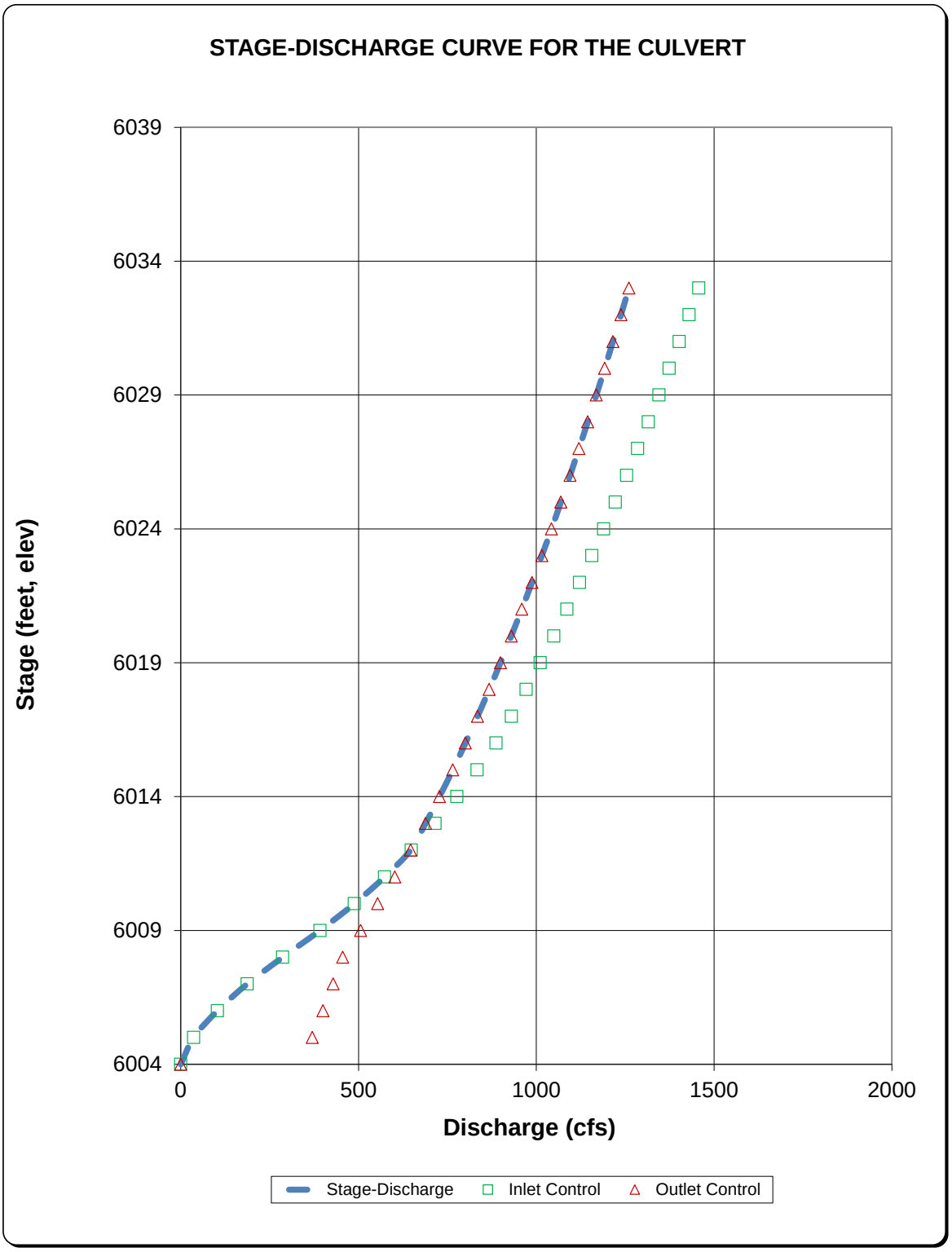
Calculations of Culvert Capacity (output):

Water Surface Elevation (ft., linked)	Tailwater Surface Elevation ft	Culvert Inlet-Control Flowrate cfs	Culvert Outlet-Control Flowrate cfs	Controlling Culvert Flowrate cfs (output)	Inlet Equation Used:	Flow Control Used
6004.00	1.20	0.00	0.00	**0.00**	No Flow (WS < inlet)	N/A
6005.00	1.20	36.60	369.98	**36.60**	Min. Energy. Eqn.	INLET
6006.00	1.20	103.20	400.15	**103.20**	Min. Energy. Eqn.	INLET
6007.00	1.20	187.00	428.64	**187.00**	Regression Eqn.	INLET
6008.00	1.20	287.00	455.69	**287.00**	Regression Eqn.	INLET
6009.00	1.20	391.80	506.20	**391.80**	Regression Eqn.	INLET
6010.00	1.20	488.40	553.76	**488.40**	Regression Eqn.	INLET
6011.00	1.20	573.40	602.02	**573.40**	Regression Eqn.	INLET
6012.00	1.20	648.40	646.66	**646.66**	Regression Eqn.	OUTLET
6013.00	1.20	715.60	688.44	**688.44**	Regression Eqn.	OUTLET
6014.00	1.20	777.00	727.81	**727.81**	Regression Eqn.	OUTLET
6015.00	1.20	833.80	765.13	**765.13**	Regression Eqn.	OUTLET
6016.00	1.20	886.80	800.73	**800.73**	Regression Eqn.	OUTLET
6017.00	1.20	930.20	834.83	**834.83**	Orifice Eqn.	OUTLET
6018.00	1.20	971.60	867.59	**867.59**	Orifice Eqn.	OUTLET
6019.00	1.20	1,011.20	899.14	**899.14**	Orifice Eqn.	OUTLET
6020.00	1.20	1,049.40	929.64	**929.64**	Orifice Eqn.	OUTLET
6021.00	1.20	1,086.20	959.14	**959.14**	Orifice Eqn.	OUTLET
6022.00	1.20	1,121.80	987.80	**987.80**	Orifice Eqn.	OUTLET
6023.00	1.20	1,156.40	1,015.63	**1,015.63**	Orifice Eqn.	OUTLET
6024.00	1.20	1,189.80	1,042.71	**1,042.71**	Orifice Eqn.	OUTLET
6025.00	1.20	1,222.40	1,069.12	**1,069.12**	Orifice Eqn.	OUTLET
6026.00	1.20	1,254.20	1,094.86	**1,094.86**	Orifice Eqn.	OUTLET
6027.00	1.20	1,285.20	1,120.04	**1,120.04**	Orifice Eqn.	OUTLET
6028.00	1.20	1,315.40	1,144.67	**1,144.67**	Orifice Eqn.	OUTLET
6029.00	1.20	1,345.00	1,168.77	**1,168.77**	Orifice Eqn.	OUTLET
6030.00	1.20	1,374.00	1,192.39	**1,192.39**	Orifice Eqn.	OUTLET
6031.00	1.20	1,402.20	1,215.52	**1,215.52**	Orifice Eqn.	OUTLET
6032.00	1.20	1,430.00	1,238.27	**1,238.27**	Orifice Eqn.	OUTLET
6033.00	1.20	1,457.20	1,260.56	**1,260.56**	Orifice Eqn.	OUTLET

Processing Time: 01.14 Seconds

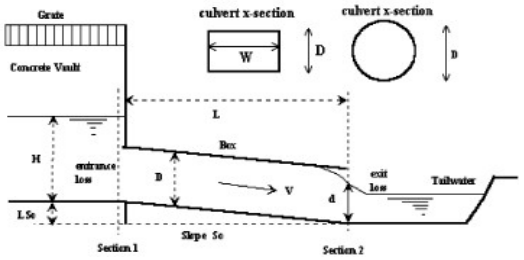
CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfooot
Basin ID: Channel West Culvert



CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfoot
Basin ID: Pond A Outlet Pipe
Status:



Design Information (Input):

Circular Culvert: Barrel Diameter in Inches — D = 48 inches
Inlet Edge Type (choose from pull-down list) — Grooved End with Headwall

OR:

Box Culvert: Barrel Height (Rise) in Feet — Height (Rise) = ft.
Barrel Width (Span) in Feet — Width (Span) = ft.
Inlet Edge Type (choose from pull-down list) — Square Edge w/ 30-78 deg. Flared Wingwall

Number of Barrels	No =	1	
Inlet Elevation at Culvert Invert	Inlet Elev =	6007	ft. elev.
Outlet Elevation at Culvert Invert **OR** Slope of Culvert (ft v./ft h.)	Outlet Elev =	6006	ft. elev.
Culvert Length in Feet	L =	100	ft.
Manning's Roughness	n =	0.011	
Bend Loss Coefficient	K_b =	0	
Exit Loss Coefficient	K_x =	1	

Design Information (calculated):

Entrance Loss Coefficient	K_e =	0.20
Friction Loss Coefficient	K_f =	0.35
Sum of All Loss Coefficients	K_s =	1.55
Orifice Inlet Condition Coefficient	C_d =	0.99
Minimum Energy Condition Coefficient	KE_{low} =	-0.0266

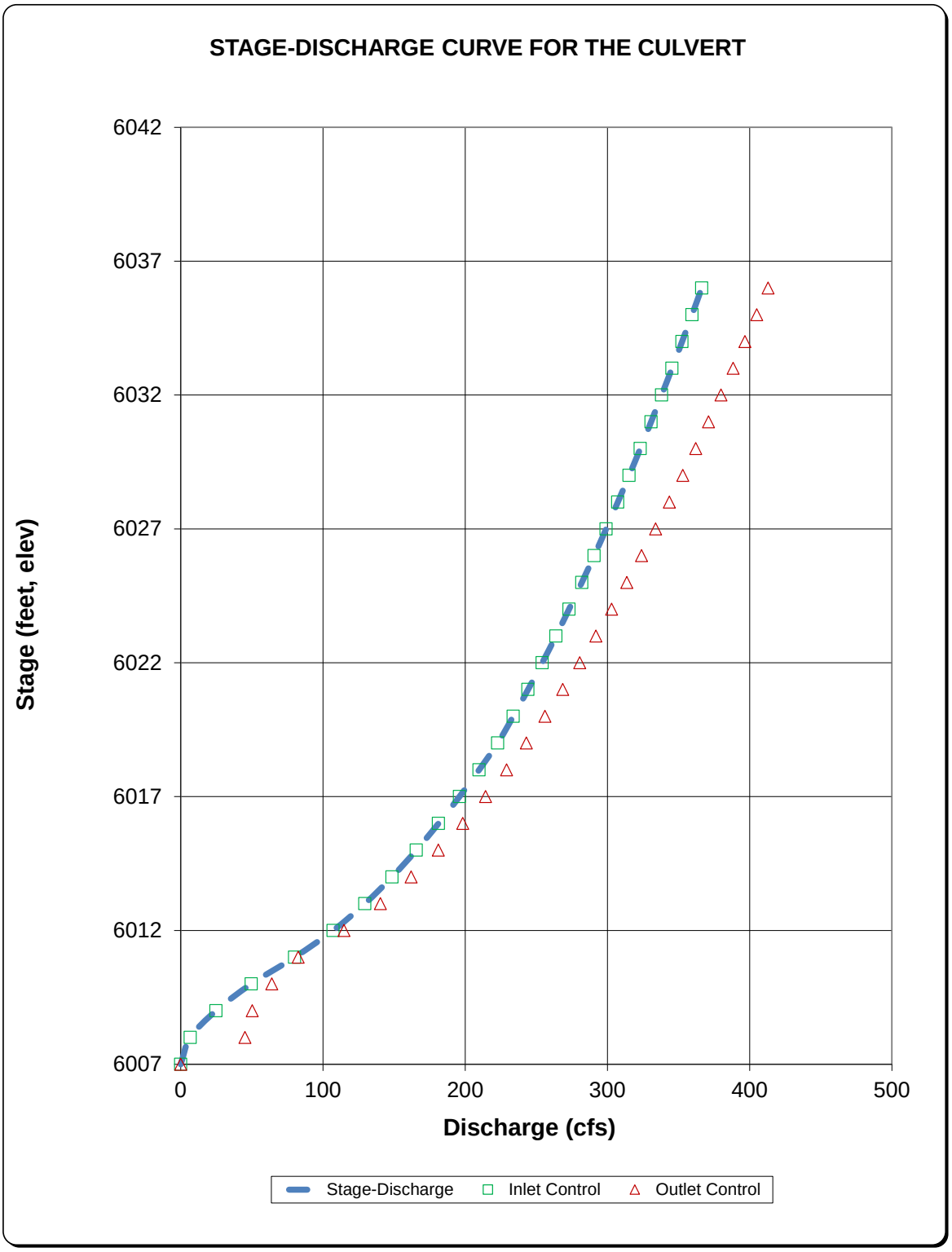
Calculations of Culvert Capacity (output):

Water Surface Elevation (ft., linked)	Tailwater Surface Elevation ft	Culvert Inlet-Control Flowrate cfs	Culvert Outlet-Control Flowrate cfs	Controlling Culvert Flowrate cfs (output)	Inlet Equation Used:	Flow Control Used
6007.00	1.20	0.00	0.00	**0.00**	No Flow (WS < inlet)	N/A
6008.00	1.20	6.70	45.16	**6.70**	Min. Energy. Eqn.	INLET
6009.00	1.20	24.80	50.33	**24.80**	Min. Energy. Eqn.	INLET
6010.00	1.20	49.60	64.08	**49.60**	Regression Eqn.	INLET
6011.00	1.20	80.10	82.67	**80.10**	Regression Eqn.	INLET
6012.00	1.20	107.20	114.84	**107.20**	Regression Eqn.	INLET
6013.00	1.20	129.50	140.39	**129.50**	Regression Eqn.	INLET
6014.00	1.20	148.60	162.04	**148.60**	Regression Eqn.	INLET
6015.00	1.20	165.70	181.13	**165.70**	Regression Eqn.	INLET
6016.00	1.20	181.30	198.36	**181.30**	Regression Eqn.	INLET
6017.00	1.20	196.00	214.32	**196.00**	Regression Eqn.	INLET
6018.00	1.20	209.80	229.09	**209.80**	Regression Eqn.	INLET
6019.00	1.20	222.90	243.01	**222.90**	Regression Eqn.	INLET
6020.00	1.20	233.80	256.08	**233.80**	Orifice Eqn.	INLET
6021.00	1.20	244.20	268.65	**244.20**	Orifice Eqn.	INLET
6022.00	1.20	254.20	280.53	**254.20**	Orifice Eqn.	INLET
6023.00	1.20	263.80	291.99	**263.80**	Orifice Eqn.	INLET
6024.00	1.20	273.00	303.02	**273.00**	Orifice Eqn.	INLET
6025.00	1.20	282.00	313.63	**282.00**	Orifice Eqn.	INLET
6026.00	1.20	290.70	323.99	**290.70**	Orifice Eqn.	INLET
6027.00	1.20	299.10	333.92	**299.10**	Orifice Eqn.	INLET
6028.00	1.20	307.30	343.59	**307.30**	Orifice Eqn.	INLET
6029.00	1.20	315.30	353.02	**315.30**	Orifice Eqn.	INLET
6030.00	1.20	323.10	362.18	**323.10**	Orifice Eqn.	INLET
6031.00	1.20	330.70	371.10	**330.70**	Orifice Eqn.	INLET
6032.00	1.20	338.10	379.84	**338.10**	Orifice Eqn.	INLET
6033.00	1.20	345.40	388.41	**345.40**	Orifice Eqn.	INLET
6034.00	1.20	352.50	396.73	**352.50**	Orifice Eqn.	INLET
6035.00	1.20	359.50	404.96	**359.50**	Orifice Eqn.	INLET
6036.00	1.20	366.30	412.94	**366.30**	Orifice Eqn.	INLET

Processing Time: 00.61 Seconds

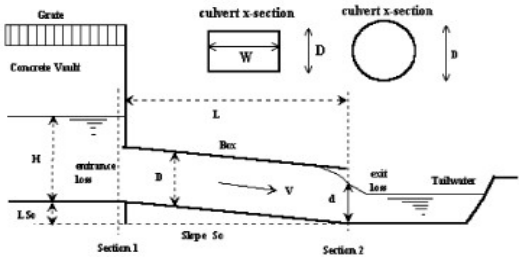
CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfoot
Basin ID: Pond A



CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfoot
Basin ID: Pond B Outlet Pipe
Status:



Design Information (Input):

Circular Culvert:	Barrel Diameter in Inches	D =	24	inches
	Inlet Edge Type (choose from pull-down list)		Grooved End with Headwall	
OR:				
Box Culvert:	Barrel Height (Rise) in Feet	Height (Rise) =		ft.
	Barrel Width (Span) in Feet	Width (Span) =		ft.
	Inlet Edge Type (choose from pull-down list)		Square Edge w/ 30-78 deg. Flared Wingwall	
	Number of Barrels	No =	1	
	Inlet Elevation at Culvert Invert	Inlet Elev =	6007	ft. elev.
	Outlet Elevation at Culvert Invert **OR** Slope of Culvert (ft v./ft h.)	Outlet Elev =	6006.5	ft. elev.
	Culvert Length in Feet	L =	200	ft.
	Manning's Roughness	n =	0.011	
	Bend Loss Coefficient	K_b =	0	
	Exit Loss Coefficient	K_x =	1	

Design Information (calculated):

Entrance Loss Coefficient	K_e =	0.20
Friction Loss Coefficient	K_f =	1.77
Sum of All Loss Coefficients	K_s =	2.97
Orifice Inlet Condition Coefficient	C_d =	0.99
Minimum Energy Condition Coefficient	KE_{low} =	-0.0342

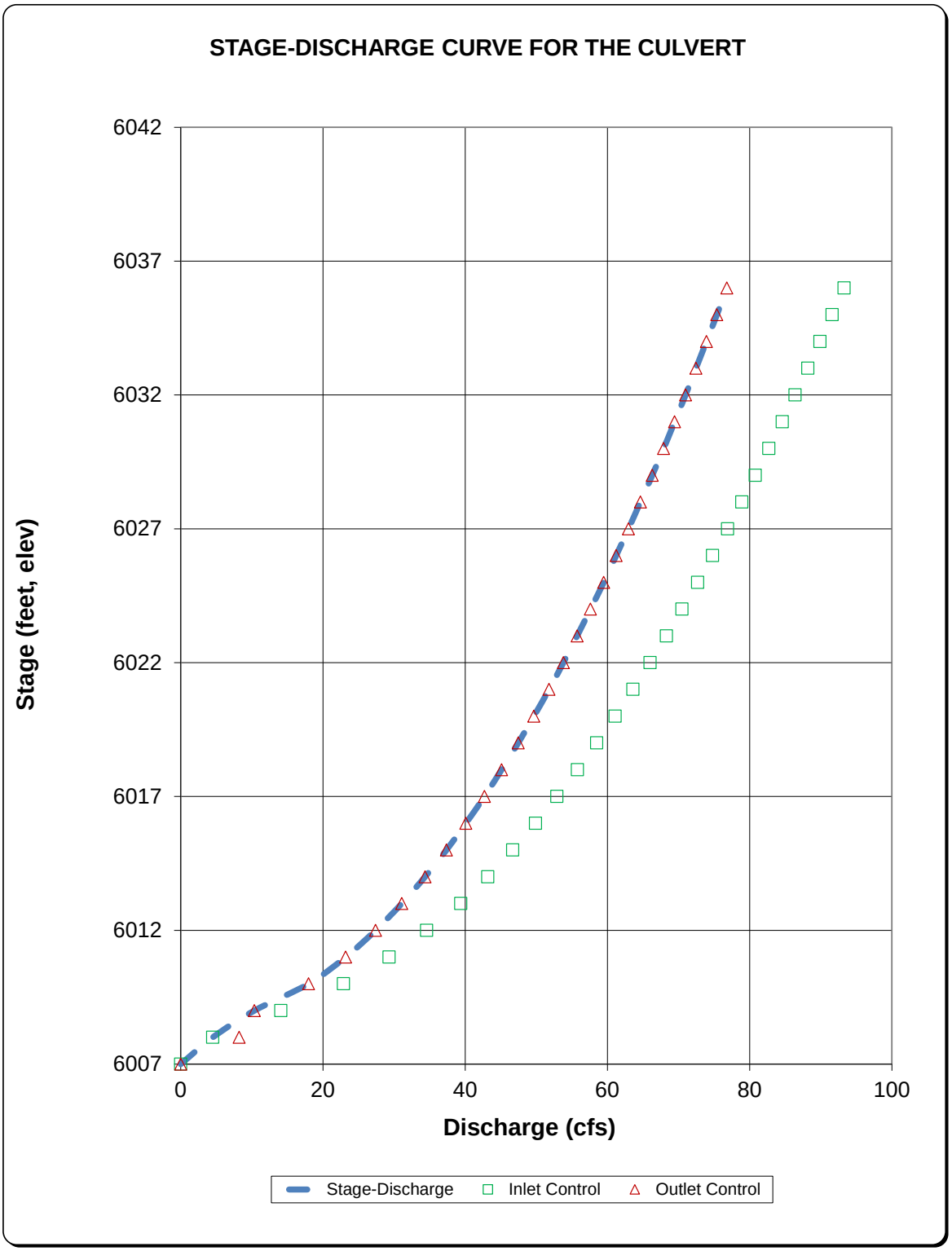
Calculations of Culvert Capacity (output):

Water Surface Elevation (ft., linked)	Tailwater Surface Elevation ft	Culvert Inlet-Control Flowrate cfs	Culvert Outlet-Control Flowrate cfs	Controlling Culvert Flowrate cfs (output)	Inlet Equation Used:	Flow Control Used
6007.00	1.20	0.00	0.00	**0.00**	No Flow (WS < inlet)	N/A
6008.00	1.20	4.50	8.22	**4.50**	Min. Energy. Eqn.	INLET
6009.00	1.20	14.10	10.36	**10.36**	Regression Eqn.	OUTLET
6010.00	1.20	22.90	17.98	**17.98**	Regression Eqn.	OUTLET
6011.00	1.20	29.30	23.19	**23.19**	Regression Eqn.	OUTLET
6012.00	1.20	34.60	27.40	**27.40**	Regression Eqn.	OUTLET
6013.00	1.20	39.40	31.08	**31.08**	Regression Eqn.	OUTLET
6014.00	1.20	43.20	34.36	**34.36**	Orifice Eqn.	OUTLET
6015.00	1.20	46.70	37.36	**37.36**	Orifice Eqn.	OUTLET
6016.00	1.20	49.90	40.10	**40.10**	Orifice Eqn.	OUTLET
6017.00	1.20	52.90	42.71	**42.71**	Orifice Eqn.	OUTLET
6018.00	1.20	55.80	45.12	**45.12**	Orifice Eqn.	OUTLET
6019.00	1.20	58.50	47.46	**47.46**	Orifice Eqn.	OUTLET
6020.00	1.20	61.10	49.66	**49.66**	Orifice Eqn.	OUTLET
6021.00	1.20	63.60	51.80	**51.80**	Orifice Eqn.	OUTLET
6022.00	1.20	66.00	53.81	**53.81**	Orifice Eqn.	OUTLET
6023.00	1.20	68.30	55.74	**55.74**	Orifice Eqn.	OUTLET
6024.00	1.20	70.50	57.62	**57.62**	Orifice Eqn.	OUTLET
6025.00	1.20	72.70	59.49	**59.49**	Orifice Eqn.	OUTLET
6026.00	1.20	74.80	61.22	**61.22**	Orifice Eqn.	OUTLET
6027.00	1.20	76.90	62.96	**62.96**	Orifice Eqn.	OUTLET
6028.00	1.20	78.90	64.63	**64.63**	Orifice Eqn.	OUTLET
6029.00	1.20	80.80	66.30	**66.30**	Orifice Eqn.	OUTLET
6030.00	1.20	82.70	67.91	**67.91**	Orifice Eqn.	OUTLET
6031.00	1.20	84.60	69.45	**69.45**	Orifice Eqn.	OUTLET
6032.00	1.20	86.40	70.98	**70.98**	Orifice Eqn.	OUTLET
6033.00	1.20	88.20	72.45	**72.45**	Orifice Eqn.	OUTLET
6034.00	1.20	89.90	73.92	**73.92**	Orifice Eqn.	OUTLET
6035.00	1.20	91.60	75.39	**75.39**	Orifice Eqn.	OUTLET
6036.00	1.20	93.30	76.80	**76.80**	Orifice Eqn.	OUTLET

Processing Time: 00.44 Seconds

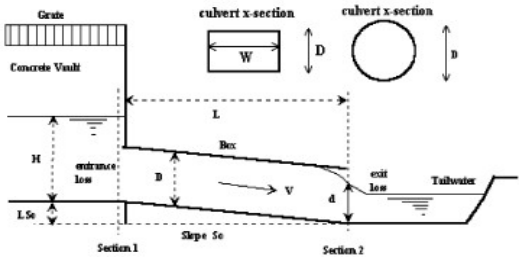
CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Sky Ranch
Basin ID: Pond B



CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfoot
Basin ID: Pond C Outlet Pipe
Status:



Design Information (Input):

Circular Culvert: Barrel Diameter in Inches — D = 48 inches
Inlet Edge Type (choose from pull-down list) — Grooved End with Headwall

OR:

Box Culvert: Barrel Height (Rise) in Feet — Height (Rise) = ft.
Barrel Width (Span) in Feet — Width (Span) = ft.
Inlet Edge Type (choose from pull-down list) — Square Edge w/ 30-78 deg. Flared Wingwall

