

**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

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JOB #2015-015

NOVEMBER 28, 2018

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I. CERTIFICATION PAGE

This report for the final design of (Name of Development) was prepared by me or under my direct supervision in accordance with the provisions of the Town of Parker Storm Drainage and Environmental Criteria Manual. I understand that the Town of Parker and its designated town authority do not and will not assume liability for drainage facilities designed by others.

Jerry W. Davidson, P.E.
Colorado P.E. License No. 30226
For and on Behalf of
Perception Design Group, Inc.

Seal and Date

**FINAL DRAINAGE REPORT
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II GENERAL LOCATION AND DESCRIPTION

A. Site Location:

This Final Drainage Report is prepared by Perception Design Group, Inc. as part of the Construction Plan / Final Plat submittal process for the Parker Pointe project proposed in Parker, Colorado. Parker Pointe, (Project / Site) is located on an unplatted parcel of land situated at the southeast corner of South Parker Road and Stroh Road. See appendix for vicinity map. The Site lies within the southwest quarter of Section 3, Township 7 South, Range 66 West of the 6th Prime Meridian, Douglas County, State of Colorado. The site is bounded by South Parker Road to the west, and Stroh Road to the north. Adjacent developments include the Colorado Golf club in Douglas County to the east, new commercial and residential development in the Town of Parker on the north side of Stroh Road, Commercial development in Parker on the west side of Parker Road, and undeveloped open space in Douglas County south of the property.

B. Site Location:

The Site occupies approximately 14.7 acres. Ground cover consists of pasture grasses. Site topography generally slopes from a tall mound in the northerly portion of the site down to the southwest towards Kinney Creek. Runoff north of the mound flows northwesterly towards the intersection of Stroh Road and Parker Road. Slopes vary widely from 3:1 on the mound to 6% over flatter portions of the site.

Site soils as shown by the USDA Web Soil Survey indicate that primarily Sampson Loam and Bresser Truckton Sandy Loam soil is present. This soil is sandy clay loam in nature. It is a type B hydrologic soil. Additionally Loamy Alluvial Land soils are present to a lesser extent. This soil is also clay loam in nature. It is a type C hydrologic soil.

The site falls within the Cherry Creek basin. The Kinney Creek tributary lies along the southern border of the site. This tributary has a delineated floodplain which encroaches on the extreme southwest corner of the site.

There are no irrigation canals or ditches on site. Additionally, there are no significant geologic features on site.

The site is presently partially developed with a house and barns. These structures are to be removed as a part of the proposed development. As a part of this application, the site will be developed with graded pad sites for commercial and retail businesses, drives, and utilities.

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III DRAINAGE BASINS AND SUB-BASINS

A. Major Drainage Basins:

The site falls within two major drainage basins. The southerly portion of the site is tributary to Kinney Creek. Areas from the peak of the aforementioned mound and to the north are tributary to Stroh Gulch. The southerly line of basin H1 defines the historic break between the two basins. Per the Town of Parker, the majority of this runoff to Stroh Gulch is captured and conveyed via storm sewer to the new detention pond and ultimately to Kinney Creek. A final drainage report was prepared for Stroh Crossing Filing No. 1 by Calibre Engineering. This is the development on the north side of Stroh Road. This report anticipated runoff from the Parker Pointe property and made allowance to handle the flow. Basin ST-2b from the Calibre report quantifies 18.9 cfs for the basin. Basin SR2 in this report indicates 4.1 cfs tributary to Stroh Road downstream of the newly placed inlet on Stroh Road for Basin SR1 plus carryover from Inlet SR1 of 3.2 cfs for a total tributary to Stroh Gulch of 7.3 cfs.

Kinney Creek was studied by WRC Engineering Inc. in a report entitled "Flood Hazard Area Delineation for Kinney Creek Fonder Draw and Tributaries" date April 2004. Floodplain was determined along the southwest corner of the site. Minor grading is proposed in the floodplain along Parker Road. Roadway widening encroaches upon and places fill in the floodplain. To mitigate this the shoulder borrow ditch is shifted east in similar size to replace filled floodplain with like volume and shape.

B. Minor Drainage Basins:

To facilitate design, the site is divided into multiple sub-basins described as follows:

Basins L1 thru L15 are used to represent each of the proposed lots. As development conditions are not yet determined, an assumed 95% imperviousness is established for each basin. A storm sewer stub is provided for each lot to convey developed runoff to the extended detention basin at the southeast corner of the site providing both detention and water quality facilities. While Basins L10 thru L15 drain towards Parker road in the overlot condition, it is required that these lots convey site runoff to the mainline storm sewer down the center access drive. Due to the presence of the Magellan gas pipeline and it's limited cover requirements as well as site visibility lines to the easterly lots, The west side of lots 10 thru 15 will remain below the center access drive. The storm sewer has been placed at maximum depth to accommodate these lots "bucking" grade with the storm sewer system.

Basins L1A thru L5A represents the easterly portion of Lots 1 thru 5. Runoff from these

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basins flow overland to the east to the drainage swale along the east property line thence into the extended detention facility. As development conditions are not yet determined, an assumed 95% imperviousness is established for each basin.

Basins IN1 thru IN3 are established to quantify runoff collected in a series of inlets along the central north-south access drive. This runoff is piped to the extended detention basin at the southeast corner of the site providing both detention and water quality facilities.

Basin SR1 is used to quantify runoff to Stroh Road from the road itself as well as offsite areas to the east. Detention and water quality are provided for this basin. See additional discussion under Major Basins above.

Basin SR2 (along with basin U4) is used to quantify runoff to the new inlet at the intersection of Stroh Road and Parker Road. Detention and water quality are not provided for this basin.

Basin PR1 combines with Basins U2 and PR2 to define runoff to the new pair of inlets located at the low point of Parker Road. Basin PR1 is separated to quantify new paved area requiring water quality treatment. Treatment for Basin PR1 combined with Basin U2 is provided in a grass swale in the ROW of Parker Road leading down to Kinney Creek.

Basin PR2 is used to quantify runoff from existing Parker Road improvements to the new pair of inlets located at the low point of Parker Road. Water quality is not provided for this basin. Total flow to the inlets is a combination of Basins PR1, PR2, and U2.

Basin PR3 is not illustrated on the plan. This basin is used to quantify new paved areas in Parker Road north of Stroh Road. This basin encompasses the new left turn bay on Parker Road to Stroh Road. Runoff from this basin is treated for water quality in the existing grass buffer along the west side of Parker Road.

Basin U1 is on-site area that is not tributary to the detention / water quality facility. This basin encompasses Tracts A and B which are floodplain and mouse habitat areas. Detention and water quality are not provided for this basin.

Basin U2 is on-site area that is not tributary to the detention / water quality facility. This basin quantifies runoff escaping the site down the access road to Parker Road. Detention is not provided for this basin, however, water quality is provided in the grass swale referenced above in the PR1 basin description.

Basin U3 is on-site area that is not tributary to the detention / water quality facility. Runoff from this basin adjacent to Parker Road flows overland into Parker Road.

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Detention and water quality are not provided for this basin.

Basin U4 is on-site area that is not tributary to the detention / water quality facility. Runoff from this basin enters Stroh Road and is collected in inlet SR2.

Basin H1 is a historic basin quantifying historic runoff to Stroh Road. It is used as a check for Calibre basin ST-2b. Basin H1 indicates runoff of 17.0 cfs while basin ST2-b indicates 18.9 cfs. Variance is due to more accurate topography available for the Parker Pointe site and better defined drainage basin as well as differences in time of concentration.

Basin OS1 quantifies flows entering the extended detention pond from offsite flows from the Colorado Golf Club property east of the Parker Pointe property. Detention and water quality are provided for this offsite flow area in its present condition.

IV DRAINAGE DESIGN CRITERIA

A. Regulations:

Design calculations and methodologies are based upon the Town of Parker Storm Drainage and Environmental Criteria Manual. Additionally, the Urban Drainage Storm Drainage Criteria Manual Volumes 1 thru 3 are utilized.

B. Drainage Studies, Outfall System Plans:

The Final Drainage Report for Stroh Crossing Filing No. 1 by Calibre Engineering is used to identify allowable site discharge to Stroh Gulch. The WRC Engineering Inc. report entitled "Flood Hazard Area Delineation for Kinney Creek Fonder Draw and Tributaries" date April 2004 was utilized to map the floodplain elevations along the south property line. This study has negligible impact on the design presented.