Number of Barrels	No =	1
Inlet Elevation at Culvert Invert	Inlet Elev =	6007 ft. elev.
Outlet Elevation at Culvert Invert **OR** Slope of Culvert (ft v./ft h.)	Outlet Elev =	6006 ft. elev.
Culvert Length in Feet	L =	100 ft.
Manning's Roughness	n =	0.011
Bend Loss Coefficient	K_b =	0
Exit Loss Coefficient	K_x =	1

Design Information (calculated):

Entrance Loss Coefficient	K_e =	0.20
Friction Loss Coefficient	K_f =	0.35
Sum of All Loss Coefficients	K_s =	1.55
Orifice Inlet Condition Coefficient	C_d =	0.99
Minimum Energy Condition Coefficient	KE_{low} =	-0.0266

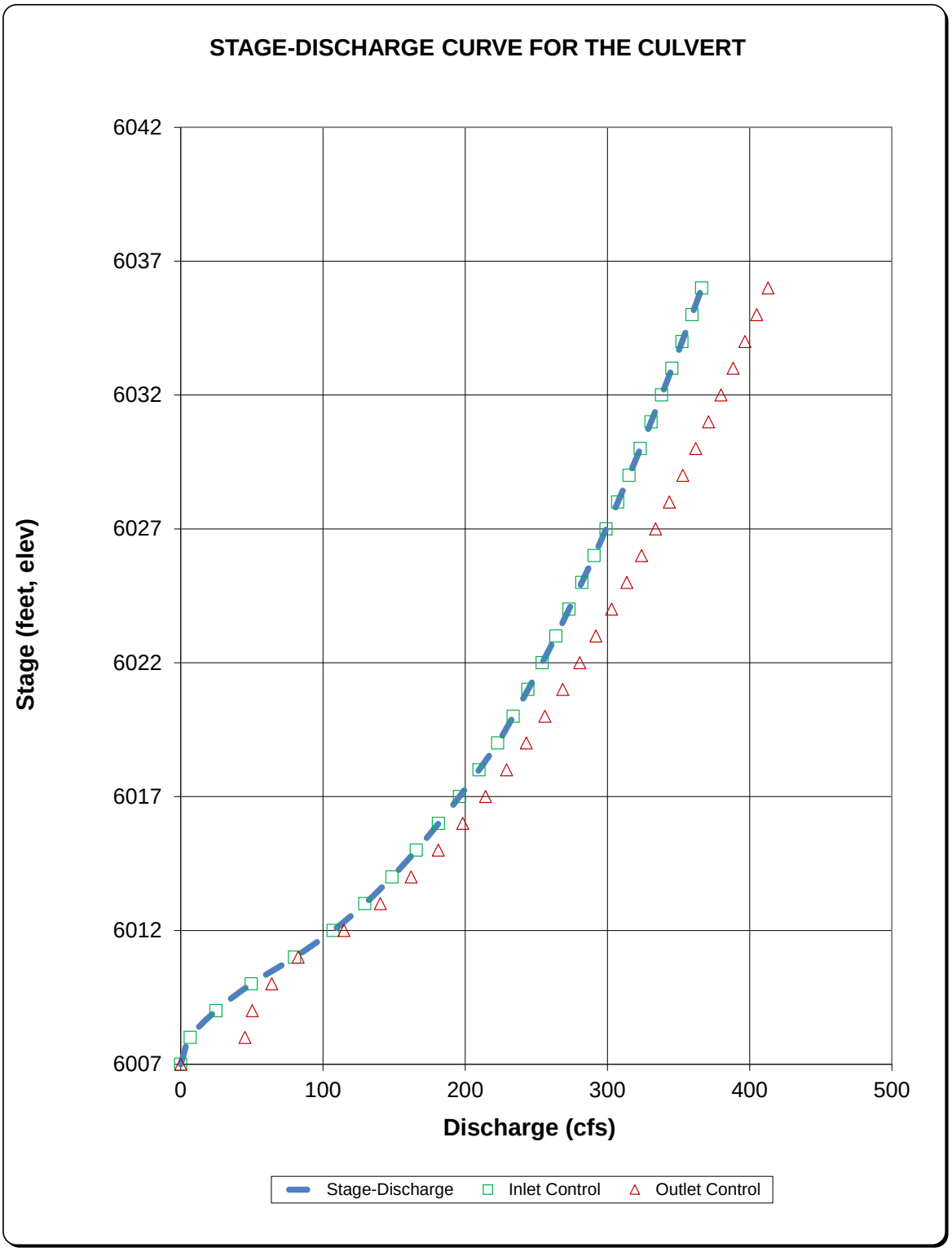
Calculations of Culvert Capacity (output):

Water Surface Elevation (ft., linked)	Tailwater Surface Elevation ft	Culvert Inlet-Control Flowrate cfs	Culvert Outlet-Control Flowrate cfs	Controlling Culvert Flowrate cfs (output)	Inlet Equation Used:	Flow Control Used
6007.00	1.20	0.00	0.00	**0.00**	No Flow (WS < inlet)	N/A
6008.00	1.20	6.70	45.16	**6.70**	Min. Energy. Eqn.	INLET
6009.00	1.20	24.80	50.33	**24.80**	Min. Energy. Eqn.	INLET
6010.00	1.20	49.60	64.08	**49.60**	Regression Eqn.	INLET
6011.00	1.20	80.10	82.67	**80.10**	Regression Eqn.	INLET
6012.00	1.20	107.20	114.84	**107.20**	Regression Eqn.	INLET
6013.00	1.20	129.50	140.39	**129.50**	Regression Eqn.	INLET
6014.00	1.20	148.60	162.04	**148.60**	Regression Eqn.	INLET
6015.00	1.20	165.70	181.13	**165.70**	Regression Eqn.	INLET
6016.00	1.20	181.30	198.36	**181.30**	Regression Eqn.	INLET
6017.00	1.20	196.00	214.32	**196.00**	Regression Eqn.	INLET
6018.00	1.20	209.80	229.09	**209.80**	Regression Eqn.	INLET
6019.00	1.20	222.90	243.01	**222.90**	Regression Eqn.	INLET
6020.00	1.20	233.80	256.08	**233.80**	Orifice Eqn.	INLET
6021.00	1.20	244.20	268.65	**244.20**	Orifice Eqn.	INLET
6022.00	1.20	254.20	280.53	**254.20**	Orifice Eqn.	INLET
6023.00	1.20	263.80	291.99	**263.80**	Orifice Eqn.	INLET
6024.00	1.20	273.00	303.02	**273.00**	Orifice Eqn.	INLET
6025.00	1.20	282.00	313.63	**282.00**	Orifice Eqn.	INLET
6026.00	1.20	290.70	323.99	**290.70**	Orifice Eqn.	INLET
6027.00	1.20	299.10	333.92	**299.10**	Orifice Eqn.	INLET
6028.00	1.20	307.30	343.59	**307.30**	Orifice Eqn.	INLET
6029.00	1.20	315.30	353.02	**315.30**	Orifice Eqn.	INLET
6030.00	1.20	323.10	362.18	**323.10**	Orifice Eqn.	INLET
6031.00	1.20	330.70	371.10	**330.70**	Orifice Eqn.	INLET
6032.00	1.20	338.10	379.84	**338.10**	Orifice Eqn.	INLET
6033.00	1.20	345.40	388.41	**345.40**	Orifice Eqn.	INLET
6034.00	1.20	352.50	396.73	**352.50**	Orifice Eqn.	INLET
6035.00	1.20	359.50	404.96	**359.50**	Orifice Eqn.	INLET
6036.00	1.20	366.30	412.94	**366.30**	Orifice Eqn.	INLET

Processing Time: 00.52 Seconds

CULVERT STAGE-DISCHARGE SIZING (INLET vs. OUTLET CONTROL WITH TAILWATER EFFECTS)

Project: Trails at Crowfoot
Basin ID: Pond C



IV. Copies of graphs, tables, and nomographs used

A. FIRM
B. Soils
C. 1-Hour Rainfall Data
D. Excerpts from adjacent Studies

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables shown on this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Floodway Data table shown on this FIRM.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD 83, GRS 1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, Maryland 20910-3282
(301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at **(301) 713- 3242**, or visit its website at http://www.ngs.noaa.gov.

Base map information shown on this FIRM was provided by the Douglas County GIS Department and the Town of Castle Rock GIS Department. Additional input was provided by the City of Lone Tree and Town of Parker. These data are current as of 2010.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **profile baselines** depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the **profile baseline**, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

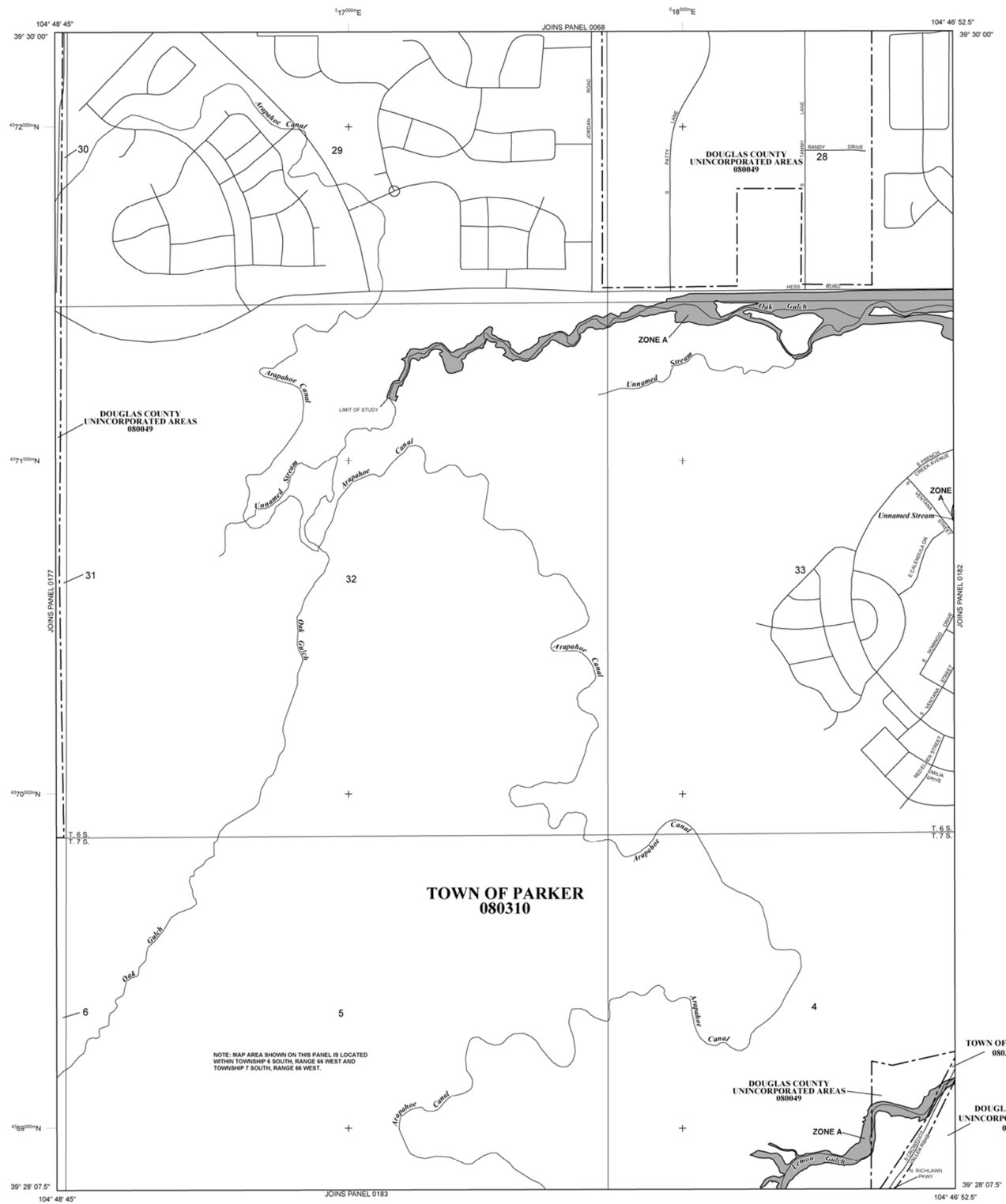
Based on updated topographic information, this map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. As a result, the Flood Profiles and Floodway Data tables for multiple streams in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unrevised streams may differ from what is shown on previous maps.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

For information on available products associated with this FIRM visit the **Map Service Center (MSC)** website at http://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the MSC website.

If you have **questions about this map,** how to order products, or the National Flood Insurance Program in general, please call the **FEMA Map Information eXchange (FMIX)** at **1-877-FEMA-MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.



LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Areas formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

	1% Annual Chance Floodplain Boundary
	0.2% Annual Chance Floodplain Boundary
	Floodway boundary
	Zone D boundary
	CBRS and OPA boundary
	Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities.
513	Base Flood Elevation line and value; elevation in feet*
(EL 987)	Base Flood Elevation value where uniform within zone; elevation in feet*

*Referenced to the North American Vertical Datum of 1988

A — A	Cross section line
23 — 23	Transect line
45° 02' 08", 93° 02' 12"	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83) Western Hemisphere
4989000mN	1000-meter Universal Transverse Mercator grid values, zone 13
DX5510 ×	Bench mark (see explanation in Notes to Users section of this FIRM panel)
M1.5	River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

SEPTEMBER 30, 2005

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

MARCH 16, 2016: to update corporate limits, to change base flood elevations, to add base flood elevations, to add special flood hazard areas, to update map format, to add roads and road names, to reflect updated topographic information, to incorporate previously issued letters of map revision.

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP
NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0181G

FIRM

FLOOD INSURANCE RATE MAP

DOUGLAS COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 181 OF 495
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
DOUGLAS COUNTY	080049	0181	G
PARKER, TOWN OF	080310	0181	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08035C0181G
MAP REVISED
MARCH 16, 2016

Federal Emergency Management Agency

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables shown on this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Floodway Data table shown on this FIRM.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD 83, GRS 1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, Maryland 20910-3282
(301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at **(301) 713- 3242**, or visit its website at http://www.ngs.noaa.gov.

Base map information shown on this FIRM was provided by the Douglas County GIS Department and the Town of Castle Rock GIS Department. Additional input was provided by the City of Lone Tree and Town of Parker. These data are current as of 2010.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **profile baselines** depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the **profile baseline**, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

Based on updated topographic information, this map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. As a result, the Flood Profiles and Floodway Data tables for multiple streams in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unrevised streams may differ from what is shown on previous maps.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

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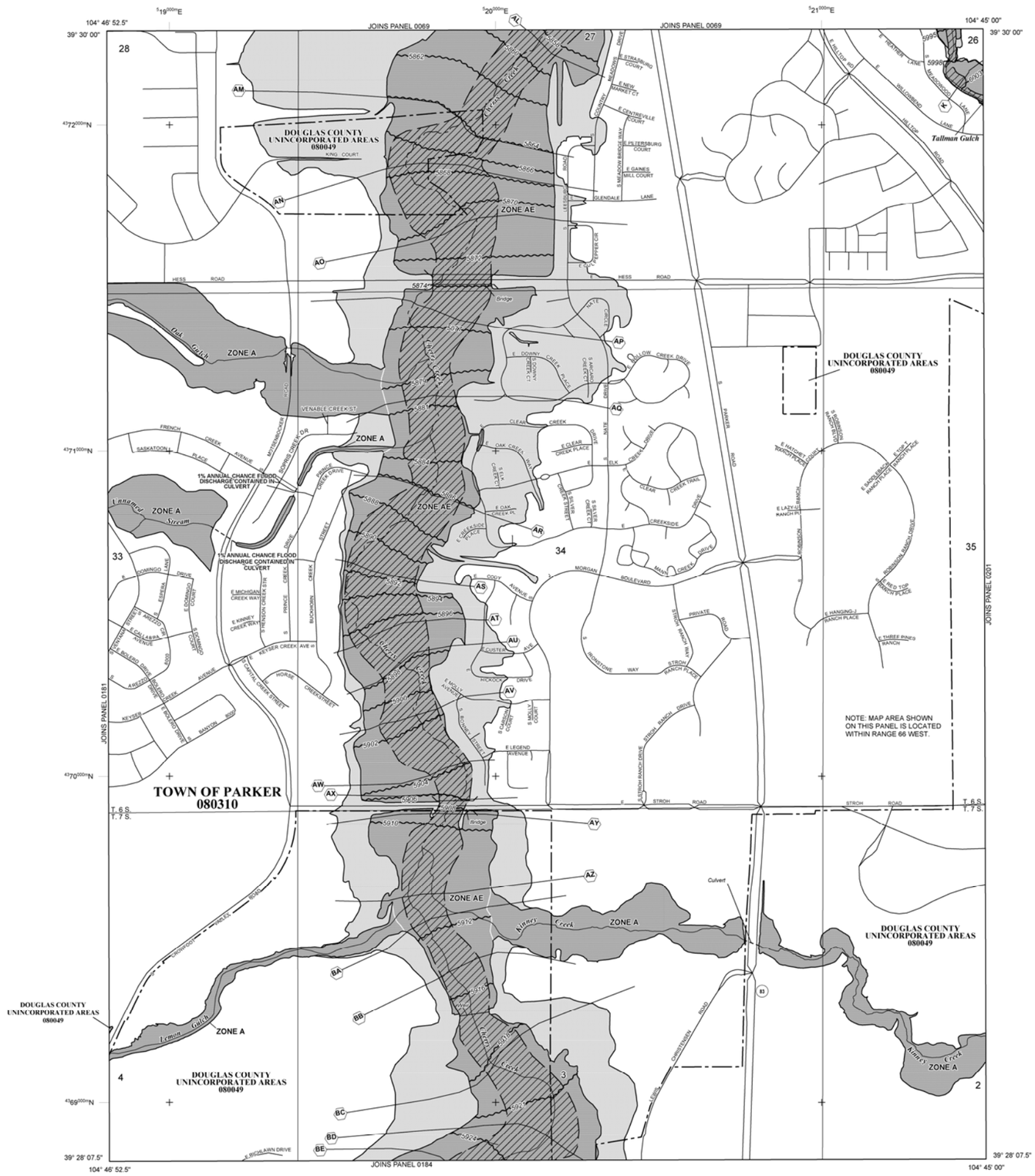

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Areas formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% Annual Chance Floodplain Boundary
- 0.2% Annual Chance Floodplain Boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities.
- 513 — Base Flood Elevation line and value; elevation in feet*
- (EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

*Referenced to the North American Vertical Datum of 1988

- A — A — Cross section line
- 23 — 23 — Transect line
- 45° 02' 08", 93° 02' 12" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83) Western Hemisphere
- 4369000mN — 1000-meter Universal Transverse Mercator grid values, zone 13
- DX5510 × — Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 • — River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

SEPTEMBER 30, 2005

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

MARCH 16, 2016: to update corporate limits, to change base flood elevations, to add base flood elevations, to add special flood hazard areas, to update map format, to add roads and road names, to reflect updated topographic information, to incorporate previously issued letters of map revision.

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP

NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0182G

FIRM

FLOOD INSURANCE RATE MAP

DOUGLAS COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 182 OF 495
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
DOUGLAS COUNTY	080049	0182	G
PARKER, TOWN OF	080310	0182	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08035C0182G
MAP REVISED
MARCH 16, 2016

Federal Emergency Management Agency

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables shown on this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Floodway Data table shown on this FIRM.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD 83, GRS 1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

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National Geodetic Survey
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Silver Spring, Maryland 20910-3282
(301) 713-3242

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Base map information shown on this FIRM was provided by the Douglas County GIS Department and the Town of Castle Rock GIS Department. Additional input was provided by the City of Lone Tree and Town of Parker. These data are current as of 2010.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **profile baselines** depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the **profile baseline**, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

Based on updated topographic information, this map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. As a result, the Flood Profiles and Floodway Data tables for multiple streams in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unrevised streams may differ from what is shown on previous maps.

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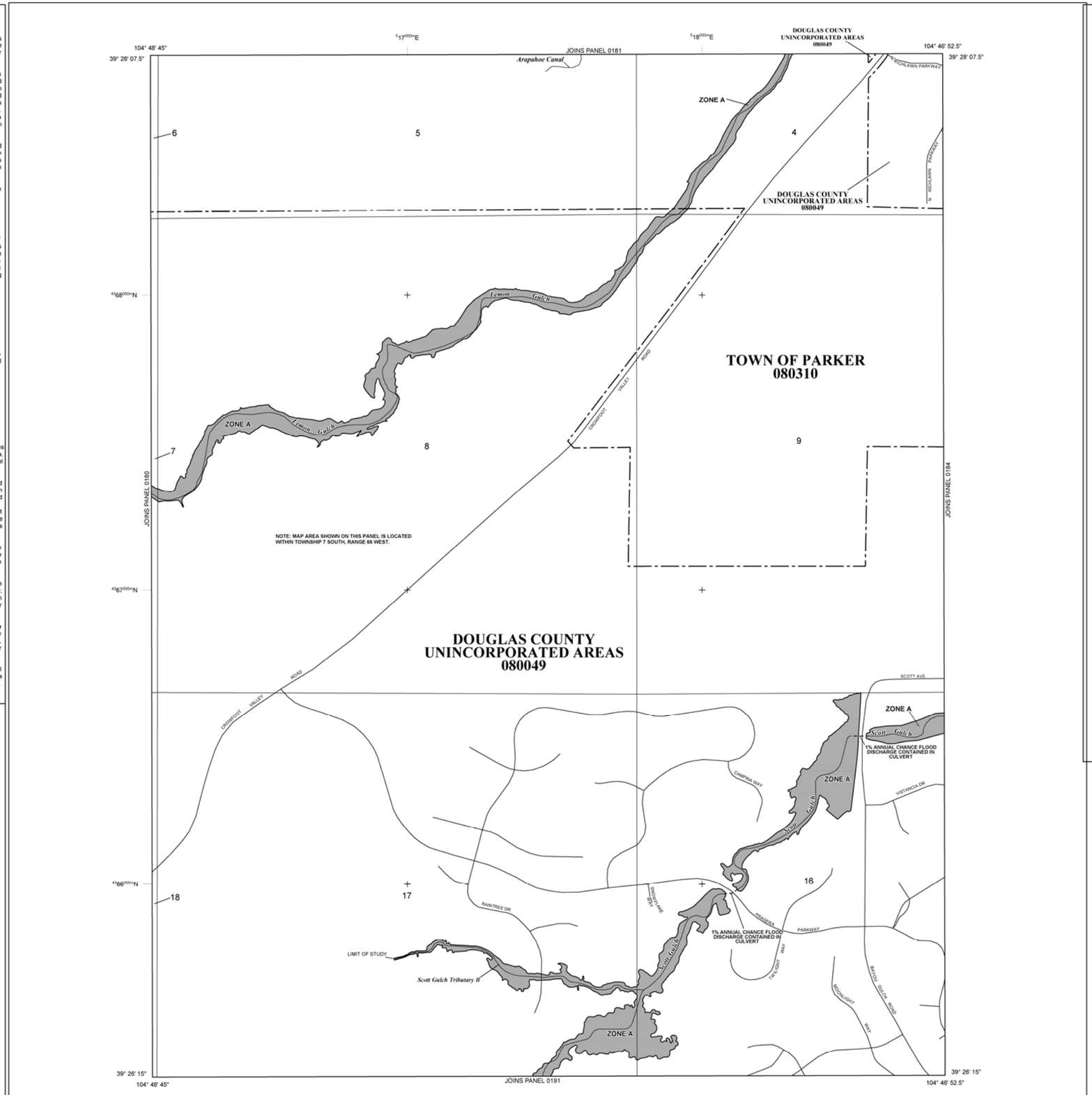

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Areas formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% Annual Chance Floodplain Boundary
- 0.2% Annual Chance Floodplain Boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities.
- 513 — Base Flood Elevation line and value; elevation in feet*
- (EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

*Referenced to the North American Vertical Datum of 1988

- A — A — Cross section line
- 23 — 23 — Transect line
- 45° 02' 08", 93° 02' 12" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83) Western Hemisphere
- $^{49}89^{000m}$N — 1000-meter Universal Transverse Mercator grid values, zone 13
- DX5510 × — Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 • — River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

SEPTEMBER 30, 2005

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

MARCH 16, 2016: to update corporate limits, to change base flood elevations, to add base flood elevations, to add special flood hazard areas, to update map format, to add roads and road names, to reflect updated topographic information, to incorporate previously issued letters of map revision.