C. Hydrology:

Runoff is calculated for both the 5 year and 100 year storms using the rational method. On-site basins utilize a 5 minute time of concentration with 5 year intensity of 4.7 in/hr and 100 year at 8.85 in/hr. Detention storage volumes are calculated using the UDFCD ver 3.07 UD-Detention spreadsheet. This spreadsheet is also utilized to calculate allowable release rates.

D. Hydraulics:

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Storm sewer capacities are calculated using Hydraflow Storm Sewers extension for AutoCAD Civil 3D ver 2017. The system is designed such to provide minimal surcharge for the 100 year event, and no surcharge for the 5 year event. The Hydraflow software is also used to calculate hydraulic grade lines for the storm sewer.

E. Water Quality Enhancement:

Water quality is achieved in an extended detention facility designed to EURV specifications using UDFCD ver 3.07 UD-Detention spreadsheet.

V STORMWATER MANAGEMENT FACILITY DESIGN

A. Stormwater Conveyance Facilities:

Developed stormwater is generally conveyed towards the central north south driveway where stubs are provided that connect to a storm sewer mainline. The storm main runs in a southerly then easterly direction to the proposed EDB detention facility. Total developed site runoff tributary to the EDB is 146.51 cfs. Storm sewer outfall into the EDB occurs at a concrete forebay. Outfall from the EDB is controlled to code levels and discharged via storm sewer pipe to Kinney Creek where riprap is provided to control erosion. Storm sewer is placed in an easement for perpetual maintenance. Do to the depth of the pond and invert of the adjacent Kinney Creek, outfall is piped westerly to discharge near the box culvert under Parker Road where more favorable elevations exist.

B. Stormwater Storage Facilities:

Stormwater storage on site is accomplished in an extended detention basin located offsite near the southeast corner of the site. Required pond design elements are summarized below:

<u>Volume Element</u>	<u>Volume</u>	<u>Elevation</u>	<u>Release Rate</u>
WQCV	0.566 Ac-Ft	5966.12	41 hours
EURV + WQCV	1.472 Ac-Ft	5968.03	70 hours
100 year	2.753 Ac-Ft	5970.05	36.7 cfs
Storage Provided	2.753 Ac-Ft	5970.05	

Outflow metering is accomplished in a concrete outlet structure. 2 orifices are used. One for WQ and EURV while a second is used covering the outfall pipe to limit the 100 year flow. A double type D inlet is proposed to provide sufficient weir flow to accommodate

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the 100 year release rate. Micropool and trash racks are provided. Emergency overflow occurs directly into Kinney Creek via overflow weir and riprap embankment.

Allowable 100 year discharge must be reduced to allow for uncaptured basin U1 thru U4 and PR1. The combined 100 year un-detained flow from basins U1 thru U4 is 4.4 cfs. Basin PR1 is 3.2 cfs. Combined reduction in allowable discharge is 7.6 cfs. Allowable detention discharge as shown on the UDFCD spreadsheet is 44.5 cfs. The outlet structure design limits discharge to 36.7 cfs. Calculating the allowable discharge results in $44.5 - 7.6 = 36.9$ cfs. The design discharge is less than allowable therefore this provides adequate compensation for the uncaptured flows.

A maintenance access is provided entering at the northwest corner of the pond. Roadbase surfacing is provided and slopes not exceeding 10% are employed to enhance access for maintenance. An easement is provided over the pond should Town access, inspection, or repairs be required.

C. Water quality Enhancement Best Management Practices:

The EDB pond design includes water quality capture volume. Developed flows are conveyed via underground storm sewer to a single discharge point into the pond. At this point, a concrete forebay is provided to capture heavier particulate material.

Water quality treatment is also provided for the new paved areas of Parker Road. Basin PR1 is treated in a grass swale with discharge to Kinney Creek. Basin PR3 is treated in the existing grass buffer along the west side of Parker Road north of Stroh Road. UDFCD spreadsheets are provided for each treatment facility in the appendix.

D. Floodplain Modification:

Minor grading is proposed in the floodplain along Parker Road. Roadway widening encroaches upon and places fill in the floodplain. To mitigate this the shoulder borrow ditch is shifted east in similar size to replace filled floodplain with like volume and shape. A floodplain development permit will be required for this work as well as disturbances due to outfall construction. A no rise analysis has been performed and the results indicating compliance are included in the appendix.

E. Additional Permitting Requirements:

State stormwater permit for discharges during construction.
Town of Parker permits.
Douglas County permits.

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V CONCLUSIONS

A. Compliance with Standards:

The plans and calculations presented are in compliance with Town of Parker, Douglas County, and Urban Drainage requirements.

B. Variances:

No variances are requested.

V REFERENCES

Urban Drainage and Flood Control District Drainage Criteria Manual, Current addition.
Town of Parker Storm Drainage Criteria Manual
Town of Parker Construction Best Management Practices
Douglas County Storm Drainage Design and Technical Criteria Manual
USDA Web Soil Survey

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APPENDICES

APPENDIX A	HYDROLOGIC CALCULATIONS
APPENDIX B	DETENTION AND WATER QUALITY CALCULATIONS
APPENDIX C	HYDRAULIC CALCULATIONS
APPENDIX D	KEYMAP, FIRM, SOILS
APPENDIX E	DRAINAGE MAP
APPENDIX F	FLOODPLAIN NO RISE REPORT

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APPENDIX A HYDROLOGIC CALCULATIONS

Table RO-3—Recommended Percentage Imperviousness Values

Land Use or Surface Characteristics	Percentage Imperviousness
Business:	
Commercial areas	95
Neighborhood areas	85
Residential:	
Single-family	*
Multi-unit (detached)	60
Multi-unit (attached)	75
Half-acre lot or larger	*
Apartments	80
Industrial:	
Light areas	80
Heavy areas	90
Parks, cemeteries	5
Playgrounds	10
Schools	50
Railroad yard areas	15
Undeveloped Areas:	
Historic flow analysis	2
Greenbelts, agricultural	2
Off-site flow analysis (when land use not defined)	45
Streets:	
Paved	100
Gravel (packed)	40
Drive and walks	90
Roofs	90
Lawns, sandy soil	0
Lawns, clayey soil	0

* See [Figures RO-3](#) through [RO-5](#) for percentage imperviousness.

$$C_A = K_A + (1.31i^3 - 1.44i^2 + 1.135i - 0.12) \text{ for } C_A \geq 0, \text{ otherwise } C_A = 0 \quad (\text{RO-6})$$