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MAP SCALE 1" = 500'

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0183G

FIRM

FLOOD INSURANCE RATE MAP

DOUGLAS COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 183 OF 495
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
DOUGLAS COUNTY	080049	0183	G
PARKER, TOWN OF	080310	0183	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08035C0183G
MAP REVISED
MARCH 16, 2016

Federal Emergency Management Agency

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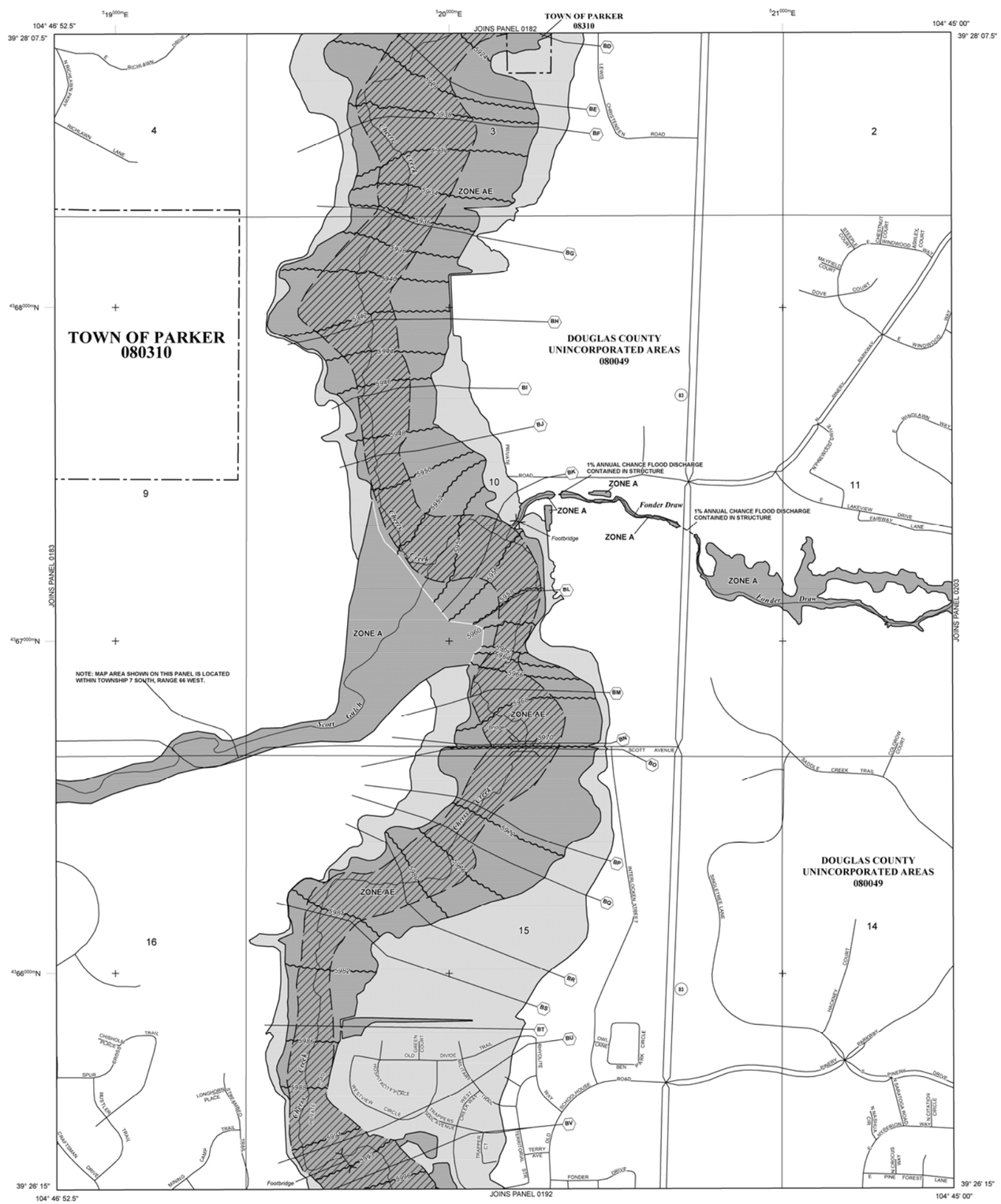

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

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ZONE A	No Base Flood Elevations determined.
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ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
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COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% Annual Chance Floodplain Boundary
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- Floodway boundary
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- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities.
- 513 — Base Flood Elevation line and value; elevation in feet*
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*Referenced to the North American Vertical Datum of 1988

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- 4989000mN — 1000-meter Universal Transverse Mercator grid values, zone 13
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MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

SEPTEMBER 30, 2005

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

MARCH 16, 2016: to update corporate limits, to change base flood elevations, to add base flood elevations, to add special flood hazard areas, to update map format, to add roads and road names, to reflect updated topographic information, to incorporate previously issued letters of map revision.

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0184G

FIRM

FLOOD INSURANCE RATE MAP

DOUGLAS COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 184 OF 495
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

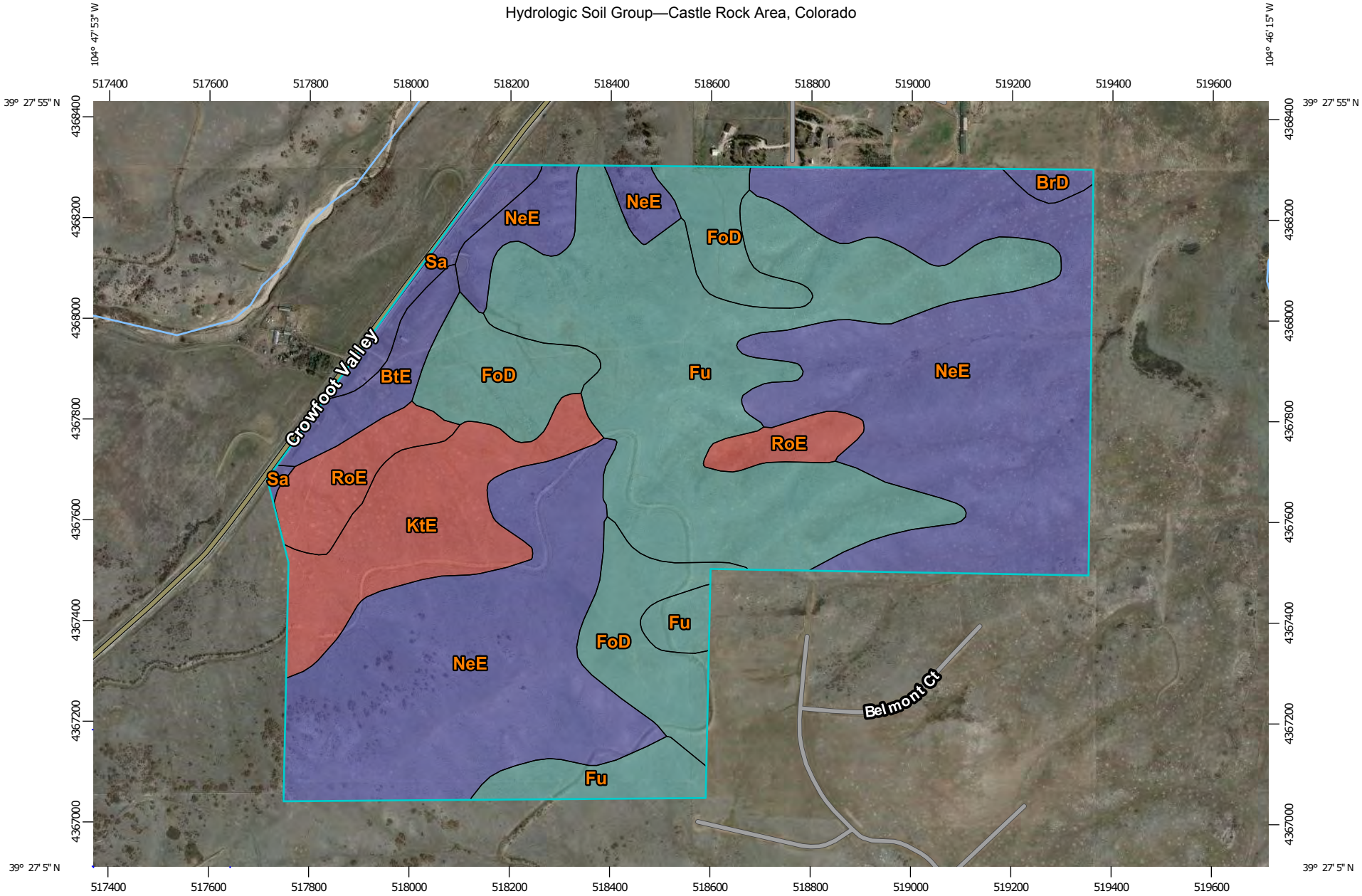
CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
DOUGLAS COUNTY	080049	0184	G
PARKER, TOWN OF	080310	0184	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08035C0184G
MAP REVISED
MARCH 16, 2016

Federal Emergency Management Agency


104° 47' 53" W
104° 46' 15" W
39° 27' 55" N
39° 27' 5" N
517400
517600
517800
518000
518200
518400
518600
518800
519000
519200
519400
519600
4368400
4368200
4368000
4367800
4367600
4367400
4367200
4367000
NeE
Sa
FoD
BrD
Fu
BtE
RoE
KtE
Crowfoot Valley
Belmont Ct
Map Scale: 1:10,700 if printed on A landscape (11" x 8.5") sheet.
N
Meters
0 150 300 600 900
Feet
0 500 1000 2000 3000
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Castle Rock Area, Colorado
Survey Area Data: Version 8, Sep 23, 2014

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 16, 2012—Apr 13, 2012

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Castle Rock Area, Colorado (CO622)				
Map unit symbol	**Map unit name**	**Rating**	**Acres in AOI**	**Percent of AOI**
BrD	Bresser sandy loam, 3 to 9 percent slopes	B	2.1	0.5%
BtE	Bresser-Truckton sandy loams, 5 to 25 percent slopes	B	9.1	2.4%
FoD	Fondis clay loam, 3 to 9 percent slopes	C	41.3	10.7%
Fu	Fondis-Kutch association	C	105.5	27.3%
KtE	Kutch sandy loam, 5 to 20 percent slopes	D	30.5	7.9%
NeE	Newlin gravelly sandy loam, 8 to 30 percent slopes	B	174.1	45.0%
RoE	Renohill sandy loam, reddish variant, 5 to 20 percent slopes	D	16.5	4.3%
Sa	Sampson loam	B	7.5	1.9%
Totals for Area of Interest			**386.5**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

TABLE 5.1
ONE-HOUR POINT RAINFALL

Frequency of Design Event (yr)	One-hour Point Rainfall, P_1 (in)
2	0.99
5	1.39
10	1.64
25	1.98
50	2.31
100	2.60

5.3 FLOOD HYDROLOGY OVERVIEW

Various methods exist to determine appropriate flood peaks or hydrographs for storm drainage planning and design. Methods for determining flood peaks or hydrographs are the Rational Method, the Colorado Urban Hydrograph Procedure (CUHP), and Urban Drainage Stormwater Management (UDSWM) model. The Town of Parker discourages the use of computer models other than CUHP and UDSWM since these programs are preferred, if not required, by UDFCD for studies involving major drainageways where UDFCD approval is sought or where maintenance eligibility is requested.

The three methods are briefly described in this section, and a discussion of their applicability to the Town of Parker is discussed. UDSWM is mostly used to combine and route the hydrographs generated using CUHP.

In general, the Rational Method is the most widely used and accepted technique for determining peak flows in urban areas for small basins. Within the constraints outlined in the MANUAL, use of the Rational Method provides a relatively simple but effective way to analyze storm runoff.

CUHP is somewhat more complicated than the Rational Method. It allows a manual computation of a runoff hydrograph which may be used for further hydraulic routing through channels and/or detention ponds. Historically, CUHP is best used in urban areas for which runoff coefficients have been derived. However, recent improvements by UDFCD include consideration for different soil types, thus CUHP is now more applicable to rural areas. The reader is referred to UDFCD for the latest version of CUHP.

UDSWM is a computer model that generates runoff hydrographs and routes and combines these hydrographs. UDSWM is a modified version of the Runoff Block of the Environmental Protection Agency's Storm Water Management Model (SWMM). It has been modified to be used in conjunction with CUHP. Table 5.2 herein provides guidance on selecting the appropriate method for a given project.

Conceptual Drainage Report

Hess Ranch Development
Parker, Colorado

CODE: SRD01

Prepared For:
SDI Inc.
5105 DTC Parkway, Suite 240
Greenwood Village, Colorado 80111
Contact: Gary Hunter
720-482-7707

Prepared By:

Manhard CONSULTING

7442 South Tucson Way, Suite 190-A
Centennial, Colorado 80112
Contact: Russell Burrows, P.E.
303-708-0500

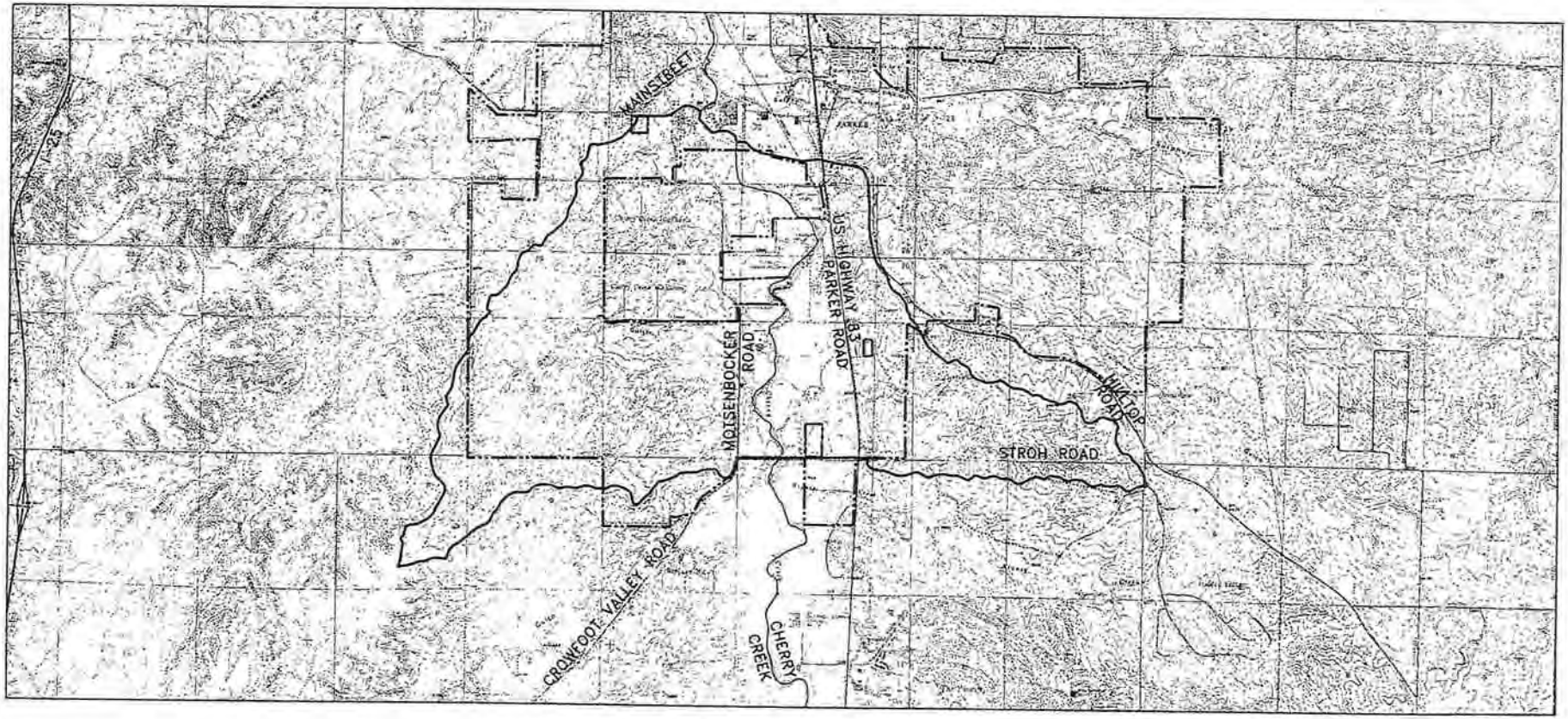
Initially Submitted: November 25, 2014
Revised: February 15, 2015
Updated: June 1, 2015

APPENDIX D

Excerpts from Oak Gulch & Stroh Ranch Area Outfall Systems Planning

OAK GULCH AND STROH RANCH AREA
OUTFALL SYSTEMS PLANNING

PRELIMINARY DESIGN REPORT



TOWN OF PARKER DOUGLAS COUNTY
URBAN DRAINAGE AND FLOOD CONTROL DISTRICT
FEBRUARY 2003

Prepared by:
Knight Piésold CONSULTING
1050 17th Street, Suite 500
Denver, Colorado 80265-0500

II. STUDY AREA DESCRIPTION

A. General

The Oak Gulch and Stroh Ranch study area is located in Douglas County and partially within the southern portion of the Town of Parker. It encompasses the tributary areas on both the east and west sides of Cherry Creek between Main street on the north and Stroh Road on the south. The area contains several major drainageways, including Oak Gulch, smaller unnamed drainageways, and direct flow areas that discharge along a 3.6-mile reach of Cherry Creek. Cherry Creek flows from south to north through the study area. Figures II-1 and II-2 show the general location of the study area and existing facilities in the study area, respectively.

Presently, a portion of the 7.7-mi^2 (4,955-acre) study area lies within the corporate limits of the Town of Parker; those areas that are not within the corporate limits are under the jurisdiction of Douglas County. It should be noted that in the future much of the study area might eventually be annexed into the Town of Parker.

At the time of this study, the area comprised a mixture of residential, commercial, and open areas. The residential areas range from low-density with several acres per residence to high-density with as many as six units per acre. The open areas include native open space, undeveloped land, parks, greenbelts, sod farms (agricultural), and the Cherry Creek floodplain. There are also many areas that are presently being developed for various residential densities and commercial sites, and several more areas have already been proposed for development.

The major arterial roads through the study area include Parker Road (State Highway 83), Motsenbocker Road, and Stroh Road. Several minor collector and local roads, both paved and unpaved, are also within the study area.

B. Topography

The study area generally slopes toward Cherry Creek from both the east and west sides. West of Cherry Creek, slopes typically range from 2 to 4 percent in the upper reaches of the catchments, from 1 to 2 percent in the middle reaches, and are generally less than 1 percent in the lower reaches near the Cherry Creek floodplain. East of Cherry Creek, slopes are typically 3 to 6 percent in the upper reaches, 1 to 3 percent in the middle reaches, and less than 1 percent in the lower reaches. The topography, represented by 25-foot contour intervals, is shown on Figure II-3.

C. Soil Characteristics

Soil classifications throughout the study area were determined from the Soil Survey of Castle Rock Area, Colorado (Reference 1). Areas with common hydrologic soil properties were delineated based on their Hydrologic Soils Group classifications; these areas are shown on Figure II-4. The study area contains all four Hydrologic Soils Group classifications but primarily comprises Type B (moderately high infiltration and moderately low runoff potential) and Type C (moderately low infiltration and moderately high runoff potential) soils. There are, however, small areas of Type A (high infiltration and low runoff potential) and Type D (low infiltration and high runoff potential) soils. In general, those areas west of Cherry Creek have predominantly Type C soils while those areas east of Cherry Creek have predominantly Type B soils.

D. Catchment Imperviousness

The study area was divided into regions of similar hydrologic response based on the existing and proposed future imperviousness. For purposes of this study, four categories of imperviousness were used to generalize the entire study area. Areas with similar imperviousness under existing conditions were outlined from aerial photography of the project area; these areas are shown on Figure II-5. Areas with similar imperviousness for the proposed future conditions were outlined based on information from submitted drainage reports for various developments as well as from discussions with representatives of Parker and the County; these areas are shown on Figure II-6. The imperviousness values used to represent the four distinct areas were developed from Table RO-3 and Figures RO-3 through RO-5 of the Manual. The following values were used:

- 95 percent impervious for areas with many structures, buildings, large paved areas such as parking lots, or other impervious structures; typical for commercial or industrial-type developments.
- 45 percent impervious for areas intermixed with pervious and impervious features; typical for residential or business areas with open space, lawns, or greenbelts situated among buildings, houses, or other structures.
- 40 percent for the West Stroh and North Crowfoot Valley catchments, due to Town of Parker accepted plans in this area.
- 18 percent impervious for areas with fewer structures and more open space, typical for rural residential areas with large lots, few houses, roadways, or other structures.
- 2 percent impervious for generally open areas; typical for open space, parks, or undeveloped land with only an occasional roadway, rock outcropping, or other impervious feature.

E. Sub-Catchment Descriptions

The study area was divided into eight major catchments, which were further divided into 91 sub-catchments. The catchment and sub-catchment boundaries are shown on Figures II-3 through II-6. These boundaries were developed based on topography, existing drainage facilities, aerial photography, and field observations. The sub-catchment delineation of Oak Gulch and the two unnamed tributaries immediately south of Oak Gulch were obtained from the Flood Hazard Area Delineation (FHAD) for Oak Gulch and Lemon Gulch (Reference 7). Since Oak Gulch is the only previously named tributary in the study area, the major catchments were assigned convenient names for identification in this report.

Oak Gulch Catchment

The Oak Gulch Catchment is located on the west side of Cherry Creek in the southwest portion of the study area. The 1.8-mi^2 (1,142-acre) tributary area comprises 16 sub-catchments. Presently, this area is in a relatively native condition and generally undeveloped. However, the Master Drainage Plan for Stroh Ranch provides extensive development plans that primarily include residential developments, a golf course, and some commercial sites (Reference 8 and 43). This master drainage plan calls for six detention basins and channel improvements. Runoff generated from this catchment culminates at an 11-foot-wide by 8-foot-high reinforced concrete box (RCB) culvert that passes under Motsenbocker Road. From Motsenbocker Road to the outfall into Cherry Creek, there is an improved drainageway with existing check structures and an on-site retention pond.

West Stroh Catchment

The West Stroh Catchment is located on the west side of Cherry Creek immediately south of Oak Gulch. The 1.0-mi^2 (664-acre) tributary area comprises 13 sub-catchments. This area is presently in a relatively native condition and undeveloped. However, development plans for the portion between Motsenbocker Road and Cherry Creek have been prepared through Stroh Ranch Filing No. 9 (Reference 9). Additionally, development plans for the area west of Motsenbocker Road have been prepared through Stroh Ranch Filing No. 12 (Reference 10), Stroh Ranch Filing No. 13 (Reference 11), and Stroh Ranch Filing Nos. 17, 18, and 19 (Reference 12). An existing regional detention basin is located adjacent to Motsenbocker Road. This pond has a maximum storage capacity of 80.90 acre-feet, which includes 11.48 acre-feet for water quality and discharges at a peak rate of 516 cfs for the 100-year storm event (References 13 and 14). The discharge from the detention basin passes under Motsenbocker Road through a 48-inch reinforced concrete pipe (RCP) culvert that carries the frequent low flows and a 10-foot by 10-foot RCB culvert for additional flows. The RCB culvert is also intended to be used as a pedestrian crossing. Although not constructed at the time of this report, improvements to the drainageway between Motsenbocker Road and Cherry Creek have been designed through Stroh Ranch Filing No. 9.

North Crowfoot Valley Catchment

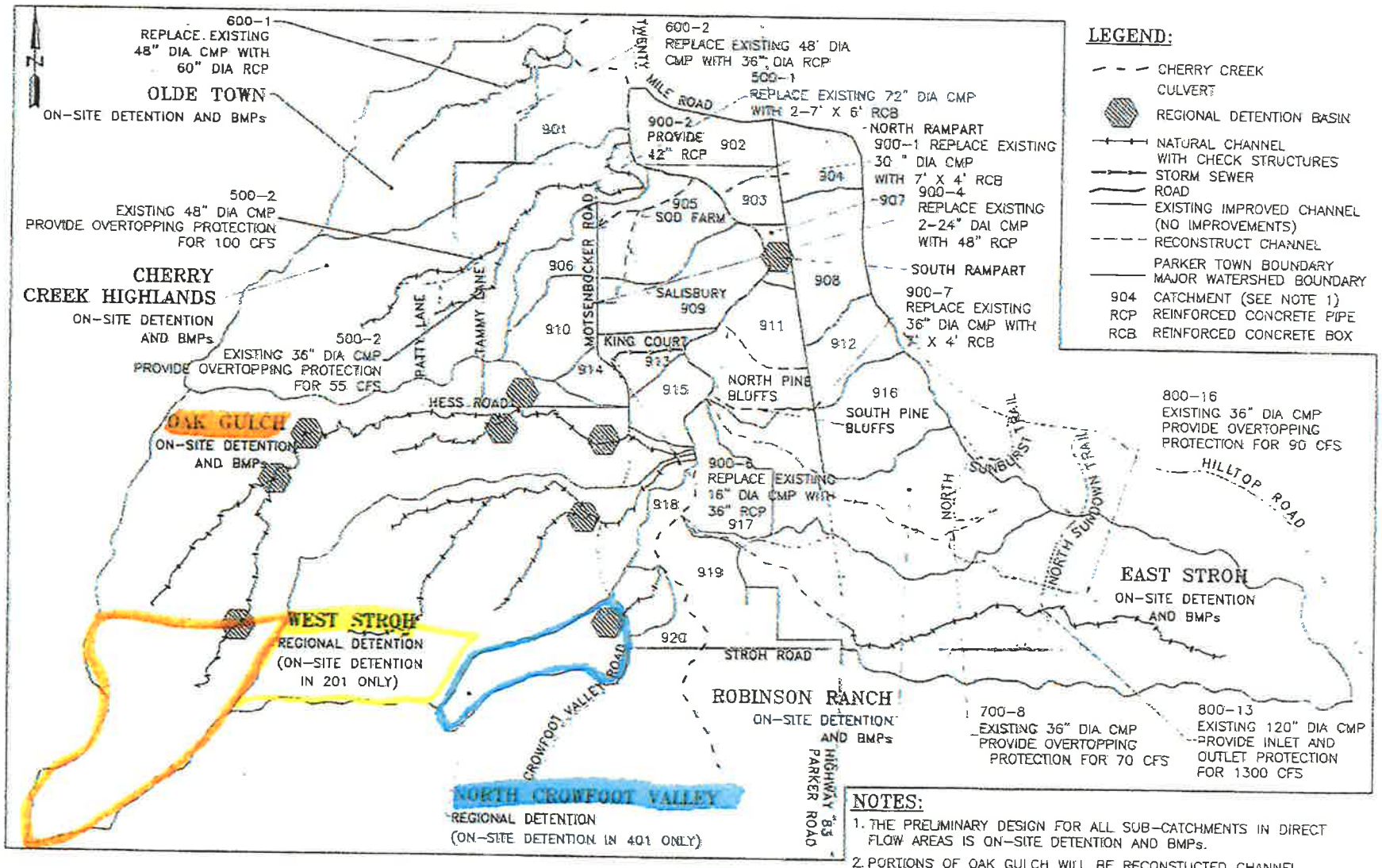
The North Crowfoot Valley Catchment is located on the west side of Cherry Creek immediately south of the West Stroh Catchment. The 0.2-mi^2 (132-acre) tributary area comprises two sub-catchments. This area is presently in a relatively native condition and undeveloped. However, development plans for the portion between Motsenbocker Road and Cherry Creek have been prepared through Stroh Ranch Filing No. 9 (Reference 9). Additionally, development plans for the area west of Motsenbocker Road have been prepared through Stroh Ranch Filing No. 13 (Reference 11) and Stroh Ranch Filing Nos. 17, 18, and 19 (Reference 12). An existing regional detention basin is located immediately adjacent to Motsenbocker Road. This pond has a maximum storage capacity of 12.15 acre-feet, which includes 1.54 acre-feet for water quality, and discharges at a rate 72 cfs for the 100-year storm event (References 13 and 14). The discharge from the detention basin passes under Motsenbocker Road through a 30-inch RCP culvert. Although not constructed at the time of this report, improvements to the drainageway between Motsenbocker Road and Cherry Creek have been designed through Stroh Ranch Filing No. 9.