$$C_{CD} = K_{CD} + (0.858i^3 - 0.786i^2 + 0.774i + 0.04) \quad (\text{RO-7})$$

$$C_B = (C_A + C_{CD})/2$$

Table RO-5— Runoff Coefficients, C

Percentage Imperviousness	Type C and D NRCS Hydrologic Soil Groups					
	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr
0%	0.04	0.15	0.25	0.37	0.44	0.50
5%	0.08	0.18	0.28	0.39	0.46	0.52
10%	0.11	0.21	0.30	0.41	0.47	0.53
15%	0.14	0.24	0.32	0.43	0.49	0.54
20%	0.17	0.26	0.34	0.44	0.50	0.55
25%	0.20	0.28	0.36	0.46	0.51	0.56
30%	0.22	0.30	0.38	0.47	0.52	0.57
35%	0.25	0.33	0.40	0.48	0.53	0.57
40%	0.28	0.35	0.42	0.50	0.54	0.58
45%	0.31	0.37	0.44	0.51	0.55	0.59
50%	0.34	0.40	0.46	0.53	0.57	0.60
55%	0.37	0.43	0.48	0.55	0.58	0.62
60%	0.41	0.46	0.51	0.57	0.60	0.63
65%	0.45	0.49	0.54	0.59	0.62	0.65
70%	0.49	0.53	0.57	0.62	0.65	0.68
75%	0.54	0.58	0.62	0.66	0.68	0.71
80%	0.60	0.63	0.66	0.70	0.72	0.74
85%	0.66	0.68	0.71	0.75	0.77	0.79
90%	0.73	0.75	0.77	0.80	0.82	0.83
95%	0.80	0.82	0.84	0.87	0.88	0.89
100%	0.89	0.90	0.92	0.94	0.95	0.96
TYPE B NRCS HYDROLOGIC SOILS GROUP						
0%	0.02	0.08	0.15	0.25	0.30	0.35
5%	0.04	0.10	0.19	0.28	0.33	0.38
10%	0.06	0.14	0.22	0.31	0.36	0.40
15%	0.08	0.17	0.25	0.33	0.38	0.42
20%	0.12	0.20	0.27	0.35	0.40	0.44
25%	0.15	0.22	0.30	0.37	0.41	0.46
30%	0.18	0.25	0.32	0.39	0.43	0.47
35%	0.20	0.27	0.34	0.41	0.44	0.48
40%	0.23	0.30	0.36	0.42	0.46	0.50
45%	0.26	0.32	0.38	0.44	0.48	0.51
50%	0.29	0.35	0.40	0.46	0.49	0.52
55%	0.33	0.38	0.43	0.48	0.51	0.54
60%	0.37	0.41	0.46	0.51	0.54	0.56
65%	0.41	0.45	0.49	0.54	0.57	0.59
70%	0.45	0.49	0.53	0.58	0.60	0.62
75%	0.51	0.54	0.58	0.62	0.64	0.66
80%	0.57	0.59	0.63	0.66	0.68	0.70
85%	0.63	0.66	0.69	0.72	0.73	0.75
90%	0.71	0.73	0.75	0.78	0.80	0.81
95%	0.79	0.81	0.83	0.85	0.87	0.88
100%	0.89	0.90	0.92	0.94	0.95	0.96