Cherry Creek Highlands Catchment

The Cherry Creek Highlands Catchment is located on the west side of Cherry Creek immediately north of Oak Gulch. The 0.9-mi^2 (601-acre) tributary area comprises 11 sub-catchments. A portion of this area comprises the Cherry Creek Highlands development, which is an existing residential neighborhood in which each residence occupies several acres of land. The remaining portion is primarily in a native condition and undeveloped. At the time of this study, no further development plans existed. However, development is expected to continue in a manner similar to the Cherry Creek Highlands development for the upper portion of the catchment and may include a small area of commercial or light industrial development adjacent to Motsenbocker Road. Two distinct drainageways convey runoff from the upper reaches of the catchment. Runoff from the south reach passes under Patty Lane through a 36-inch corrugated metal pipe (CMP) culvert and then under Tammy Lane through another 36-inch CMP culvert. Runoff from the north reach passes under Tammy Lane through a 48-inch CMP culvert. Discharge from these two reaches merges into a single drainageway that passes under Motsenbocker Road through a 72-inch CMP culvert and finally discharges into Cherry Creek.

Olde Town Catchment

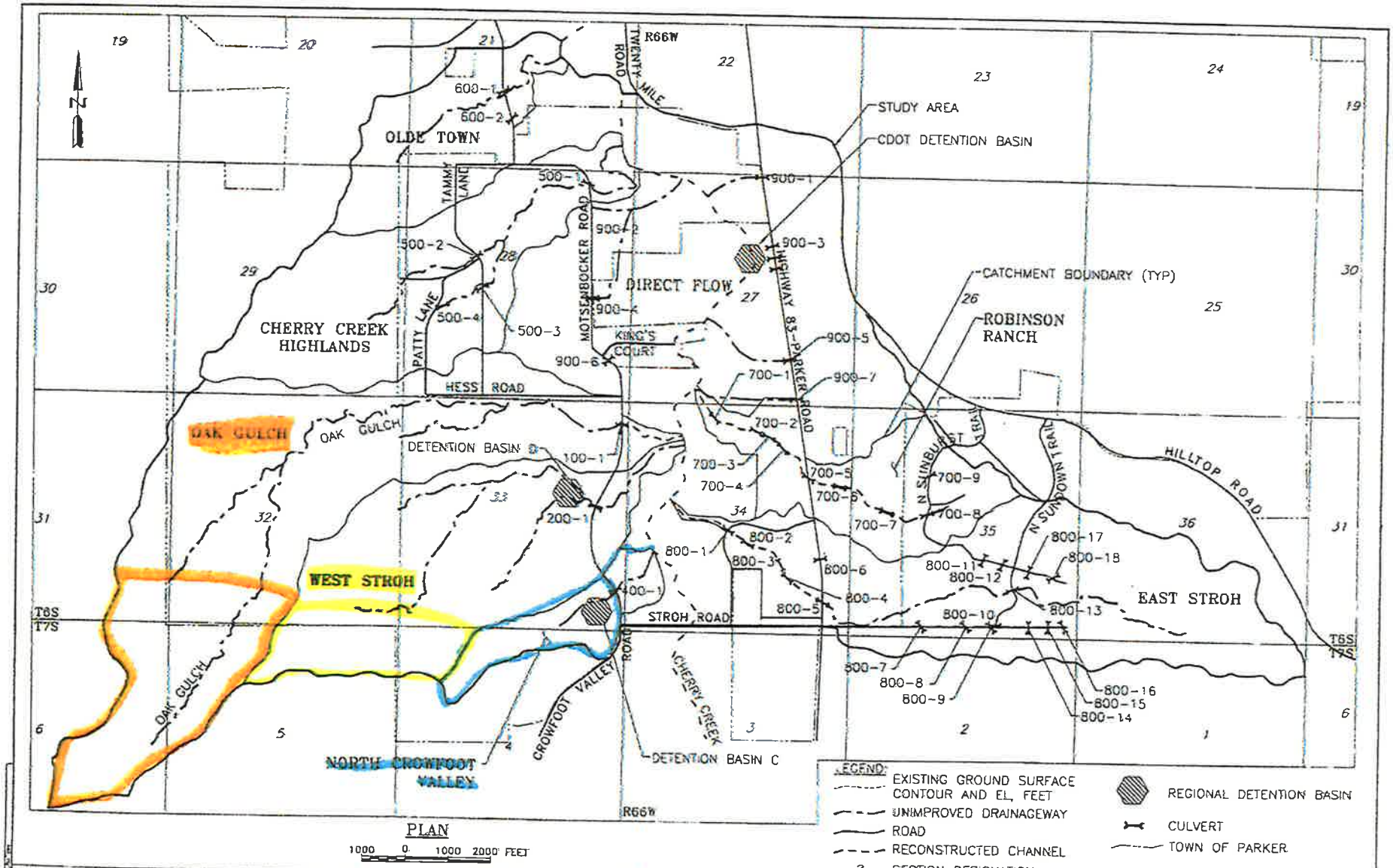
The Olde Town Catchment is located on the west side of Cherry Creek immediately north of the Cherry Creek Highlands Catchment. The 0.5-mi^2 (306-acre) tributary comprises four sub-catchments. A small portion of this catchment comprises the Cherry Creek Highlands neighborhood. The remaining area is primarily in a native condition and undeveloped. MMcEngineering, Inc. has prepared drainage plans for the portion west of Motsenbocker Road (Reference 15). This portion of the catchment includes high-density residential neighborhoods. Two distinct drainageways convey runoff from the upper portions of the catchment. Runoff from these areas passes under Motsenbocker Road separately through two 48-inch CMP



NOTES:

1. THE PRELIMINARY DESIGN FOR ALL SUB-CATCHMENTS IN DIRECT FLOW AREAS IS ON-SITE DETENTION AND BMPs.
2. PORTIONS OF OAK GULCH WILL BE RECONSTUCTED CHANNEL.
3. SEE DETAILED PRELIMINARY PLAN SHEETS FOR LOCATIONS OF CHANNEL STRUCTURES.

Knight Piésold CONSULTING	TOWN OF PARKER * DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	RECOMMENDED PRELIMINARY PLAN	FIGURE ES-2



PLAN

1000 0 1000 2000 FEET

Knight Piésold CONSULTING

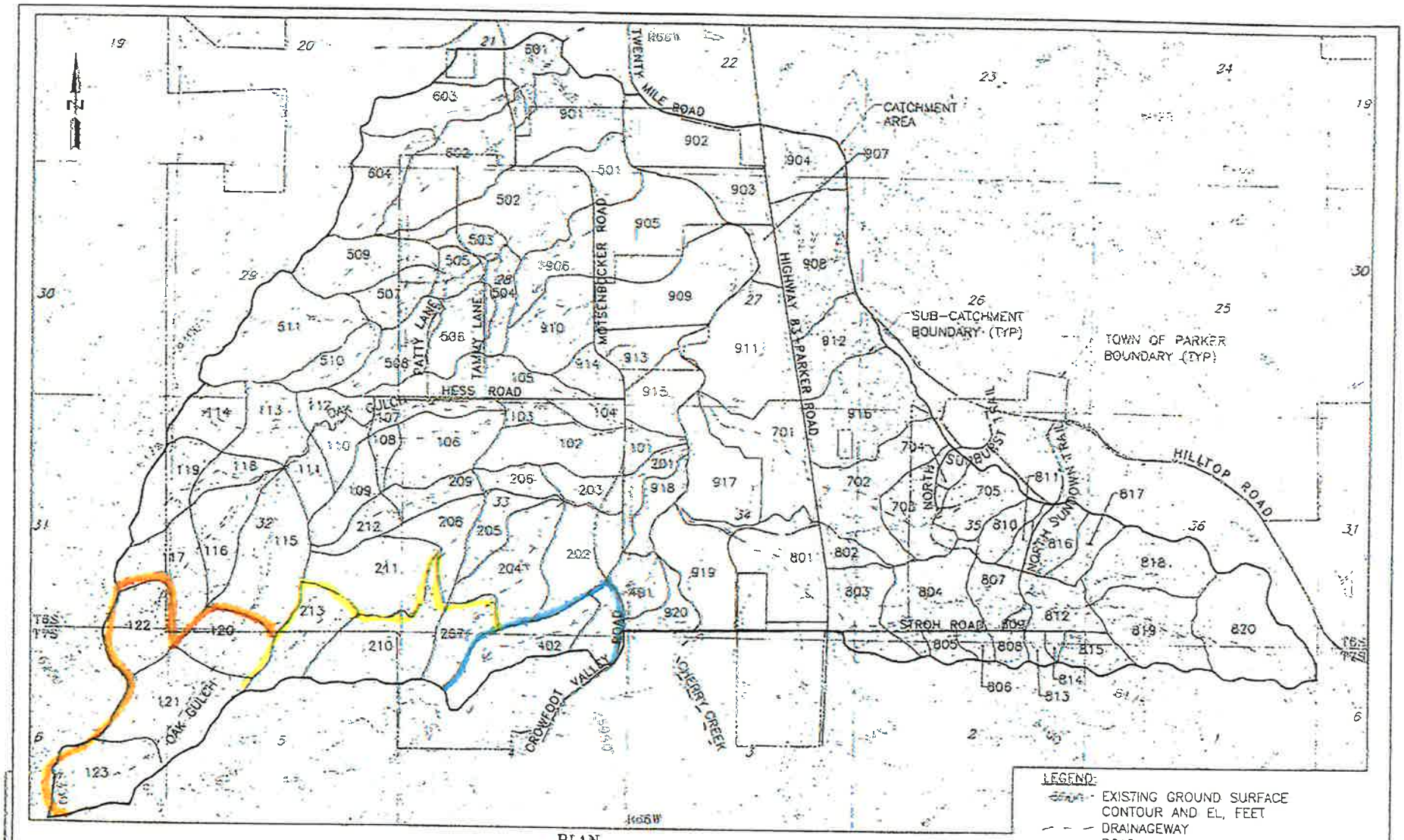
DESIGNED: JAJ DATE: 11/12/02
DRAWN: JAJ DATE: 11/12/02
CHECKED: JRK DATE: 11/12/02
REVISED: DJA DATE: 2/27/03

TOWN OF PARKER * DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT

OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING

EXISTING FACILITIES MAP

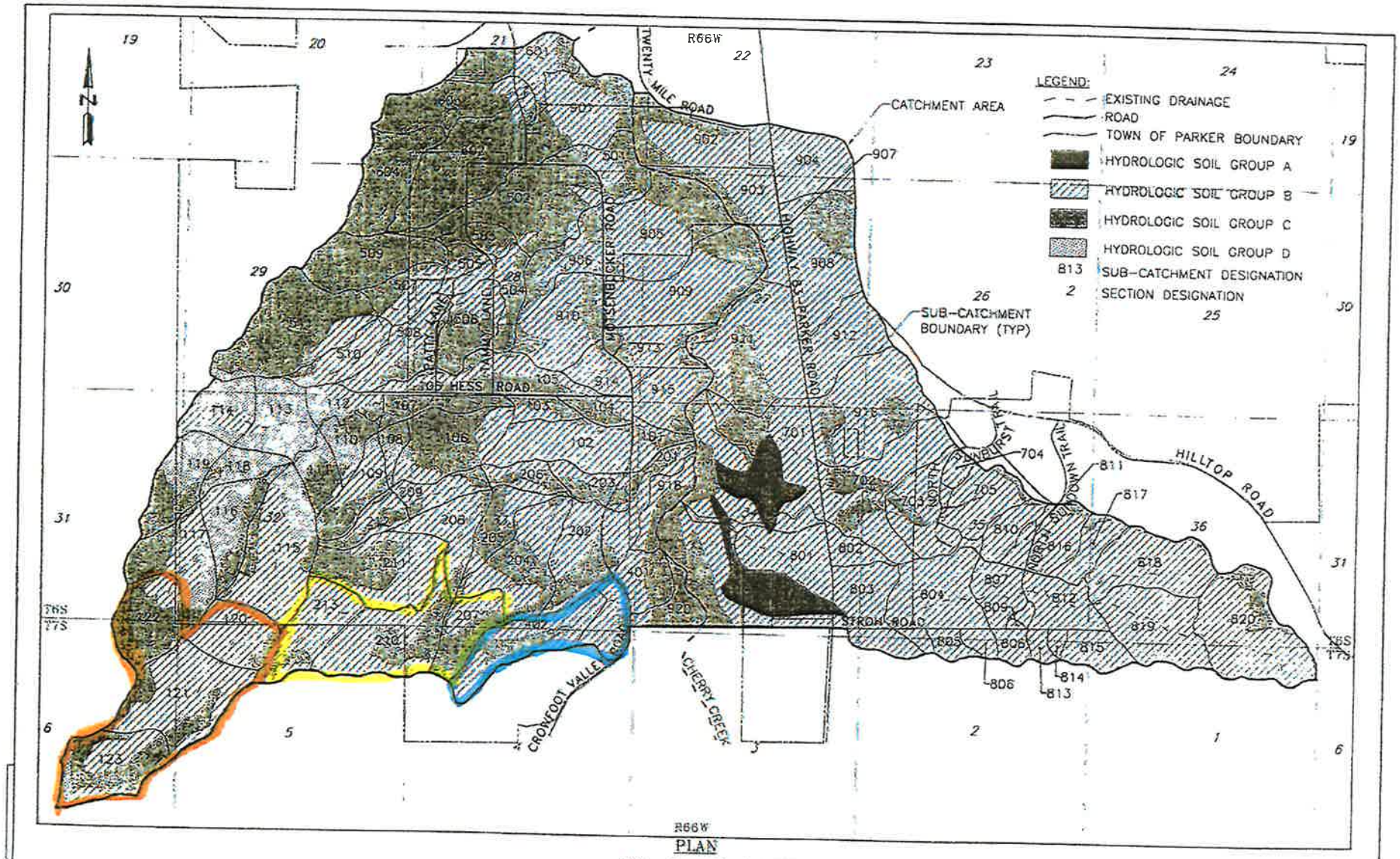
FIGURE II-2



PLAN

1000 0 1000 2000 FEET

Knight Piésold CONSULTING	DESIGNED: MDS DATE: 9/25/01 DRAWN: DS DATE: 9/25/01 CHECKED: DTW DATE: 9/25/01 REVISED: DJA DATE: 2/27/02	TOWN OF PARKER * DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	SUB-CATCHMENT AND TOPGRAPHY MAP	FIGURE II-3



REFERENCE
U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE "SOIL SURVEY OF CASTLEROCK AREA, COLORADO" 1974

Knight Piésold CONSULTING

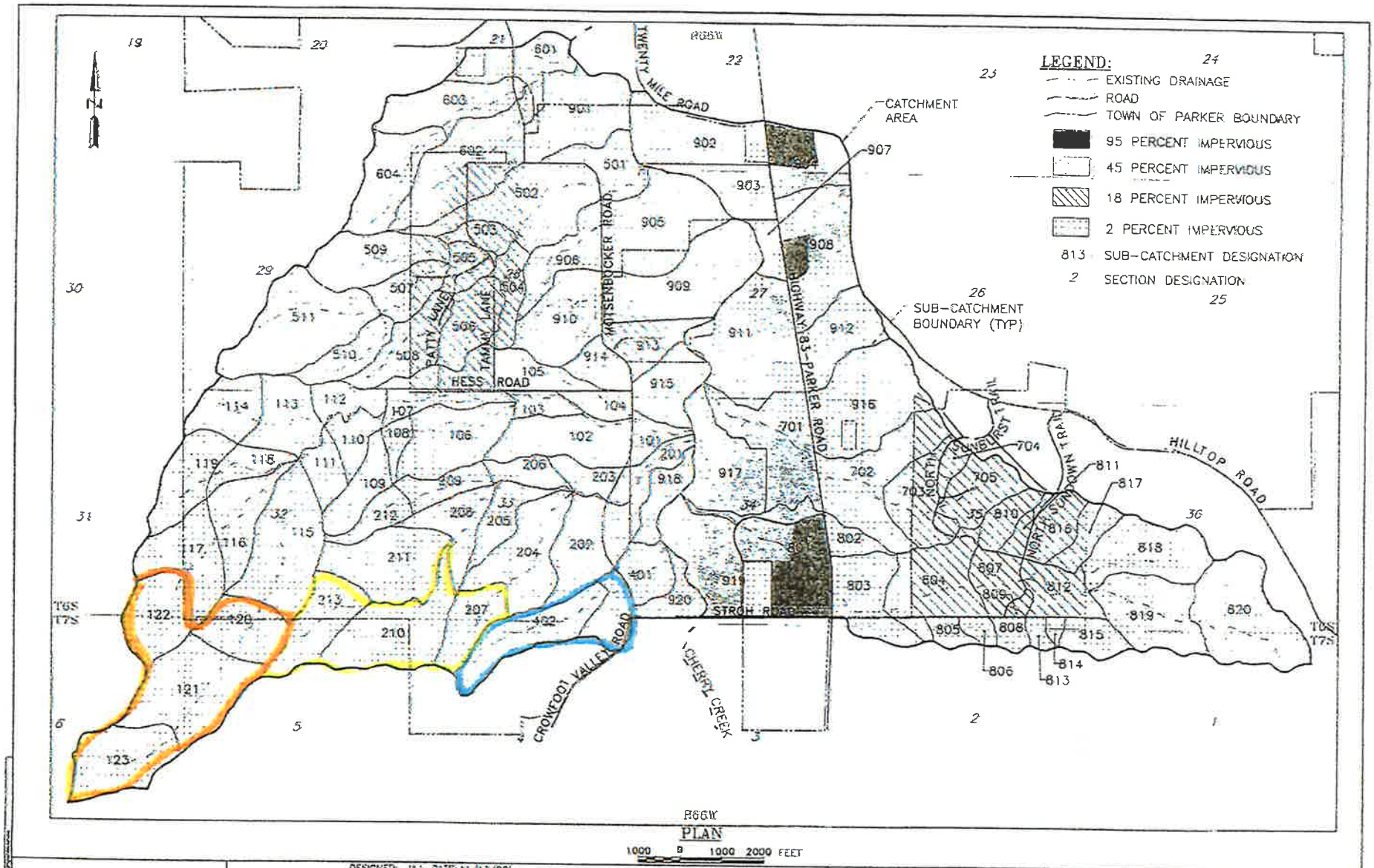
DESIGNED: JRJ DATE: 11/2/00
DRAWN: JRJ DATE: 11/12/02
CHECKED: JAJ DATE: 11/12/02
REVISED: DJA DATE: 2/27/03

TOWN OF PARKER & DOUGLAS COUNTY
URBAN DRAINAGE & FLOOD CONTROL DISTRICT

OAK GULCH AND STROH RANCH
OUTFALL SYSTEMS PLANNING

HYDROLOGIC SOILS GROUP

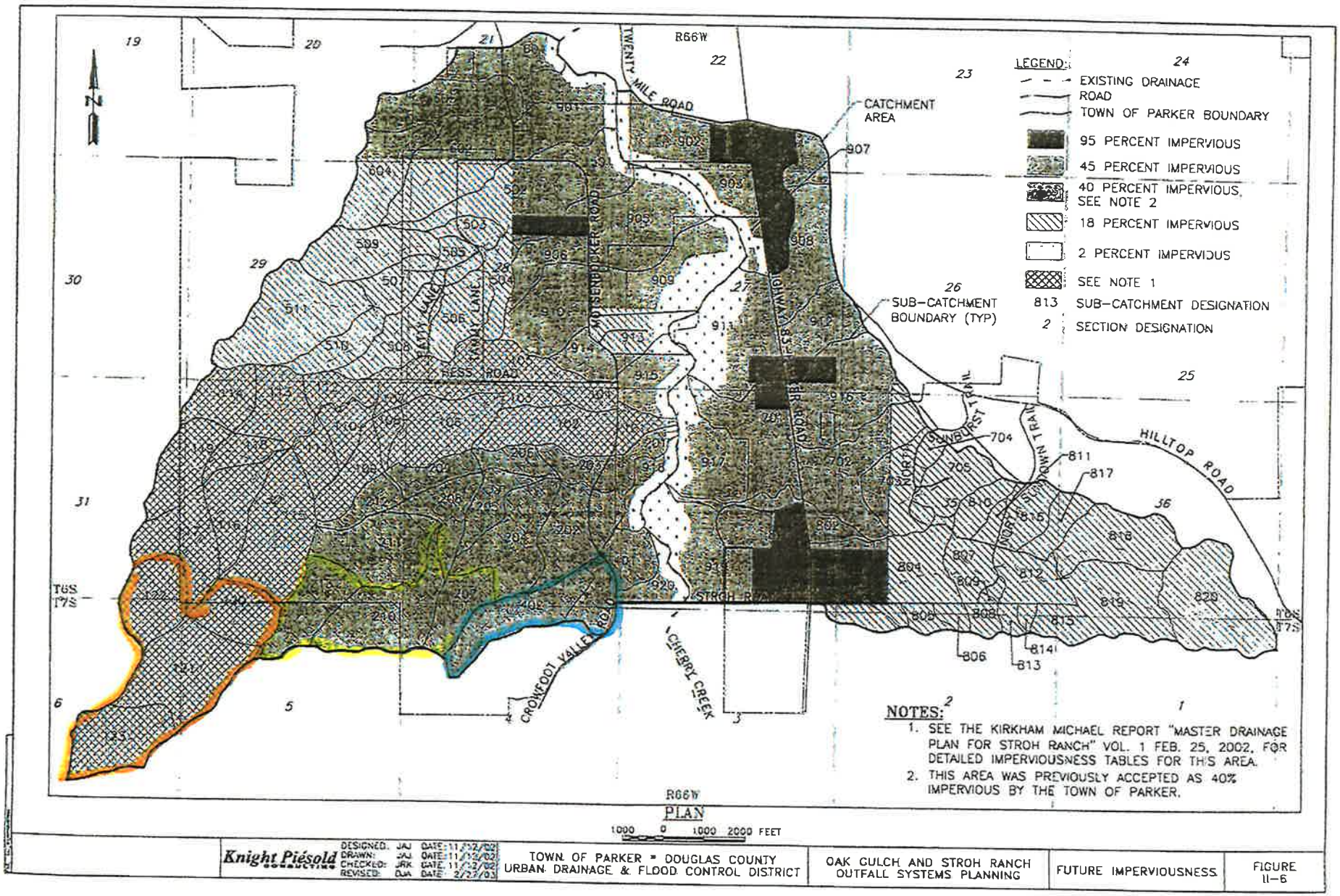
FIGURE
II-4



PLAN

1000 0 1000 2000 FEET

Knight Piésold CONSULTING	DESIGNED: JAJ DATE: 11/12/02 DRAWN: JAJ DATE: 11/12/02 CHECKED: JRS DATE: 11/12/02 REVISED: DJA DATE: 2/27/03	TOWN OF PARKER * DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	EXISTING IMPERVIOUSNESS	FIGURE II-5



NOTES:

1. SEE THE KIRKHAM MICHAEL REPORT "MASTER DRAINAGE PLAN FOR STROH RANCH" VOL. 1 FEB. 25, 2002, FOR DETAILED IMPERVIOUSNESS TABLES FOR THIS AREA.
2. THIS AREA WAS PREVIOUSLY ACCEPTED AS 40% IMPERVIOUS BY THE TOWN OF PARKER.

PLAN

1000 0 1000 2000 FEET

Knight Piésold CONSULTING	TOWN OF PARKER • DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	FUTURE IMPERVIOUSNESS	FIGURE II-6

DESIGNED: JAJ DATE: 11/12/02
DRAWN: JAJ DATE: 11/13/02
CHECKED: JRK DATE: 11/12/02
REVISED: DJA DATE: 2/27/03

TABLE III-1

CUHP SUB-CATCHMENT PARAMETERS
EXISTING IMPERVIOUS CONDITIONS

Catchment ID	Area (acres)	Area (mi^2)	Length (mi)	Centroid Length (mi)	Percent Imperv (%)	Slope (ft/ft)	t_c (min)	Pervious Depression Storage (in)	Impervious Depression Storage (in)	Initial Rate (in/hr)	Decay Coef. (1/sec)	Final Rate (in/hr)
[illegible] (parameters from Stroh Master Plan, see Reference 43, except existing imperviousness)												
101	16.1	0.0252	NOTE 1	NOTE 1	2.0	NOTE 1	18.70	0.35	0.05	4.06	0.0018	0.57
102	72.4	0.1131	NOTE 1	NOTE 1	2.0	NOTE 1	31.70	0.35	0.05	4.26	0.0018	0.56
103	19.0	0.0297	NOTE 1	NOTE 1	2.0	NOTE 1	23.00	0.35	0.05	3.41	0.0018	0.53
104	17.2	0.0269	NOTE 1	NOTE 1	2.0	NOTE 1	16.50	0.35	0.05	4.05	0.0018	0.57
105	73.7	0.1152	NOTE 1	NOTE 1	17.8	NOTE 1	31.80	0.35	0.05	4.05	0.0018	0.57
106	72.2	0.1128	NOTE 1	NOTE 1	2.0	NOTE 1	32.30	0.35	0.05	3.26	0.0018	0.52
107	34.3	0.0536	NOTE 1	NOTE 1	2.0	NOTE 1	35.20	0.35	0.05	3.36	0.0018	0.52
108	13.0	0.0203	NOTE 1	NOTE 1	2.0	NOTE 1	18.00	0.35	0.05	3.60	0.0018	0.54
109	37.6	0.0588	NOTE 1	NOTE 1	2.0	NOTE 1	24.20	0.35	0.05	3.60	0.0018	0.54
110	28.0	0.0438	NOTE 1	NOTE 1	2.0	NOTE 1	21.60	0.35	0.05	3.03	0.0018	0.50
111	26.0	0.0406	NOTE 1	NOTE 1	2.0	NOTE 1	21.40	0.35	0.05	3.03	0.0018	0.50
112	24.6	0.0384	NOTE 1	NOTE 1	2.0	NOTE 1	25.60	0.35	0.05	3.15	0.0018	0.51
113	57.3	0.0895	NOTE 1	NOTE 1	2.0	NOTE 1	21.40	0.35	0.05	3.15	0.0018	0.51
114	42.7	0.0667	NOTE 1	NOTE 1	2.0	NOTE 1	20.40	0.35	0.05	3.15	0.0018	0.51
115	108.5	0.1695	0.650	0.350	2.0	0.024	33.90	0.35	0.05	3.64	0.0018	0.54
116	58.6	0.0916	NOTE 1	NOTE 1	2.0	NOTE 1	34.20	0.35	0.05	3.39	0.0018	0.53
117	77.7	0.1214	NOTE 1	NOTE 1	2.0	NOTE 1	23.70	0.35	0.05	3.39	0.0018	0.53
118	21.9	0.0342	NOTE 1	NOTE 1	2.0	NOTE 1	21.30	0.35	0.05	3.00	0.0018	0.50
119	57.5	0.0898	NOTE 1	NOTE 1	2.0	NOTE 1	22.30	0.35	0.05	3.00	0.0018	0.50
[illegible]	56.3	0.0880	NOTE 1	NOTE 1	2.0	NOTE 1	20.80	0.35	0.05	4.22	0.0018	0.58
[illegible]	109.1	0.1705	0.69	0.28	2.0	0.029	28.30	0.35	0.05	3.96	0.0018	0.56
[illegible]	51.4	0.0803	NOTE 1	NOTE 1	2.0	NOTE 1	221.10	0.35	0.05	3.30	0.0018	0.52
[illegible]	68.4	0.1069	NOTE 1	NOTE 1	2.0	NOTE 1	51.70	0.35	0.05	3.54	0.0018	0.54
WEST STROH												
201	25.0	0.0390	0.626	0.304	2.0	0.017	112.16	0.40	0.05	3.86	0.0018	0.56
202	72.4	0.1131	0.623	0.284	2.0	0.030	75.33	0.40	0.05	3.99	0.0018	0.57
203	32.2	0.0503	0.424	0.202	2.0	0.022	46.65	0.40	0.05	3.99	0.0018	0.57
204	84.8	0.1325	0.740	0.352	2.0	0.028	92.40	0.40	0.05	3.86	0.0018	0.55
205	36.0	0.0562	0.693	0.384	2.0	0.029	88.25	0.40	0.05	3.66	0.0018	0.54
206	23.3	0.0364	0.426	0.202	2.0	0.015	73.34	0.40	0.05	3.89	0.0018	0.56
207	44.0	0.0687	0.472	0.227	2.0	0.048	56.46	0.40	0.05	3.67	0.0018	0.54
208	47.2	0.0738	0.514	0.241	2.0	0.040	61.46	0.40	0.05	4.31	0.0018	0.59
209	29.8	0.0466	0.472	0.237	2.0	0.042	54.49	0.40	0.05	3.99	0.0018	0.57
210	93.8	0.1466	0.920	0.460	2.0	0.032	N/A	0.40	0.05	4.11	0.0018	0.57
211	89.6	0.1400	0.663	0.349	2.0	0.044	75.16	0.40	0.05	3.72	0.0018	0.55
212	23.5	0.0367	0.380	0.146	2.0	0.046	47.88	0.40	0.05	3.69	0.0018	0.55
213	62.1	0.0971	0.586	0.252	2.0	0.039	67.39	0.40	0.05	4.23	0.0018	0.58

NOTE 1 This information was not supplied with the Kirkham Micheal Report. Runoff was calculated by CUHP using the rational method for these areas, so this information is not strictly necessary

TABLE III-1 (CONTINUED)

CUHP SUB-CATCHMENT PARAMETERS
EXISTING IMPERVIOUS CONDITIONS

Catchment ID	Area (acres)	Area (mi^2)	Length (mi)	Centroid Length (mi)	Percent Imperv. (%)	Slope (ft/ft)	t_c (min)	Pervious Depression Storage (in)	Impervious Depression Storage (in)	Initial Rate (in/hr)	Decay Coef. (1/sec)	Final Rate (in/hr)
NORTH PROWERS VALLEY												
[illegible]	59.5	0.0929	0.544	0.271	2.0	0.038	50.79	0.40	0.05	4.07	0.0018	0.57
[illegible]	72.9	0.1139	0.838	0.402	2.0	0.039	53.72	0.40	0.05	3.85	0.0018	0.56
CHERRY CREEK HIGHLANDS												
501	44.6	0.0696	0.426	0.123	2.0	0.032	56.14	0.40	0.05	3.53	0.0018	0.54
502	105.4	0.1646	0.778	0.309	8.4	0.028	N/A	0.40	0.05	3.15	0.0018	0.51
503	20.0	0.0312	0.337	0.134	18.0	0.040	41.61	0.40	0.05	3.15	0.0018	0.51
504	30.4	0.0475	0.477	0.248	18.0	0.032	61.73	0.40	0.05	3.45	0.0018	0.53
505	23.9	0.0374	0.284	0.136	18.0	0.050	33.18	0.40	0.05	4.05	0.0018	0.57
506	48.9	0.0780	0.466	0.239	18.0	0.039	57.36	0.40	0.05	3.60	0.0018	0.54
507	37.4	0.0584	0.398	0.206	10.0	0.044	48.85	0.40	0.05	3.90	0.0018	0.56
508	56.1	0.0877	0.581	0.252	10.0	0.036	68.58	0.40	0.05	4.05	0.0018	0.57
509	71.3	0.1114	0.855	0.383	6.8	0.061	59.70	0.40	0.05	3.00	0.0018	0.50
510	37.4	0.0585	0.536	0.258	2.0	0.048	57.53	0.40	0.05	4.20	0.0018	0.58
511	124.4	0.1944	0.803	0.339	2.0	0.044	N/A	0.40	0.05	3.15	0.0018	0.51
OLDE TOWN												
601	57.5	0.0899	0.555	0.208	2.0	0.040	69.91	0.40	0.05	3.60	0.0018	0.54
602	113.9	0.1779	0.869	0.388	6.8	0.028	N/A	0.40	0.05	3.08	0.0018	0.51
603	66.5	0.1039	0.689	0.341	2.0	0.025	93.45	0.40	0.05	3.15	0.0018	0.51
604	68.2	0.1066	0.606	0.290	2.0	0.041	67.94	0.40	0.05	3.00	0.0018	0.50
ROBINSON RANCH												
701	95.0	0.1485	0.754	0.458	36.4	0.021	N/A	0.36	0.05	4.30	0.00158	0.66
702	102.3	0.1599	0.672	0.259	5.2	0.054	N/A	0.40	0.05	4.13	0.0018	0.58
703	59.6	0.0931	0.443	0.150	13.2	0.071	43.26	0.40	0.05	4.35	0.0018	0.59
704	10.1	0.0157	0.163	0.078	18.0	0.056	24.39	0.40	0.05	4.50	0.0018	0.60

TABLE III-2

CUHP SUB-CATCHMENT PARAMETERS
FUTURE IMPERVIOUS CONDITIONS

Catchment ID	Area (acres)	Area (mi^2)	Length (mi)	Centroid Length (mi)	Percent Imperv. (%)	Slope (ft/ft)	t_c (min)	Pervious Depression Storage (in)	Impervious Depression Storage (in)	Initial Rate (in/hr)	Decay Coef. (1/sec)	Final Rate (in/hr)
[redacted] (parameters from Stroh Master Plan, see Reference 43)												
101	16.1	0.0252	NOTE 1	Note 1	21.0	NOTE 1	18.70	0.35	0.05	4.06	0.0018	0.57
102	72.4	0.1131	NOTE 1	Note 1	20.7	NOTE 1	31.70	0.35	0.05	4.26	0.0018	0.58
103	19.0	0.0297	NOTE 1	Note 1	2.0	NOTE 1	23.00	0.35	0.05	3.41	0.0018	0.53
104	17.2	0.0269	NOTE 1	Note 1	55.0	NOTE 1	16.50	0.35	0.05	4.05	0.0018	0.57
105	73.7	0.1152	NOTE 1	Note 1	29.2	NOTE 1	31.80	0.35	0.05	4.05	0.0018	0.57
106	72.2	0.1128	NOTE 1	Note 1	24.5	NOTE 1	32.30	0.35	0.05	3.25	0.0018	0.52
107	34.3	0.0536	NOTE 1	Note 1	9.1	NOTE 1	35.20	0.35	0.05	3.36	0.0018	0.52
108	13.0	0.0203	NOTE 1	Note 1	20.6	NOTE 1	16.00	0.35	0.05	3.60	0.0018	0.54
109	37.6	0.0588	NOTE 1	Note 1	24.2	NOTE 1	24.20	0.35	0.05	3.60	0.0018	0.54
110	28.0	0.0438	NOTE 1	Note 1	26.3	NOTE 1	21.60	0.35	0.05	3.03	0.0018	0.50
111	26.0	0.0406	NOTE 1	Note 1	27.0	NOTE 1	21.40	0.35	0.05	3.03	0.0018	0.50
112	24.6	0.0384	NOTE 1	Note 1	30.0	NOTE 1	25.60	0.35	0.05	3.15	0.0018	0.51
113	57.3	0.0895	NOTE 1	Note 1	40.7	NOTE 1	21.40	0.35	0.05	3.15	0.0018	0.51
114	42.7	0.0667	NOTE 1	Note 1	30.0	NOTE 1	20.40	0.35	0.05	3.15	0.0018	0.51
115	198.5	0.1695	0.650	0.350	27.3	0.024	33.90	0.35	0.05	3.64	0.0018	0.54
116	58.6	0.0916	NOTE 1	Note 1	19.3	NOTE 1	34.20	0.35	0.05	3.39	0.0018	0.53
117	77.7	0.1214	NOTE 1	Note 1	24.8	NOTE 1	23.70	0.35	0.05	3.39	0.0018	0.53
118	21.9	0.0342	NOTE 1	Note 1	32.7	NOTE 1	21.30	0.35	0.05	3.00	0.0018	0.50
119	57.5	0.0898	NOTE 1	Note 1	24.9	NOTE 1	22.30	0.35	0.05	3.00	0.0018	0.50
[redacted]	56.3	0.0880	NOTE 1	Note 1	32.	NOTE 1	20.80	0.35	0.05	4.22	0.0018	0.58
[redacted]	109.1	0.1705	0.69	0.28	24.5	0.029	26.30	0.35	0.05	3.96	0.0018	0.56
[redacted]	51.4	0.0803	NOTE 1	Note 1	21.5	NOTE 1	221.10	0.35	0.05	3.30	0.0018	0.52
[redacted]	68.4	0.1069	NOTE 1	Note 1	19.5	NOTE 1	51.70	0.35	0.05	3.45	0.0018	0.54
WEST STROH												
201	25.0	0.0390	0.626	0.304	40.0*	0.017	28.40	0.35	0.05	3.85	0.0018	0.56
202	72.4	0.1131	0.623	0.284	40.0	0.030	28.30	0.35	0.05	3.99	0.0018	0.57
203	32.2	0.0503	0.424	0.202	40.0	0.022	22.40	0.35	0.05	3.99	0.0018	0.57
204	84.8	0.1325	0.740	0.362	40.0	0.028	31.70	0.35	0.05	3.86	0.0018	0.56
205	36.0	0.0562	0.693	0.384	40.0	0.029	30.30	0.35	0.05	3.66	0.0018	0.54
206	23.3	0.0364	0.426	0.202	40.0	0.015	22.50	0.35	0.05	3.89	0.0018	0.56
207	44.0	0.0687	0.472	0.227	40.0	0.048	23.80	0.35	0.05	3.67	0.0018	0.54
208	47.2	0.0738	0.514	0.241	40.0	0.040	25.10	0.35	0.05	4.31	0.0018	0.59
209	29.8	0.0466	0.472	0.237	40.0	0.042	23.90	0.35	0.05	3.99	0.0018	0.57
210	93.8	0.1466	0.920	0.460	40.0	0.032	N/A	0.35	0.05	4.11	0.0018	0.57
211	89.6	0.1400	0.663	0.345	40.0	0.044	29.40	0.35	0.05	3.72	0.0018	0.55
212	23.5	0.0367	0.380	0.148	40.0	0.046	21.20	0.35	0.05	3.69	0.0018	0.55
213	62.1	0.0971	0.586	0.252	40.0	0.039	27.20	0.35	0.05	4.23	0.0018	0.58

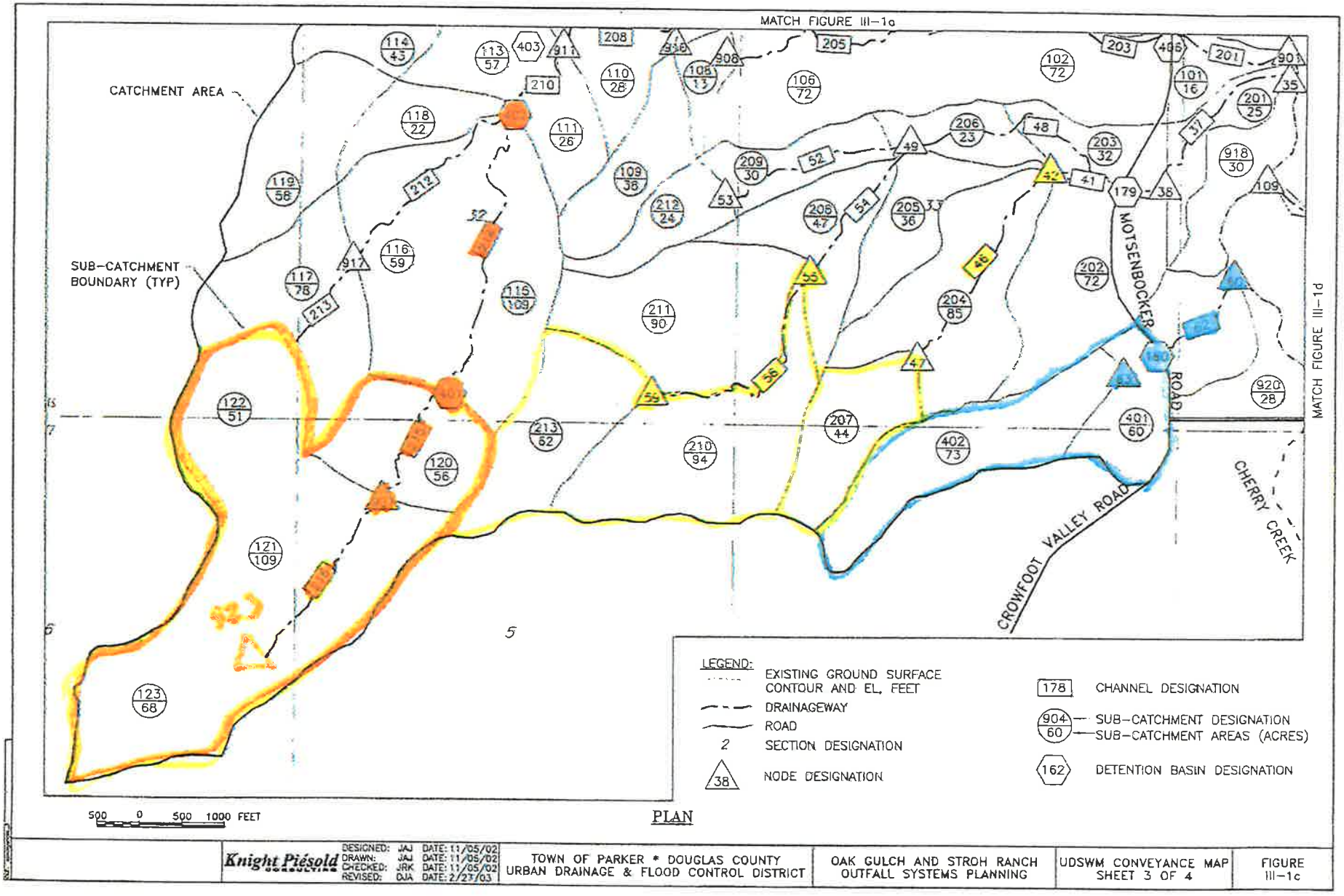
NOTE 1 This information was not supplied with the Kirkham Micheal Report. Runoff was calculated by CUHP using the rational method for these areas, so this information is not strictly necessary.

TABLE III-2 (CONTINUED)

CUHP SUB-CATCHMENT PARAMETERS
FUTURE IMPERVIOUS CONDITIONS

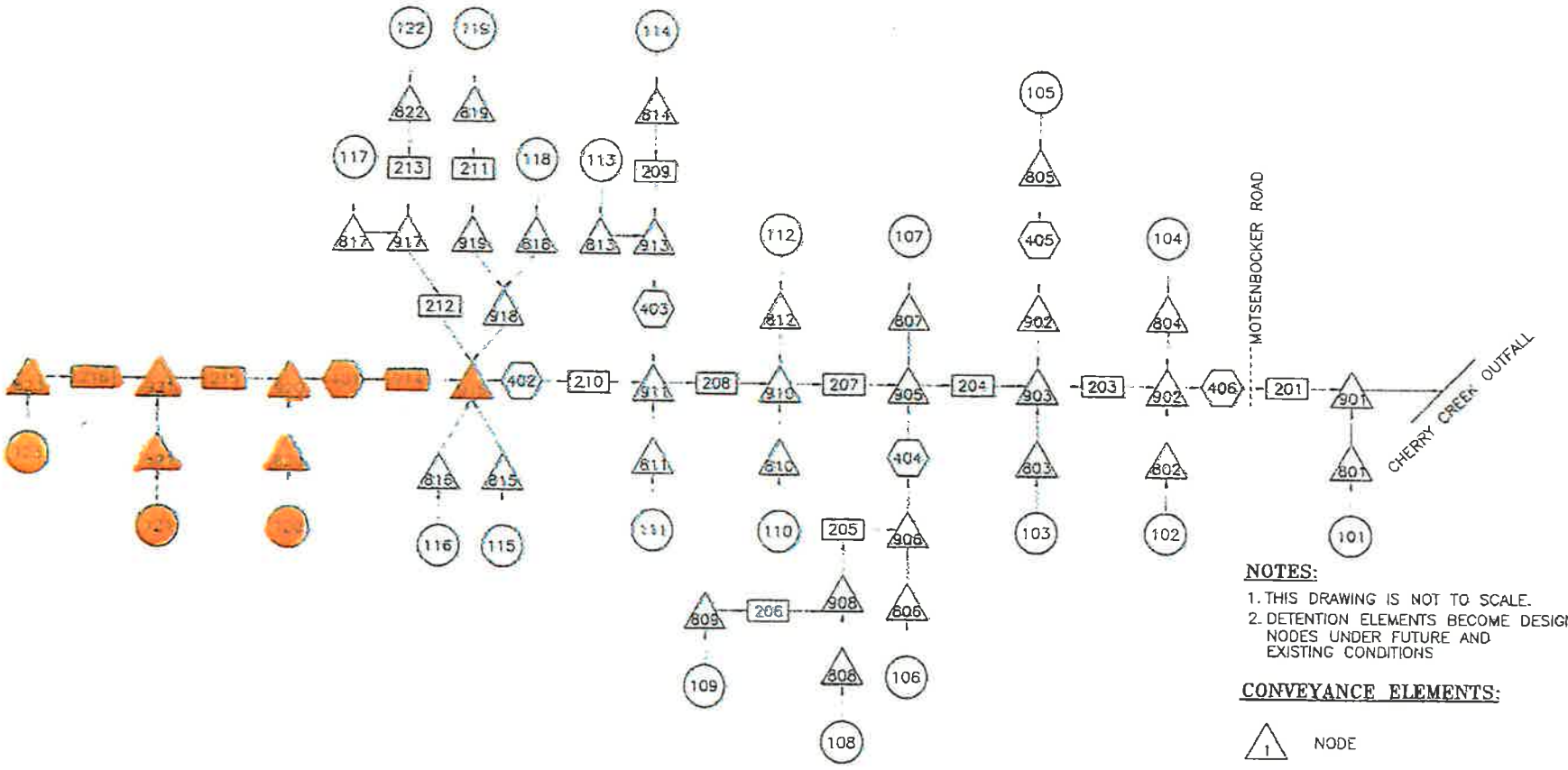
Catchment ID	Area (acres)	Area (mi^2)	Length (mi)	Centroid Length (mi)	Percent Imperv. (%)	Slope (ft/ft)	t_c (min)	Pervious Depression Storage (in)	Impervious Depression Storage (in)	Initial Rate (in/hr)	Decay Coef. (1/sec)	Final Rate (in/hr)
[redacted]												
[redacted]	59.5	0.0929	0.544	0.271	40.0*	0.038	25.90	0.35	0.05	4.07	0.0018	0.57
[redacted]	72.9	0.1139	0.838	0.402	40.0	0.039	34.60	0.35	0.05	3.85	0.0018	0.56
CHERRY CREEK HIGHLANDS												
501	44.6	0.0696	0.426	0.123	36.4	0.032	22.50	0.36	0.05	3.53	0.0018	0.54
502	105.4	0.1646	0.778	0.309	44.2	0.028	N/A	0.37	0.06	3.15	0.0018	0.51
503	20.0	0.0312	0.337	0.134	18.0	0.040	28.83	0.40	0.05	3.15	0.0018	0.51
504	30.4	0.0475	0.477	0.248	18.0	0.032	39.27	0.40	0.05	3.45	0.0018	0.53
505	23.9	0.0374	0.284	0.136	18.0	0.050	24.27	0.40	0.05	4.05	0.0018	0.57
506	49.9	0.0780	0.466	0.239	18.0	0.039	37.47	0.40	0.05	3.60	0.0018	0.54
507	37.4	0.0584	0.398	0.206	18.0	0.044	32.60	0.40	0.05	3.90	0.0018	0.56
508	56.1	0.0877	0.581	0.252	18.0	0.036	40.57	0.40	0.06	4.05	0.0018	0.57
509	71.3	0.1114	0.655	0.383	18.0	0.061	34.72	0.40	0.05	3.00	0.0018	0.50
510	37.4	0.0585	0.535	0.258	18.0	0.048	34.36	0.40	0.05	4.20	0.0018	0.58
511	124.4	0.1944	0.803	0.339	18.0	0.044	N/A	0.40	0.05	3.15	0.0018	0.51
OLDE TOWN												
601	57.5	0.0899	0.555	0.208	36.4	0.040	26.28	0.36	0.05	3.60	0.0018	0.54
602	113.9	0.1779	0.869	0.368	31.5	0.028	N/A	0.38	0.05	3.08	0.0018	0.51
603	66.5	0.1039	0.689	0.341	45.0	0.025	30.22	0.35	0.05	3.15	0.0018	0.51
604	68.2	0.1066	0.636	0.290	28.8	0.041	35.61	0.38	0.05	3.00	0.0018	0.50
ROBINSON RANCH												
701	95.0	0.1485	0.754	0.458	45.7	0.021	N/A	0.36	0.06	4.30	0.00158	0.66
702	102.3	0.1599	0.672	0.259	39.6	0.054	N/A	0.36	0.05	4.13	0.0018	0.58
703	59.6	0.0931	0.443	0.150	26.1	0.071	26.72	0.39	0.05	4.35	0.0018	0.59
704	10.1	0.0157	0.153	0.078	18.0	0.056	20.77	0.40	0.05	4.50	0.0018	0.60
705	47.8	0.0747	0.409	0.186	18.0	0.060	28.70	0.40	0.05	4.35	0.0018	0.59

* 40.0% imperviousness used for West Stroh and North Crowfoot Valley catchments per previously platted and accepted drainage designs