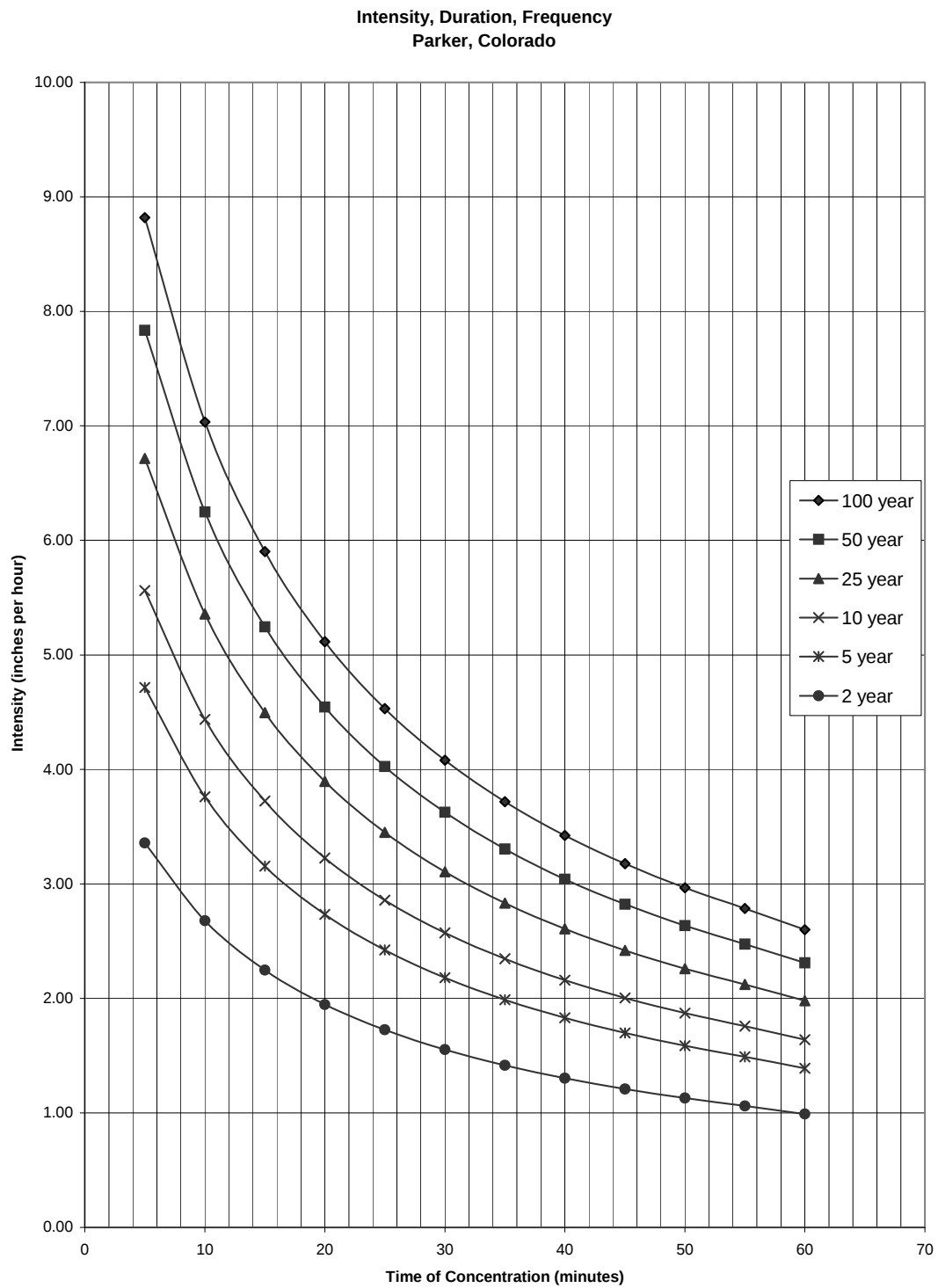


FIGURE 5.1
RAINFALL INTENSITY VERSUS DURATION CURVES FOR PARKER, COLORADO

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Designed by: JWD
 Checked by: JWD
 Date: 18-Sep-17
 Job Number: 2015-015

Project: Parker Pointe

COMPOSITE RUNOFF COEFFICIENTS

TYPE B SOILS

	FUTURE COMMERCIAL		DRIVES/WALKS/ROOF		LANDSCAPING				
Catchment	Area (Ac.)	C	Area (Ac.)	C	Area (Ac.)	C	Composite C	Catchment Area (Ac.)	Imperviousness
	Imperviousness = 95%		Imperviousness = 100%		Imperviousness = 2%				
H1 (5 YR)	0.00	0.81	0.00	0.90	10.52	0.09	0.09	10.52	2.0%
H1 (100 YR)	0.00	0.88	0.00	0.96	10.52	0.36	0.36	10.52	
L1 (5 YR)	0.71	0.81	0.00	0.90	0.00	0.09	0.81	0.71	95.0%
L1 (100 YR)	0.71	0.88	0.00	0.96	0.00	0.36	0.88	0.71	
L2 (5 YR)	0.50	0.81	0.00	0.90	0.00	0.09	0.81	0.50	95.0%
L2 (100 YR)	0.50	0.88	0.00	0.96	0.00	0.36	0.88	0.50	
L2A (5 YR)	0.19	0.81	0.00	0.90	0.00	0.09	0.81	0.19	95.0%
L2A (100 YR)	0.19	0.88	0.00	0.96	0.00	0.36	0.88	0.19	
L3 (5 YR)	0.43	0.81	0.00	0.90	0.00	0.09	0.81	0.43	95.0%
L3 (100 YR)	0.43	0.88	0.00	0.96	0.00	0.36	0.88	0.43	
L3A (5 YR)	0.16	0.81	0.00	0.90	0.00	0.09	0.81	0.16	95.0%
L3A (100 YR)	0.16	0.88	0.00	0.96	0.00	0.36	0.88	0.16	
L4 (5 YR)	0.63	0.81	0.00	0.90	0.00	0.09	0.81	0.63	95.0%
L4 (100 YR)	0.63	0.88	0.00	0.96	0.00	0.36	0.88	0.63	
L4A (5 YR)	0.24	0.81	0.00	0.90	0.00	0.09	0.81	0.24	95.0%
L4A (100 YR)	0.24	0.88	0.00	0.96	0.00	0.36	0.88	0.24	