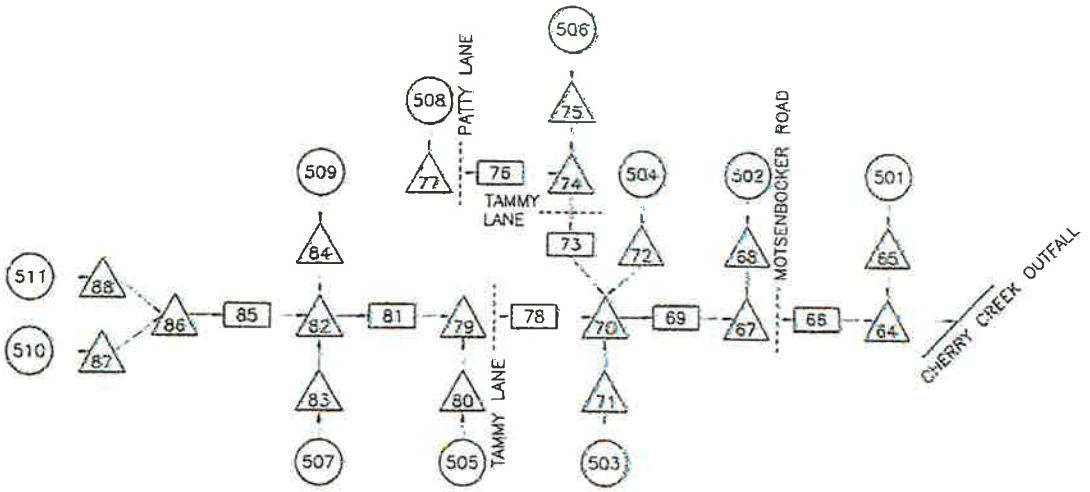
PLAN

Knight Piésold	DESIGNED: JAJ DATE: 11/05/02 DRAWN: JAJ DATE: 11/05/02 CHECKED: JRK DATE: 11/05/02 REVISED: DJA DATE: 2/27/03	TOWN OF PARKER * DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	UDSWM CONVEYANCE MAP SHEET 3 OF 4	FIGURE III-1c

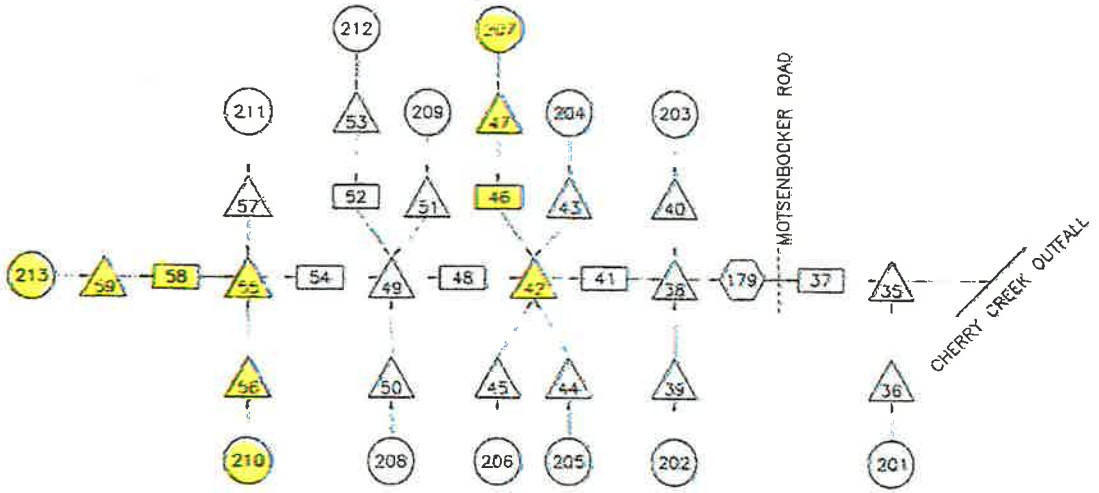


OAK GULCH

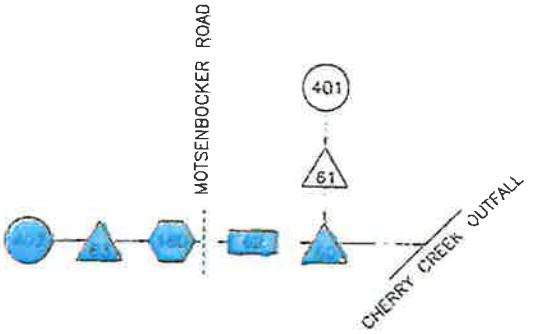
Knight Piésold CONSULTING DESIGNED: JAJ DATE: 11/12/02 DRAWN: JAJ DATE: 11/12/02 CHECKED: JRK DATE: 11/12/02 REVISED: DJA DATE: 2/27/03	TOWN OF PARKER * DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	CONNECTIVITY SHEET 1 OF 4	FIGURE III-2a



CHERRY CREEK HIGHLANDS



WEST STROH



NORTH CROWFOOT VALLEY

NOTES:

1. SUB-CATCHMENTS 901, 915, 918 AND 920 ARE DIRECT FLOW CATCHMENTS ADJACENT TO CHERRY CREEK. CONNECTIVITY DIAGRAMS AND UDSWM MODELING ARE NOT APPROPRIATE FOR THESE SUB-CATCHMENTS.
2. THIS DRAWING IS NOT TO SCALE.

CONVEYANCE ELEMENTS:

- 1 — NODE
- 107 — SUB-CATCHMENT
- 3 — CHANNEL
- 162 — DETENTION
- CHERRY CREEK OUTFALL
- ROAD CROSSING

Knight Piésold CONSULTING DESIGNED: JAJ DATE: 11/12/02 DRAWN: JAJ DATE: 11/12/02 CHECKED: JRK DATE: 11/12/02 REVISED: DJA DATE: 2/27/03	TOWN OF PARKER • DOUGLAS COUNTY URBAN DRAINAGE & FLOOD CONTROL DISTRICT	OAK GULCH AND STROH RANCH OUTFALL SYSTEMS PLANNING	CONNECTIVITY SHEET 2 OF 4	FIGURE III-2b

FIGURE III-3a

PEAK FLOW PROFILES
4616 OAK GULCH

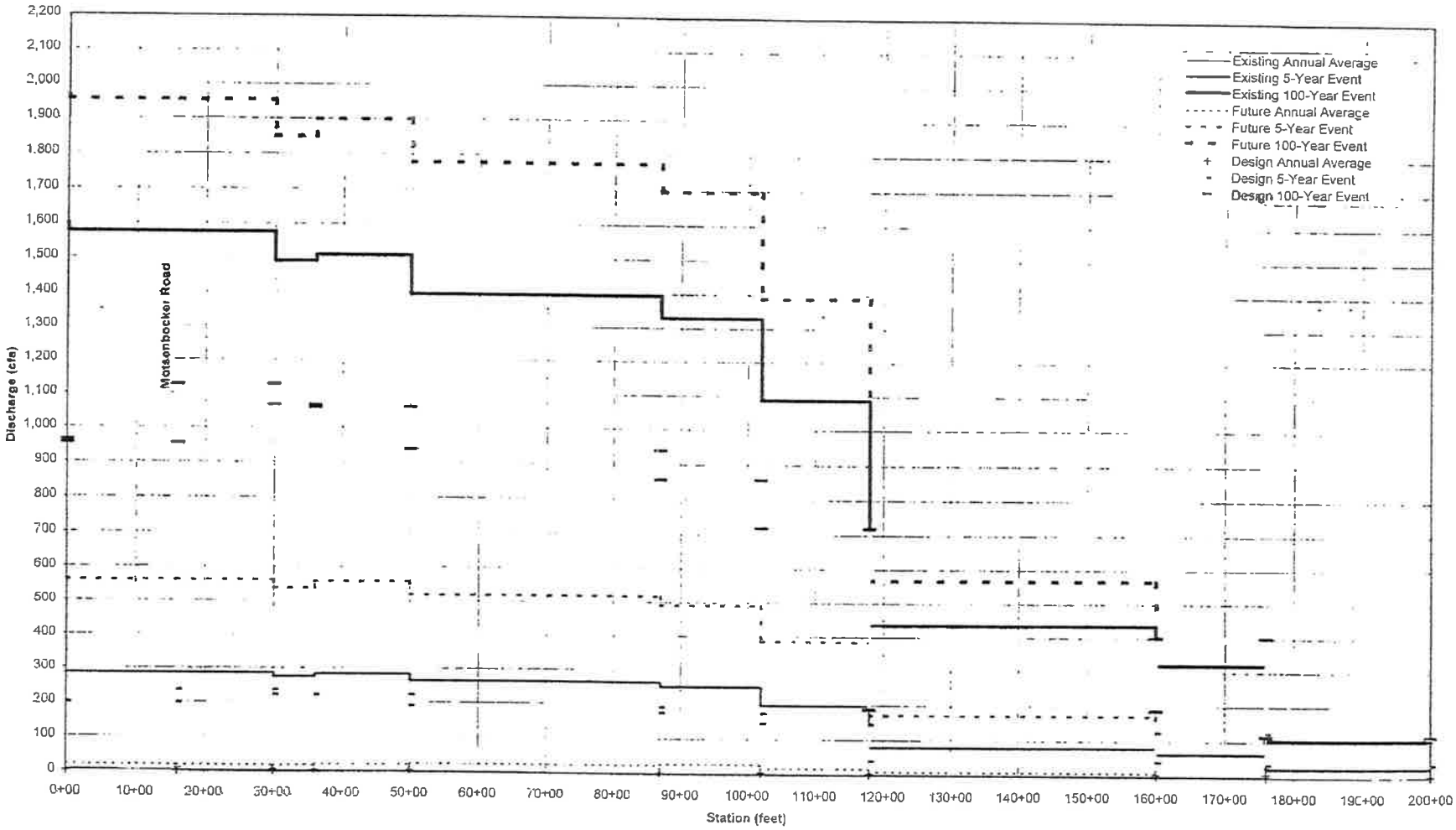


FIGURE III-3b

PEAK FLOW PROFILES
4616.1 WEST STROH

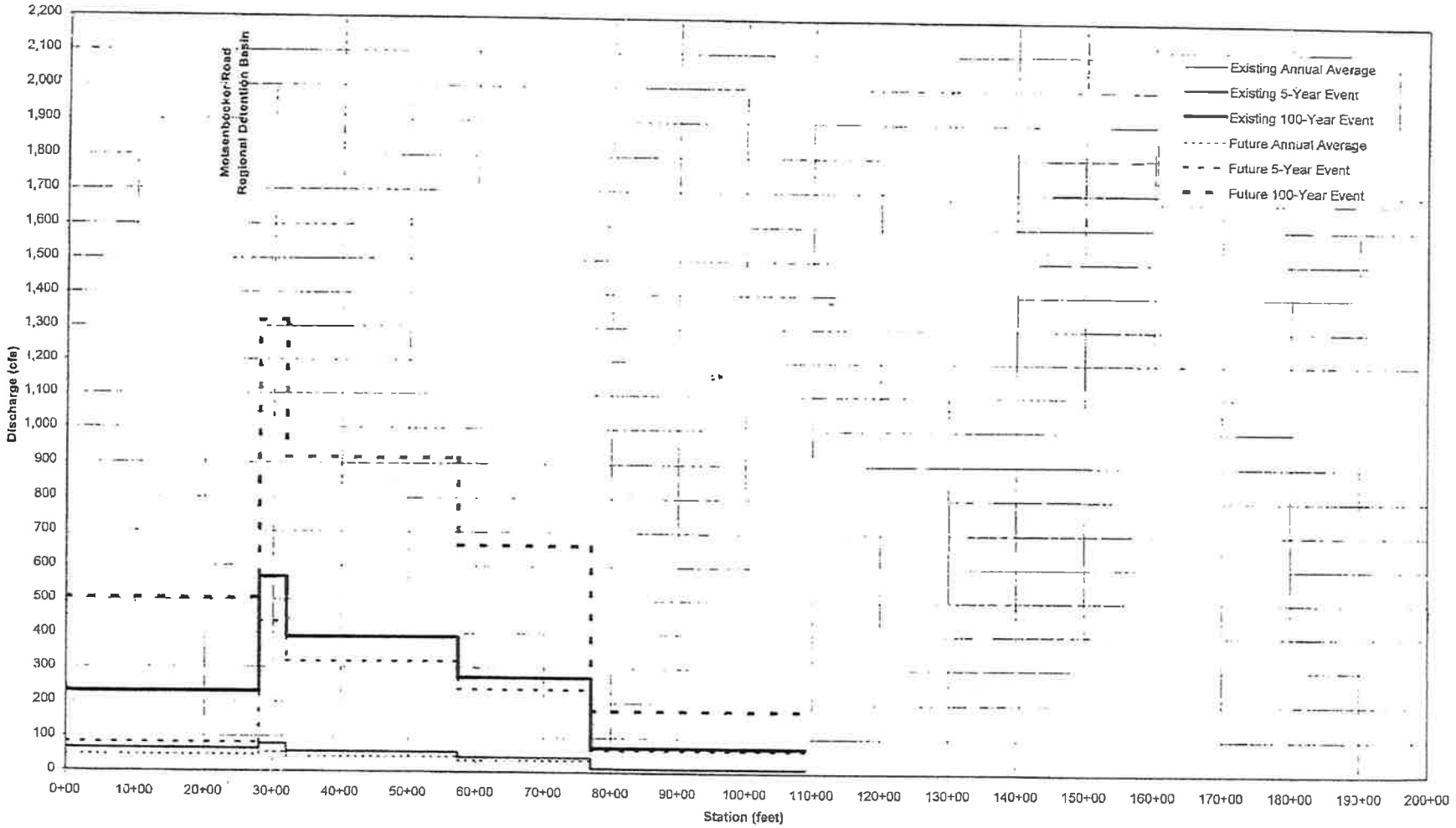


FIGURE III-3c

PEAK FLOW PROFILES
4600-11.3 NORTH CROWFOOT VALLEY

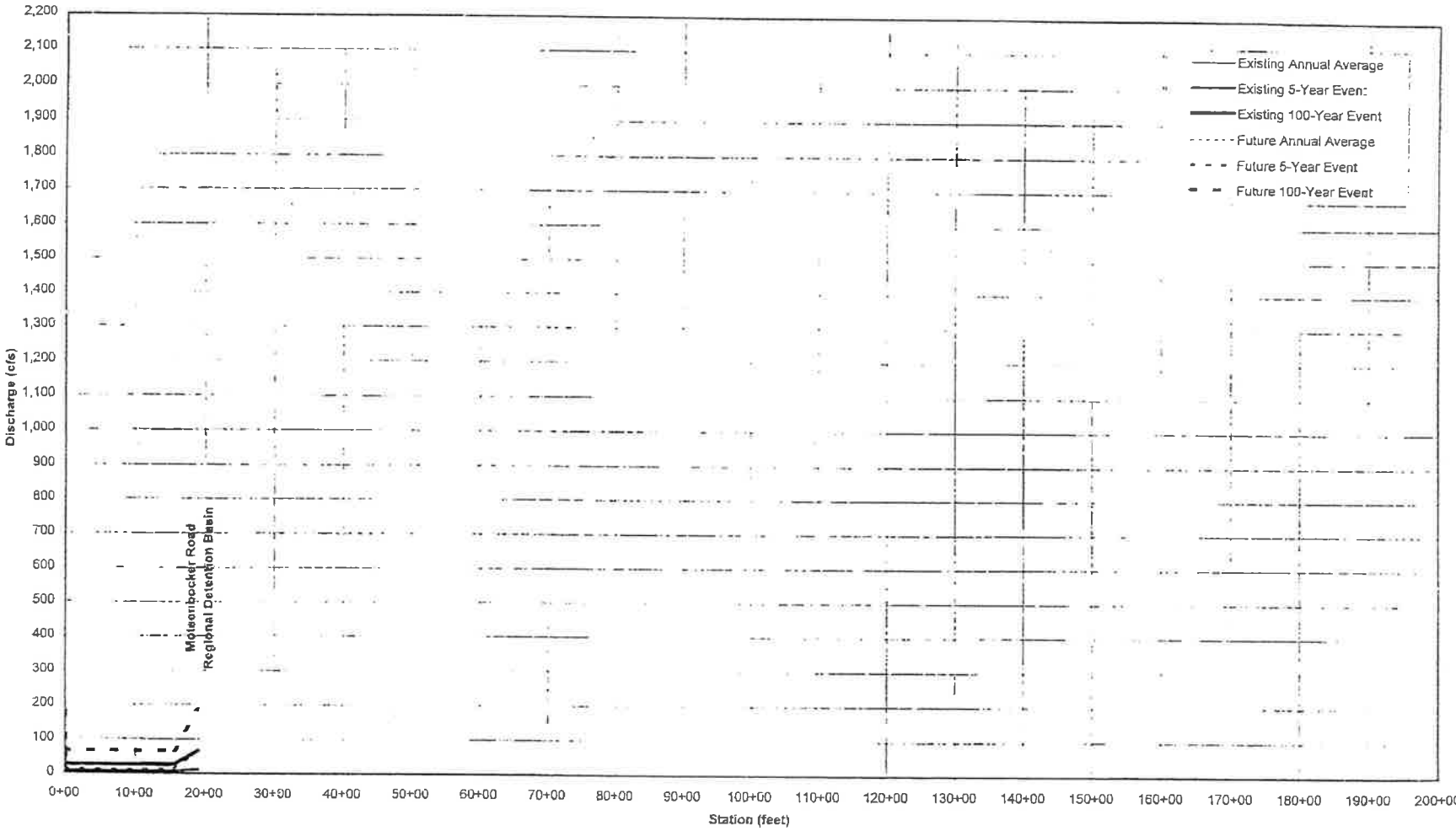


FIGURE III-4a

RUNOFF HYDROGRAPH DRAINAGEWAY 4616
UDSWM ELEMENT NO. 406

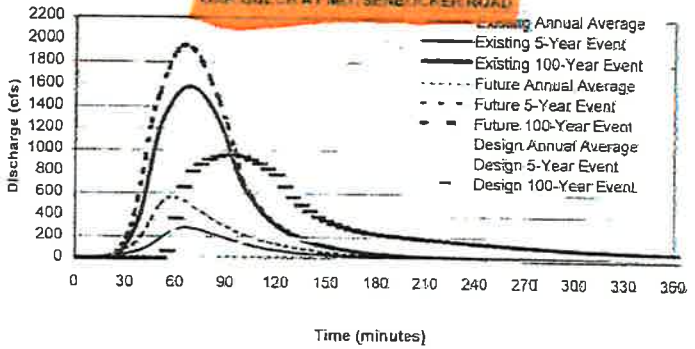


FIGURE III-4b

RUNOFF HYDROGRAPH DRAINAGEWAY 4616.1
UDSWM ELEMENT NO. 38
W. STROH AT INLET TO EXG. REG. DETENTION

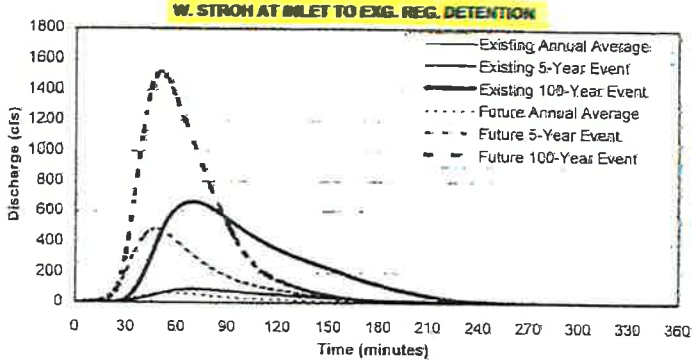


FIGURE III-4c

RUNOFF HYDROGRAPH DRAINAGEWAY 4600-11.3
UDSWM ELEMENT NO. 63
N. CROWFOOT VALLEY AT INLET TO EXG. REG. DETENTION

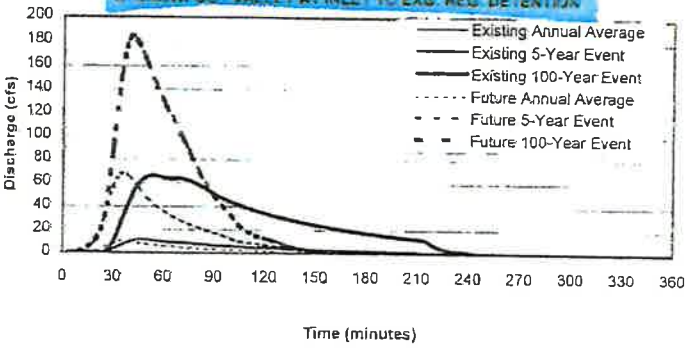
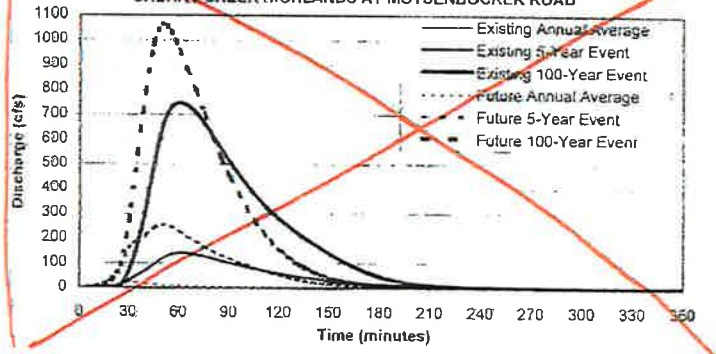


FIGURE III-4d

RUNOFF HYDROGRAPH DRAINAGEWAY 4600-10.4
UDSWM ELEMENT NO. 67
CHERRY CREEK HIGHLANDS AT MOTSENBOCKER ROAD

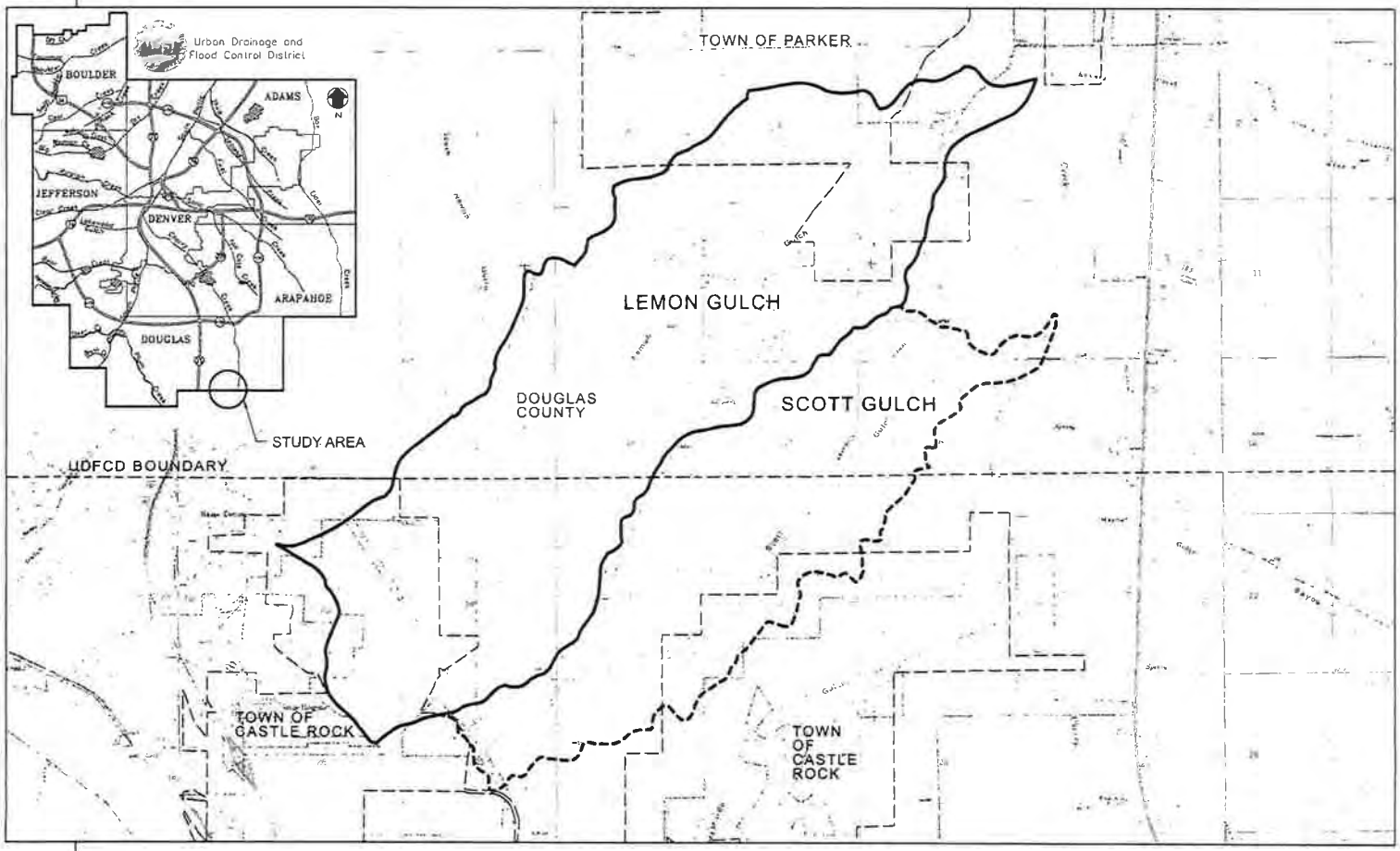


APPENDIX E

Excerpts from Scott & Lemon Gulch Watersheds Outfall Systems Planning

OUTFALL SYSTEMS PLANNING-PRELIMINARY DESIGN REPORT

SCOTT AND LEMON GULCH WATERSHEDS



PREPARED FOR:

URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

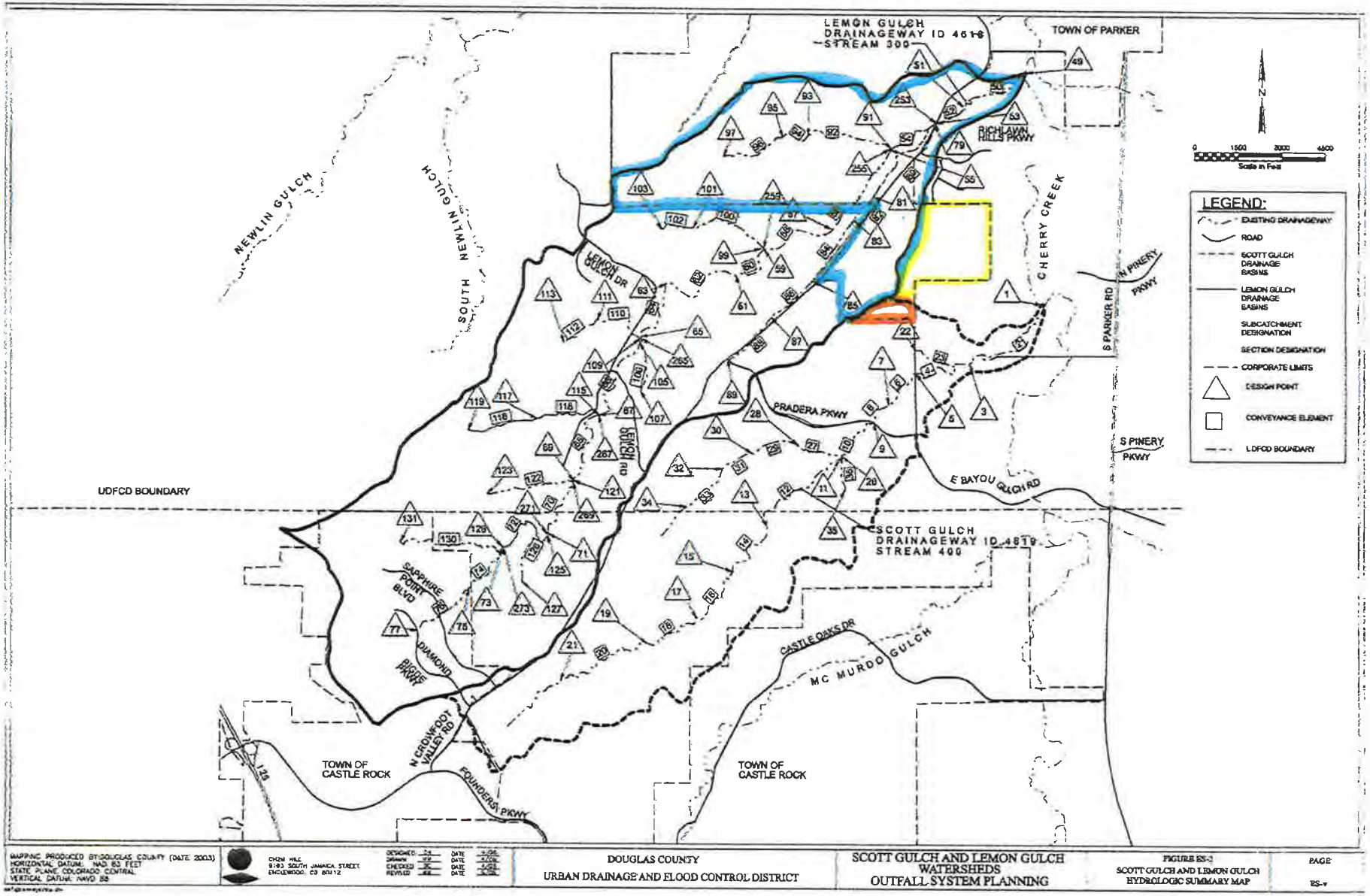
DOUGLAS COUNTY

PREPARED BY

CH2MHILL
DENVER, CO

JULY 2006

lgfigcv.dlv



MAPPING PRODUCED BY DOUGLAS COUNTY (DATE: 2003)
HORIZONTAL DATUM: NAD 83 FEET
STATE PLANE COLORADO CENTRAL
VERTICAL DATUM: NAVD 88

CH2M HILL
9193 SOUTH JAMAICA STREET
ENGLEWOOD, CO 80112

DESIGNED: JH DATE: 4/06
DRAWN: JH DATE: 4/06
CHECKED: BK DATE: 4/06
REVISED: BK DATE: 5/06

DOUGLAS COUNTY
URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

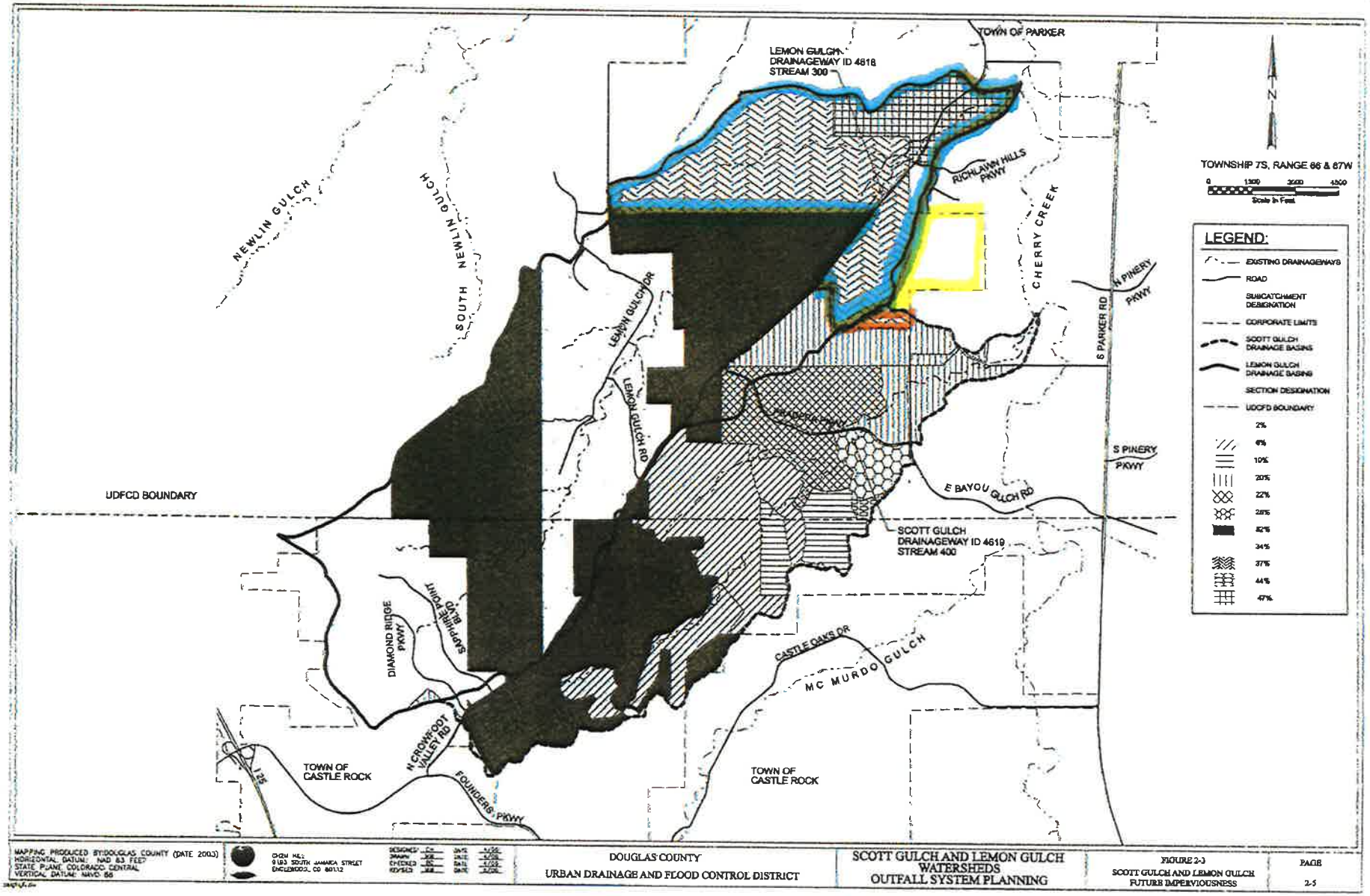
SCOTT GULCH AND LEMON GULCH WATERSHEDS
OUTFALL SYSTEM PLANNING

FIGURE ES-2
SCOTT GULCH AND LEMON GULCH HYDROLOGIC SUMMARY MAP

PAGE
ES-v

course, and a single family community averaging just over one dwelling unit per acre. Much of the infrastructure planned for Pradera development has been constructed including a regional detention facility and stream stabilization measures.

The Canyons development is planned for the upper portion of the watershed and is not yet under construction. Density for the development will consist of one home per acre. The lower portion of the watershed is zoned as a Community Separator Area and open space. The Douglas County Master Plan indicates that there is potential for development in the community separator area, however, the allowed density will be restricted.



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HORIZONTAL DATUM: NAD 83 FEET
STATE PLANE COLORADO CENTRAL
VERTICAL DATUM: NAVD 88

OMNI HKZ
9193 SOUTH JAMAICA STREET
ENGLEWOOD, CO 80112

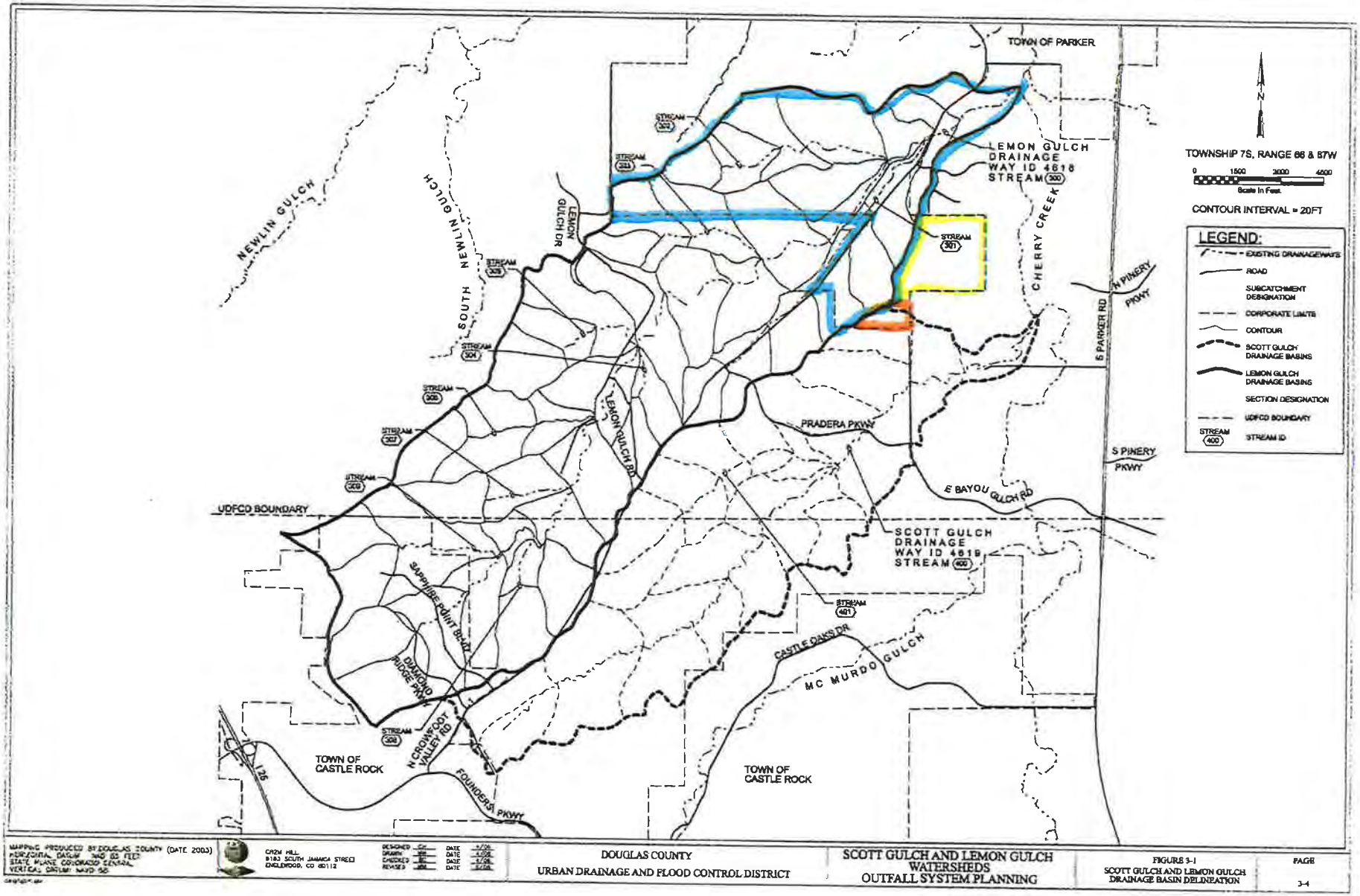
DESIGNED - CC — DATE - 4/05
DRAWN - KJE — DATE - 4/05
CHECKED - DC — DATE - 4/05
REVISED - KJE — DATE - 6/05

DOUGLAS COUNTY
URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

SCOTT GULCH AND LEMON GULCH WATERSHEDS
OUTFALL SYSTEM PLANNING

FIGURE 2-3
SCOTT GULCH AND LEMON GULCH
FUTURE IMPERVIOUSNESS

PAGE
2-5



MAPPING PRODUCED BY DOUGLAS COUNTY (DATE 2003)
HORIZONTAL DATUM: NAD 83 FEET
STATE PLANE COLORADO CENTRAL
VERTICAL DATUM: NAVD 88

CROM HILL
9183 SOUTH JAMAICA STREET
ENGLEWOOD, CO 80112

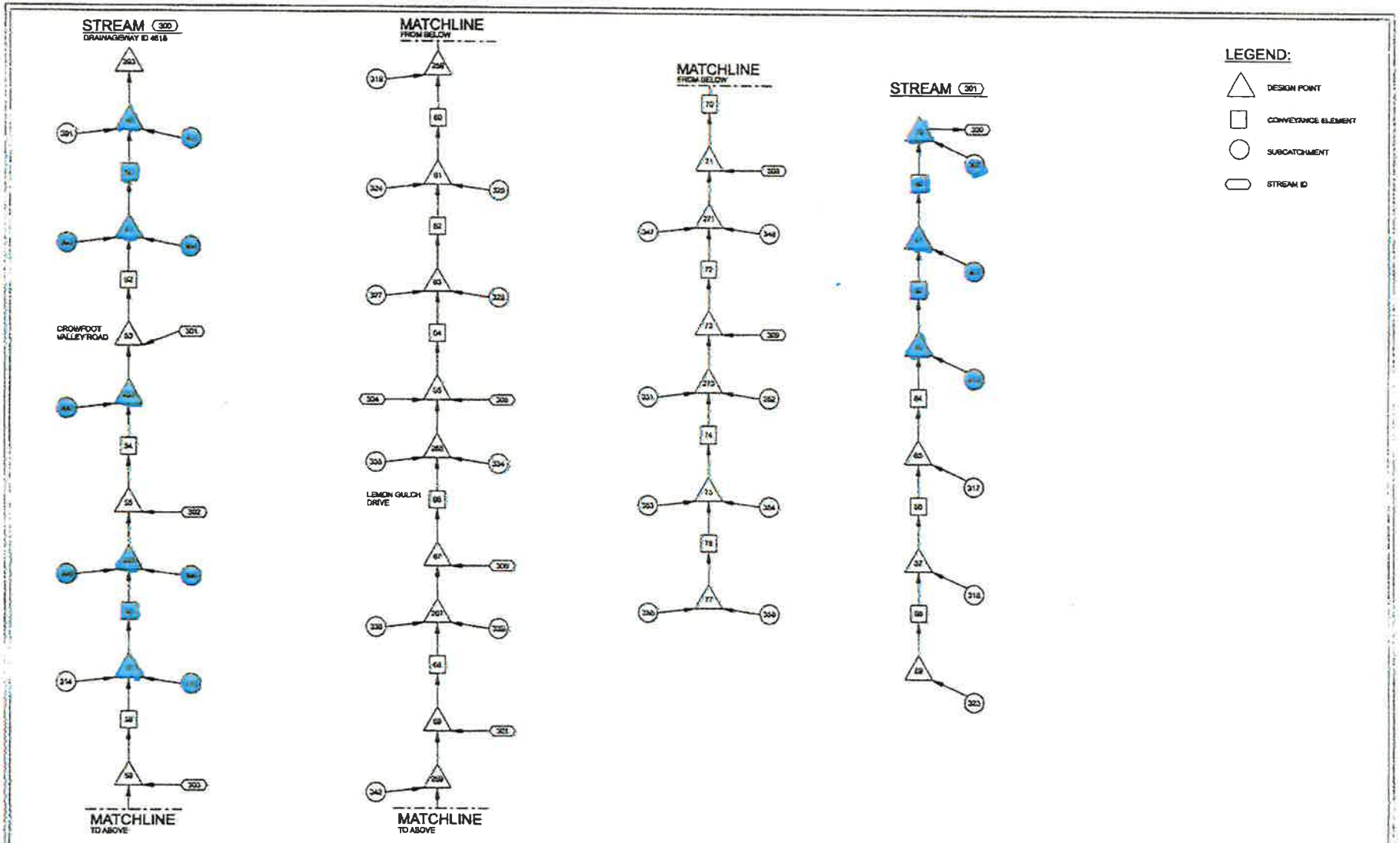
DESIGNED CH DATE 4/06
DRAWN WH DATE 4/06
CHECKED MC DATE 4/06
REVISED MH DATE 5/06

DOUGLAS COUNTY
URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

SCOTT GULCH AND LEMON GULCH WATERSHEDS
OUTFALL SYSTEM PLANNING

FIGURE 3-1
SCOTT GULCH AND LEMON GULCH DRAINAGE BASIN DELINEATION

PAGE
3-4



MAPPING PRODUCED BY DOUGLAS COUNTY (DATE 2003)
HORIZONTAL DATUM: NAD 83 FEET
STATE PLANE COLORADO CENTRAL
VERTICAL DATUM: NAVD 88

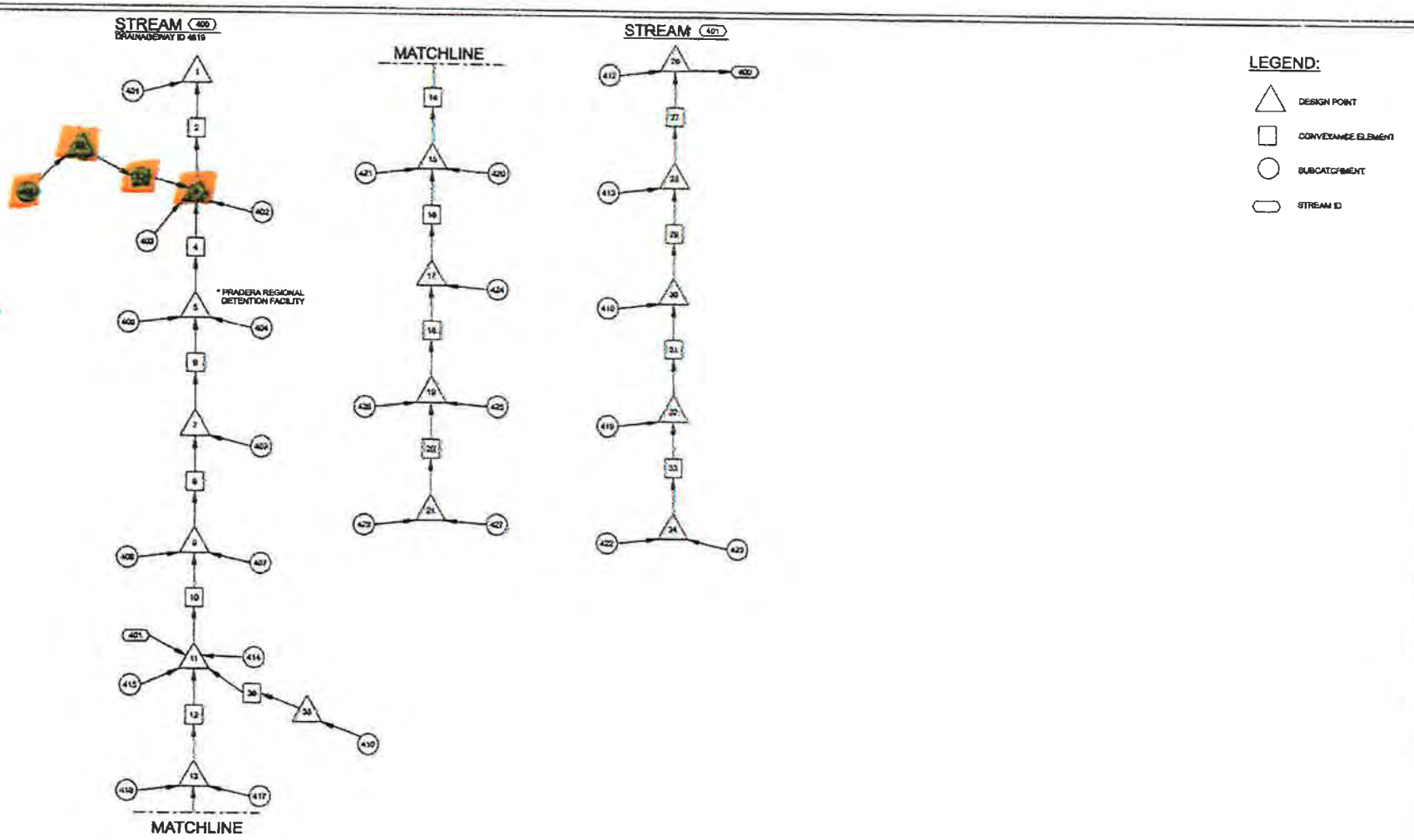
CH2M HILL
9 LIB S SOUTH JAMAICA STREET
ENGLEWOOD, CO 80111-2

DESIGNED CH
DRAWN MH
CHECKED MK
REVISED MK
DATE 6/05
DATE 6/05
DATE 6/05
DATE 6/05

DOUGLAS COUNTY
URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

SCOTT GULCH AND LEMON GULCH WATERSHEDS
OUTFALL SYSTEM PLANNING

FIGURE 3-3
LEMON GULCH
UDSWM MODEL CONNECTIVITY



MAPPING PRODUCED BY: DOUGLAS COUNTY (DATE 2003)
HORIZONTAL DATUM: NAD 83 FEET
STATE PLANE COLORADO CENTRAL
VERTICAL DATUM: NAVD 88