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TYPE B SOILS

Catchment	FUTURE COMMERCIAL		DRIVES/WALKS/ROOF		LANDSCAPING		Composite C	Catchment Area (Ac.)	Imperviousness
	Area (Ac.)	C	Area (Ac.)	C	Area (Ac.)	C			
	Imperviousness = 95%		Imperviousness = 100%		Imperviousness = 2%				
L5 (5 YR)	0.63	0.81	0.00	0.90	0.00	0.09	0.81	0.63	95.0%
L5 (100 YR)	0.63	0.88	0.00	0.96	0.00	0.36	0.88	0.63	
L5A (5 YR)	0.24	0.81	0.00	0.90	0.00	0.09	0.81	0.24	95.0%
L5A (100 YR)	0.24	0.88	0.00	0.96	0.00	0.36	0.88	0.24	
L6 (5 YR)	0.78	0.81	0.00	0.90	0.00	0.09	0.81	0.78	95.0%
L6 (100 YR)	0.78	0.88	0.00	0.96	0.00	0.36	0.88	0.78	
L7 (5 YR)	0.68	0.81	0.00	0.90	0.00	0.09	0.81	0.68	95.0%
L7 (100 YR)	0.68	0.88	0.00	0.96	0.00	0.36	0.88	0.68	
L8 (5 YR)	0.87	0.81	0.00	0.90	0.00	0.09	0.81	0.87	95.0%
L8 (100 YR)	0.87	0.88	0.00	0.96	0.00	0.36	0.88	0.87	
L9 (5 YR)	0.71	0.81	0.00	0.90	0.00	0.09	0.81	0.71	95.0%
L9 (100 YR)	0.71	0.88	0.00	0.96	0.00	0.36	0.88	0.71	
L10 (5 YR)	0.88	0.81	0.00	0.90	0.00	0.09	0.81	0.88	95.0%
L10 (100 YR)	0.88	0.88	0.00	0.96	0.00	0.36	0.88	0.88	
L11 (5 YR)	0.92	0.81	0.00	0.90	0.00	0.09	0.81	0.92	95.0%
L11 (100 YR)	0.92	0.88	0.00	0.96	0.00	0.36	0.88	0.92	

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TYPE B SOILS

Catchment	FUTURE COMMERCIAL		DRIVES/WALKS/ROOF		LANDSCAPING		Composite C	Catchment Area (Ac.)	Imperviousness
	Area (Ac.)	C	Area (Ac.)	C	Area (Ac.)	C			
	Imperviousness = 95%		Imperviousness = 100%		Imperviousness = 2%				
L12 (5 YR)	0.56	0.81	0.00	0.90	0.00	0.09	0.81	0.56	95.0%
L12 (100 YR)	0.56	0.88	0.00	0.96	0.00	0.36	0.88	0.56	
L13 (5 YR)	0.73	0.81	0.00	0.90	0.00	0.09	0.81	0.73	95.0%
L13 (100 YR)	0.73	0.88	0.00	0.96	0.00	0.36	0.88	0.73	
L14 (5 YR)	0.73	0.81	0.00	0.90	0.00	0.09	0.81	0.73	95.0%
L14 (100 YR)	0.73	0.88	0.00	0.96	0.00	0.36	0.88	0.73	
L15 (5 YR)	0.72	0.81	0.00	0.90	0.00	0.09	0.81	0.72	95.0%
L15 (100 YR)	0.72	0.88	0.00	0.96	0.00	0.36	0.88	0.72	
IN1 (5 YR)	0.00	0.81	0.26	0.90	0.00	0.09	0.90	0.26	100.0%
IN1 (100 YR)	0.00	0.88	0.26	0.96	0.00	0.36	0.96	0.26	
IN2 (5 YR)	0.00	0.81	0.53	0.90	0.00	0.09	0.90	0.53	100.0%
IN2 (100 YR)	0.00	0.88	0.53	0.96	0.00	0.36	0.96	0.53	
IN3 (5 YR)	0.00	0.81	0.11	0.90	0.00	0.09	0.90	0.11	100.0%
IN3 (100 YR)	0.00	0.88	0.11	0.96	0.00	0.36	0.96	0.11	
OS1 (5 YR)	0.00	0.81	1.21	0.90	22.13	0.09	0.13	23.34	7.1%
OS1 (100 YR)	0.00	0.88	1.21	0.96	22.13	0.36	0.39	23.34	
SR1 (5 YR)	0.00	0.81	0.40	0.90	3.35	0.09	0.18	3.75	12.5%