GKSH HILL
9193 SOUTH JAMAICA STREET
ENGLEWOOD, CO 80112

DESIGNED CH DATE 4/06
DRAWN MB DATE 4/06
CHECKED RC DATE 4/06
APV'D BLD MB DATE 6/06

DOUGLAS COUNTY
URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

SCOTT GULCH AND LEMON GULCH WATERSHEDS
OUTFALL SYSTEM PLANNING

FIGURE 3-3
SCOTT GULCH
UDSWM MODEL CONNECTIVITY

TABLE A-2

CUHP SUBWATERSHED CHARACTERISTICS
FUTURE IMPERVIOUS CONDITION

[1]Subcatchment ID	Area (acres)	Area (mi^2)	Length (mi)	Centroid Length (mi)	Percent Imperv. (%)	Slope (ft/ft)	t_c (min)	Pervious Depression Storage (in)	Impervious Depression Storage (in)	Initial Rate (in/hr)	Decay Coef. (1/sec)	Final Rate (in/hr)
LEMON GULCH (DRAINAGEWAY ID 4618)												
301	33	0.051	0.61	0.32	30.4%	0.015	27.8	0.35	0.05	4.41	0.0018	0.59
[redacted]	58	0.092	0.81	0.43	43.0%	0.035	29.0	0.35	0.05	4.30	0.0018	0.59
[redacted]	39	0.061	0.73	0.37	19.1%	0.031	70.3	0.40	0.05	4.31	0.0018	0.59
[redacted]	49	0.077	0.53	0.32	45.0%	0.049	18.3	0.35	0.05	4.29	0.0018	0.59
[redacted]	101	0.158	1.04	0.43	36.1%	0.020		0.35	0.05	4.03	0.0016	0.62
[redacted]	53	0.082	0.65	0.46	41.9%	0.025	28.9	0.35	0.05	4.39	0.0018	0.59
[redacted]	98	0.153	0.78	0.46	34.0%	0.033		0.35	0.05	3.88	0.0017	0.59
[redacted]	43	0.067	0.81	0.42	37.0%	0.016	33.8	0.35	0.05	4.58	0.0016	0.69
[redacted]	105	0.166	0.84	0.47	37.0%	0.031		0.35	0.05	3.62	0.0018	0.54
[redacted]	163	0.255	0.94	0.50	41.0%	0.038		0.35	0.05	4.34	0.0018	0.59
[redacted]	30	0.047	0.41	0.07	37.0%	0.045	22.0	0.35	0.05	3.51	0.0018	0.53
[redacted]	72	0.112	0.62	0.35	37.0%	0.040	21.9	0.35	0.05	4.42	0.0018	0.59
[redacted]	97	0.152	0.82	0.49	28.9%	0.029		0.35	0.05	3.90	0.0018	0.57
314	57	0.088	0.53	0.29	37.0%	0.012	25.5	0.35	0.05	4.29	0.0018	0.59
[redacted]	124	0.194	0.83	0.48	37.0%	0.026		0.35	0.05	3.96	0.0018	0.56
[redacted]	88	0.137	0.62	0.32	37.0%	0.036	28.5	0.35	0.05	4.38	0.0018	0.59
317	98	0.153	0.62	0.30	20.3%	0.044		0.35	0.05	3.95	0.0018	0.56
318	87	0.136	0.78	0.36	8.7%	0.037	68.5	0.40	0.05	3.15	0.0018	0.51
319	121	0.189	0.75	0.38	25.9%	0.031		0.35	0.05	4.06	0.0018	0.57
320	37	0.057	0.56	0.24	35.7%	0.032	26.5	0.35	0.05	3.09	0.0018	0.51
[redacted]	91	0.142	0.95	0.56	28.4%	0.039		0.35	0.05	4.32	0.0018	0.59
[redacted]	54	0.084	0.41	0.18	29.6%	0.049	22.0	0.35	0.05	3.88	0.0018	0.56
323	67	0.105	0.49	0.22	19.6%	0.046	45.7	0.40	0.05	3.00	0.0018	0.50
324	84	0.131	0.75	0.40	18.4%	0.049	58.8	0.40	0.05	3.81	0.0018	0.55
325	73	0.114	0.60	0.41	18.1%	0.030	62.3	0.40	0.05	3.99	0.0018	0.57
[redacted]	107	0.168	0.68	0.37	17.3%	0.043		0.40	0.05	3.48	0.0018	0.53
327	74	0.116	1.04	0.57	7.6%	0.016	124.4	0.40	0.05	3.09	0.0018	0.51
328	116	0.181	0.71	0.32	2.0%	0.050		0.40	0.05	3.51	0.0018	0.53
329	51	0.080	0.51	0.24	10.0%	0.051	45.0	0.40	0.05	3.00	0.0018	0.50
330	67	0.105	0.64	0.38	5.0%	0.044	55.1	0.40	0.05	3.00	0.0018	0.50
331	30	0.047	0.38	0.19	2.0%	0.056	36.1	0.40	0.05	3.64	0.0018	0.54
332	124	0.195	1.01	0.63	7.5%	0.031		0.40	0.05	3.08	0.0018	0.51
333	65	0.102	0.61	0.30	2.6%	0.046	53.1	0.40	0.05	3.00	0.0018	0.50
334	49	0.076	0.80	0.37	2.0%	0.014	107.0	0.40	0.05	3.23	0.0018	0.52
335	89	0.139	0.88	0.55	2.2%	0.027	86.9	0.40	0.05	3.40	0.0018	0.53
336	19	0.030	0.36	0.19	2.0%	0.041	40.0	0.40	0.05	3.00	0.0018	0.50
337	60	0.094	0.60	0.35	13.8%	0.043	53.7	0.40	0.05	3.00	0.0018	0.50
338	116	0.182	0.98	0.61	4.3%	0.021		0.40	0.05	3.03	0.0018	0.50

TABLE A-2 (CONTINUED)

CUHP SUBWATERSHED CHARACTERISTICS
FUTURE IMPERVIOUS CONDITION

[1]Subcatchment ID	Area (acres)	Area (mi^2)	Length (mi)	Centroid Length (mi)	Percent Imperv. (%)	Slope (ft/ft)	t_c (min)	Pervious Depression Storage (in)	Impervious Depression Storage (in)	Initial Rate (in/hr)	Decay Coef. (1/sec)	Final Rate (in/hr)
LEMON GULCH (CONTINUED)												
339	44	0.069	0.70	0.37	2.1%	0.022	79.9	0.40	0.05	3.04	0.0018	0.50
340	63	0.099	0.66	0.33	13.3%	0.030	66.9	0.40	0.05	3.01	0.0018	0.50
341	106	0.166	0.59	0.30	25.9%	0.043		0.35	0.05	3.00	0.0018	0.50
342	99	0.155	0.64	0.31	12.7%	0.039		0.40	0.05	3.00	0.0018	0.50
343	95	0.149	0.62	0.35	27.0%	0.035		0.35	0.05	3.00	0.0018	0.50
344	82	0.128	0.83	0.46	32.0%	0.030	34.3	0.35	0.05	3.00	0.0018	0.50
345	76	0.119	0.62	0.38	32.0%	0.036	23.7	0.35	0.05	3.00	0.0018	0.50
346	85	0.132	0.60	0.26	31.8%	0.041	27.7	0.35	0.05	3.00	0.0018	0.50
347	102	0.159	0.61	0.33	9.4%	0.038		0.40	0.05	3.00	0.0018	0.50
348	41	0.064	0.58	0.33	31.7%	0.048	20.7	0.35	0.05	3.00	0.0018	0.50
349	45	0.071	0.58	0.38	31.8%	0.040	22.9	0.35	0.05	3.00	0.0018	0.50
350	108	0.169	0.66	0.30	30.7%	0.033		0.35	0.05	3.00	0.0018	0.50
351	109	0.170	1.04	0.61	30.7%	0.028		0.35	0.05	3.00	0.0018	0.50
352	28	0.043	0.49	0.34	32.6%	0.039	18.6	0.35	0.05	3.00	0.0018	0.50
353	92	0.143	0.85	0.47	33.4%	0.036		0.35	0.05	3.00	0.0018	0.50
354	55	0.086	0.68	0.47	34.0%	0.039	32.2	0.35	0.05	3.00	0.0018	0.50
355	91	0.143	1.01	0.65	33.1%	0.027		0.35	0.05	3.00	0.0018	0.50
356	123	0.192	1.16	0.74	27.7%	0.025		0.35	0.05	3.00	0.0018	0.50
357	86	0.134	0.61	0.23	32.0%	0.040	28.0	0.35	0.05	3.00	0.0018	0.50
358	125	0.196	0.69	0.39	33.7%	0.040		0.35	0.05	3.00	0.0018	0.50
359	110	0.173	0.67	0.36	34.0%	0.046		0.35	0.05	3.00	0.0018	0.50
360	96	0.150	0.93	0.50	34.0%	0.037		0.35	0.05	3.00	0.0018	0.50
361	94	0.146	0.86	0.46	35.0%	0.035		0.35	0.05	3.00	0.0018	0.50
SCOTT GULCH (DRAINAGEWAY ID 4618)												
401	70	0.110	0.82	0.52	9.9%	0.016	25.9	0.40	0.05	4.59	0.0014	0.73
402	47	0.074	0.63	0.33	20.1%	0.030	21.2	0.35	0.05	4.29	0.0016	0.65
403	70	0.110	0.62	0.25	20.0%	0.035	24.2	0.35	0.05	4.16	0.0016	0.63
404	44	0.069	0.53	0.26	20.2%	0.021	23.9	0.35	0.05	3.83	0.0017	0.59
405	134	0.210	0.97	0.53	21.0%	0.047		0.35	0.05	4.04	0.0018	0.58
[redacted]	90	0.140	0.67	0.27	24.9%	0.046	23.3	0.35	0.05	3.87	0.0018	0.56
407	102	0.160	0.68	0.32	28.7%	0.040		0.35	0.05	3.70	0.0018	0.55
408	31	0.048	0.52	0.31	24.6%	0.057	21.2	0.35	0.05	3.79	0.0018	0.55
409	134	0.210	1.10	0.63	21.9%	0.045		0.35	0.05	3.61	0.0018	0.55
410	90	0.140	0.57	0.28	11.9%	0.045	20.7	0.40	0.05	3.86	0.0018	0.56
412	51	0.079	0.58	0.26	18.4%	0.039	22.0	0.40	0.05	3.32	0.0018	0.52
413	90	0.141	0.55	0.29	18.0%	0.049		0.40	0.05	3.44	0.0018	0.53
414	102	0.160	0.99	0.49	10.7%	0.036		0.40	0.05	3.82	0.0018	0.55

Italicized Subcatchment IDs indicate all values were adopted from the FHAD. The values for depression storage, initial and final infiltration, and decay rate were adopted from the FHAD for all subcatchments.

FIGURE A-4

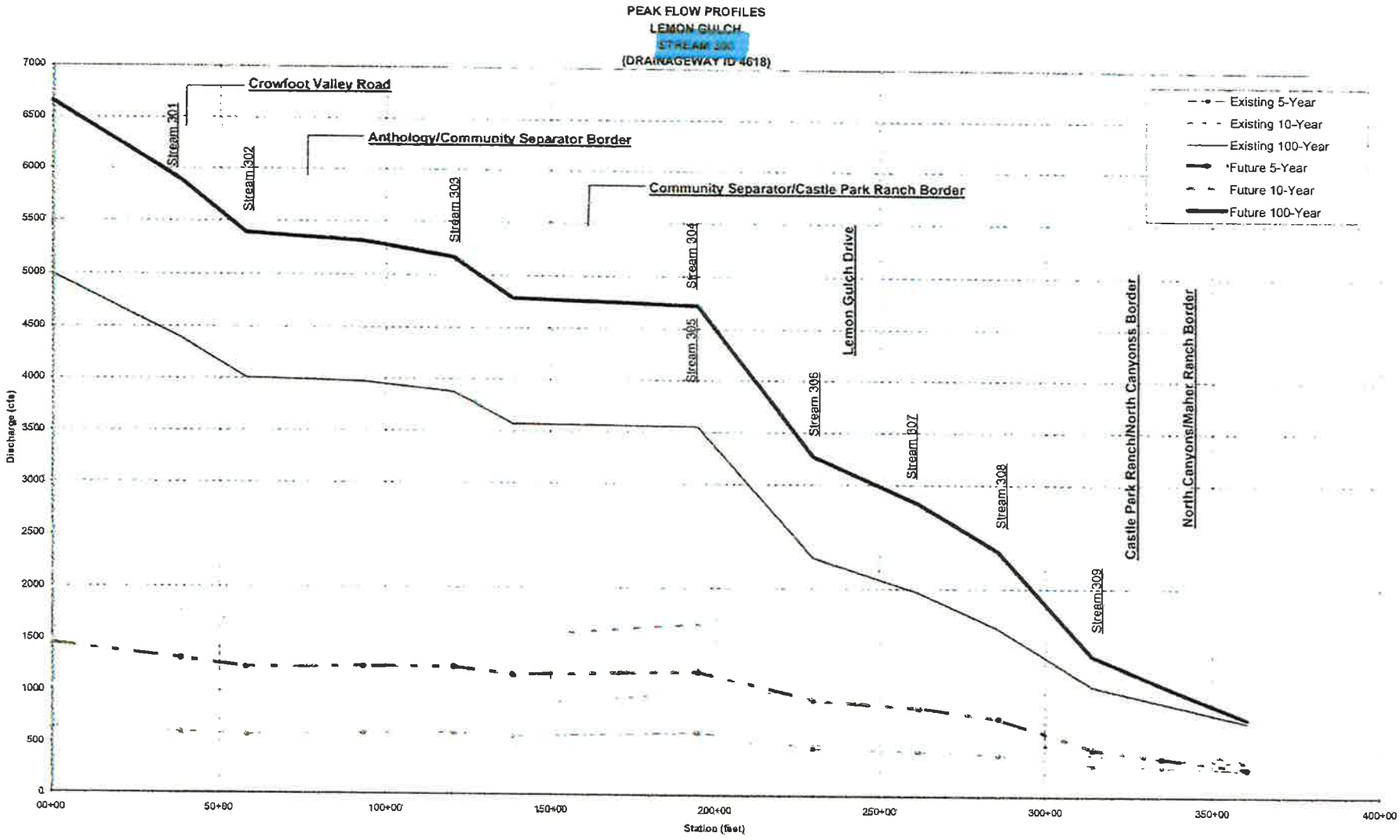
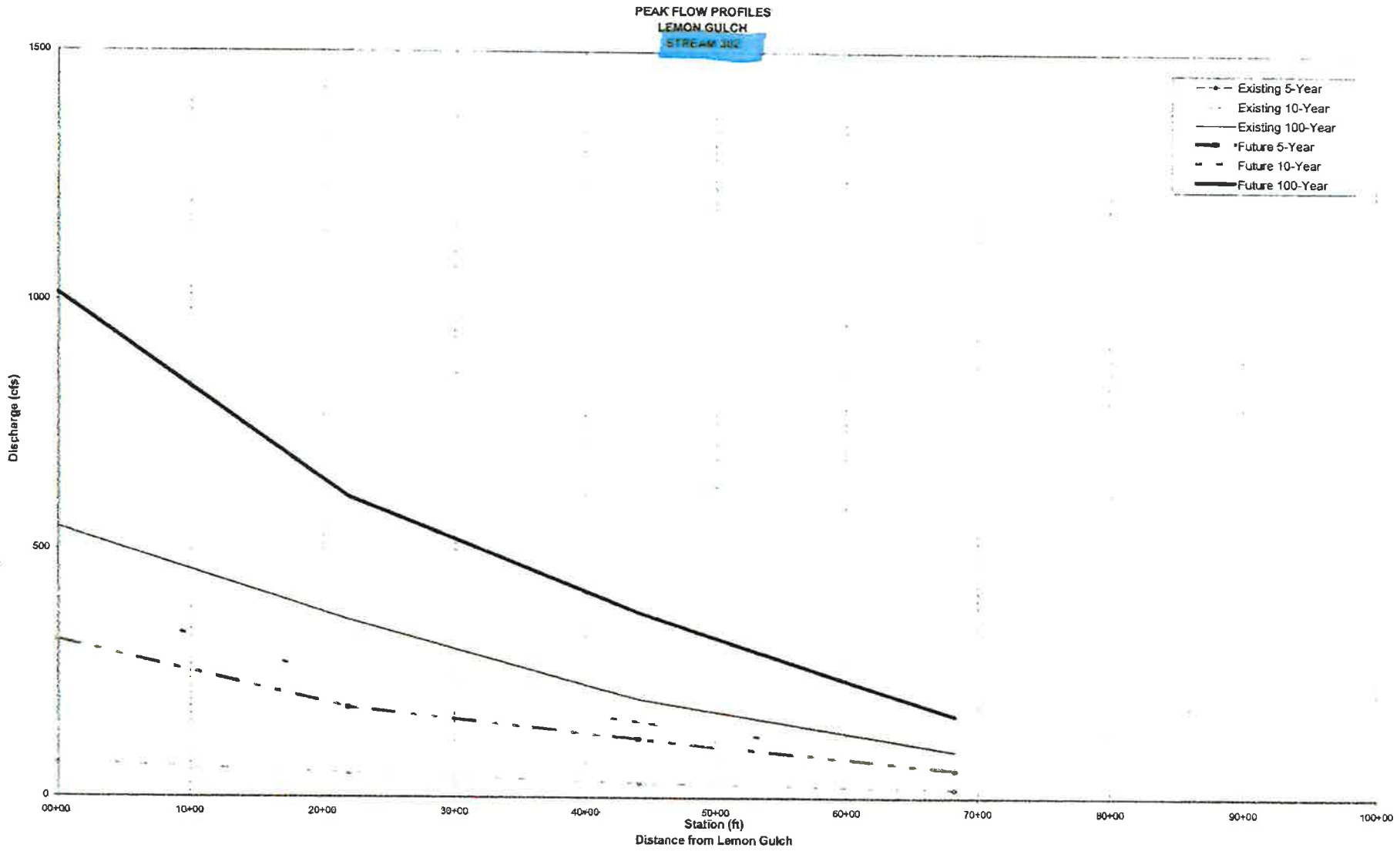
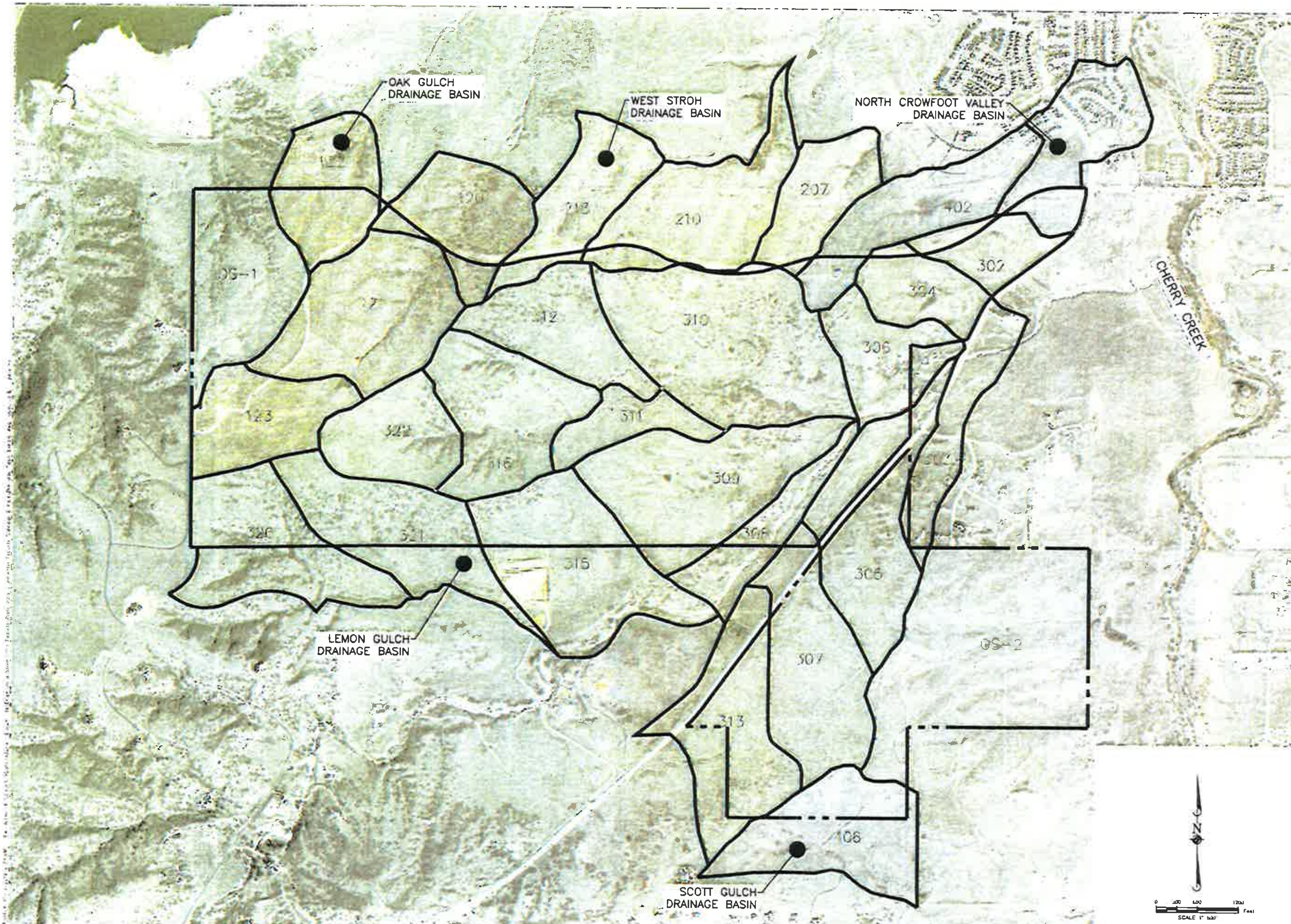

PEAK FLOW PROFILES
LEMON GULCH
STREAM 301
(DRAINAGEWAY ID 4618)
Crowfoot Valley Road
Anthology/Community Separator Border
Community Separator/Castle Park Ranch Border
Lemon Gulch Drive
Castle Park Ranch/North Canyonss Border
North Canyons/Maher Ranch Border
Stream 301
Stream 302
Stream 303
Stream 304
Stream 305
Stream 306
Stream 307
Stream 308
Stream 309
Existing 5-Year
Existing 10-Year
Existing 100-Year
*Future 5-Year
Future 10-Year
Future 100-Year
Discharge (cfs)
Station (feet)
0
500
1000
1500
2000
2500
3000
3500
4000
4500
5000
5500
6000
6500
7000
00+00
50+00
100+00
150+00
200+00
250+00
300+00
350+00
400+00

FIGURE A-6


PEAK FLOW PROFILES
LEMON GULCH
STREAM 302
Existing 5-Year
Existing 10-Year
Existing 100-Year
Future 5-Year
Future 10-Year
Future 100-Year
1500
1000
500
0
Discharge (cfs)
00+00
10+00
20+00
30+00
40+00
50+00
60+00
70+00
80+00
90+00
100+00
Station (ft)
Distance from Lemon Gulch

APPENDIX F

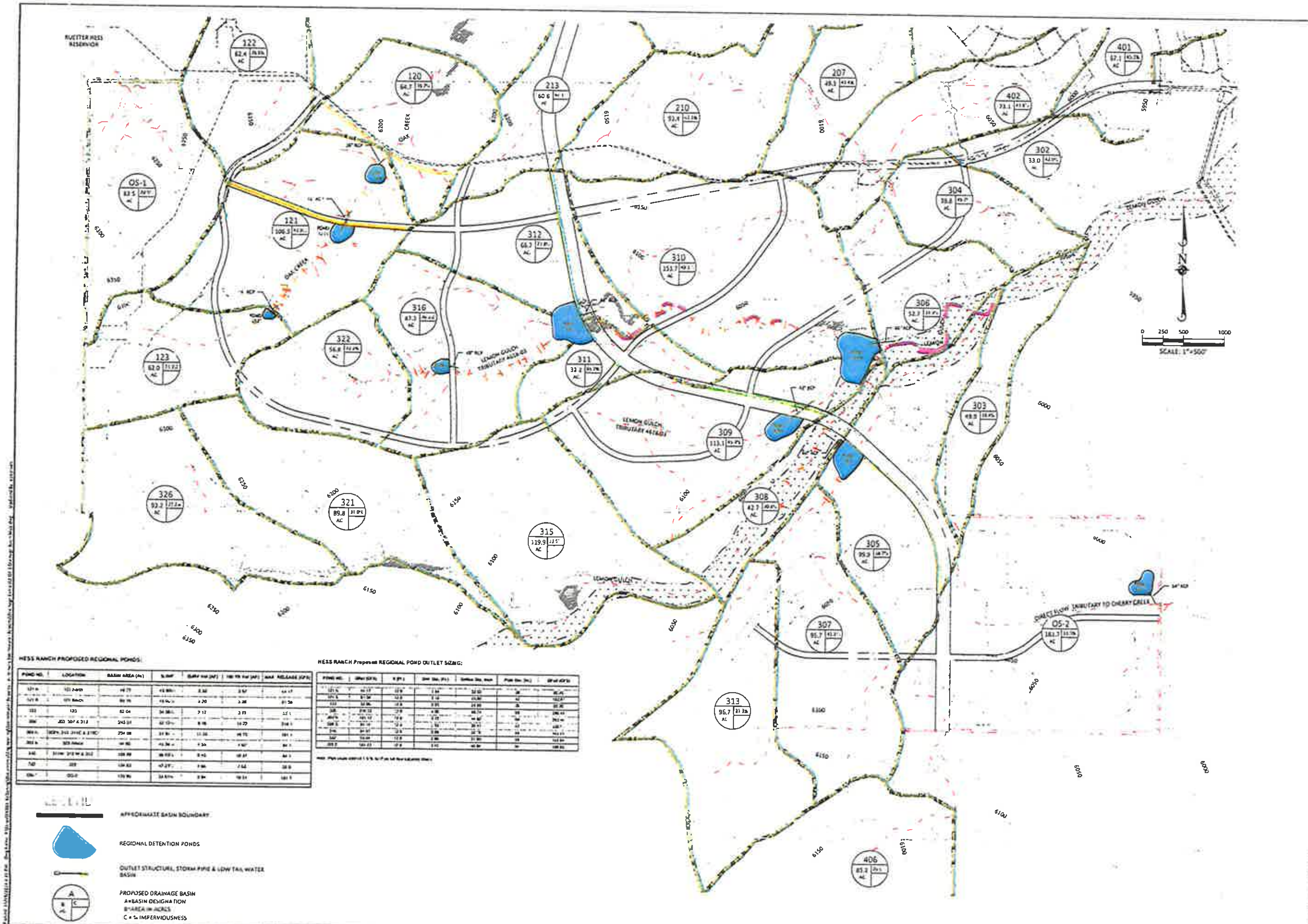
Basin Maps



Manhard CONSULTING

HESS RANCH DEVELOPMENT
TOWN OF PARKER, COLORADO
MAJOR DRAINAGE BASIN MAP

SHEET 1 OF 2



HESS RANCH PROPOSED REGIONAL PONDS:

POND NO.	LOCATION	BASIN AREA (Ac)	%IMP	[illegible] VOL (AF)	100-YR Vol (AF)	MAX. RELEASE (CFS)
121 N	121 North	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
121 S	121 South	[illegible]	[illegible]	4.20	[illegible]	[illegible]
123	123	62.04	[illegible]	2.17	[illegible]	[illegible]
[illegible]	303, 307 & 313	243.37	[illegible]	[illegible]	14.72	[illegible]
[illegible]	[illegible]	254.08	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	OS-2	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

HESS RANCH Proposed REGIONAL POND OUTLET SIZING:

POND NO.	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

LEGEND

APPROXIMATE BASIN BOUNDARY

REGIONAL DETENTION PONDS

OUTLET STRUCTURE, STORM PIPE & LOW TAIL WATER BASIN

PROPOSED DRAINAGE BASIN
A=BASIN DESIGNATION
B=AREA IN ACRES
C=% IMPERVIOUSNESS

Manhard CONSULTING

HESS RANCH DEVELOPMENT

TOWN OF PARKER, COLORADO

DRAINAGE BASIN MAP

SHEET 2 OF 2

SRD01