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 Checked by: JWD
 Date: 18-Sep-17
 Job Number: 2015-015

Project: Parker Pointe

COMPOSITE RUNOFF COEFFICIENTS

TYPE B SOILS

	FUTURE COMMERCIAL		DRIVES/WALKS/ROOF		LANDSCAPING				
Catchment	Area (Ac.)	C	Area (Ac.)	C	Area (Ac.)	C	Composite C	Catchment Area (Ac.)	Imperviousness
	Imperviousness = 95%		Imperviousness = 100%		Imperviousness = 2%				
SR1 (100 YR)	0.00	0.88	0.40	0.96	3.35	0.36	0.42	3.75	
AREA TO POND	11.31	0.88	2.51	0.96	25.48	0.36	0.55	39.30	35.0%
U1 (5 YR)	0.00	0.81	0.00	0.90	1.37	0.09	0.09	1.37	2.0%
U1 (100 YR)	0.00	0.88	0.00	0.96	1.37	0.36	0.36	1.37	
U2 (2 YR)	0.00	0.79	0.24	0.89	0.06	0.02	0.72	0.30	
U2 (5 YR)	0.00	0.81	0.24	0.90	0.06	0.09	0.74	0.30	80.4%
U2 (100 YR)	0.00	0.88	0.24	0.96	0.06	0.36	0.84	0.30	
U3 (5 YR)	0.00	0.81	0.00	0.90	0.17	0.09	0.09	0.17	2.0%
U3 (100 YR)	0.00	0.88	0.00	0.96	0.17	0.36	0.36	0.17	
U4 (5 YR)	0.00	0.81	0.14	0.90	0.09	0.09	0.58	0.23	61.7%
U4 (100 YR)	0.00	0.88	0.14	0.96	0.09	0.36	0.73	0.23	
POND DESIGN NUMBERS	11.31	0.88	2.89	0.96	27.17	0.36	0.54	41.37	34.3%
SR2 (5 YR)	0.00	0.81	0.31	0.90	0.01	0.09	0.87	0.32	96.9%
SR2 (100 YR)	0.00	0.88	0.31	0.96	0.01	0.36	0.94	0.32	
PR1 (2 YR)	0.00	0.79	0.35	0.89	0.07	0.02	0.75	0.42	
PR1 (5 YR)	0.00	0.81	0.35	0.90	0.07	0.09	0.77	0.42	83.7%
PR1 (100 YR)	0.00	0.88	0.35	0.96	0.07	0.36	0.86	0.42	

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Project: Parker Pointe

COMPOSITE RUNOFF COEFFICIENTS

TYPE B SOILS

	<u>FUTURE COMMERCIAL</u>		<u>DRIVES/WALKS/ROOF</u>		<u>LANDSCAPING</u>				
Catchment	Area (Ac.)	C	Area (Ac.)	C	Area (Ac.)	C	Composite C	Catchment Area (Ac.)	Imperviousness
	Imperviousness = 95%		Imperviousness = 100%		Imperviousness = 2%				
PR2 (5 YR)	0.00	0.81	0.91	0.90	0.00	0.09	0.90	0.91	100.0%
PR2 (100 YR)	0.00	0.88	0.91	0.96	0.00	0.36	0.96	0.91	
PR3 (2 YR)	0.00	0.79	0.22	0.89	0.00	0.02	0.89	0.22	

STANDARD FORM SF-2
TIME OF CONCENTRATION

PROJECT Parker Pointe
JN ASP-2015-015
CALCULATED BY JWD
DATE 18-Sep-17

SUB -BASIN		DATA	INITIAL/OVERLAND TIME (Ti)			TRAVEL TIME (Tt)					Tc CHECK			FINAL (Tc)	REMARKS
DESIGN	Area (ac)	C5	LENGTH Li (ft)	SLOPE (ft/ft)	Ti (min)	LENGTH - Lt (ft)	SLOPE %	K Conveyance Factor	VEL (fps)	Tt (min)	COMP (Tc)	Basin Imp. (decimal)	Tc=(UDFCD Eq 6-5) (min)	MIN	
SR1	3.23	0.27	375.0	0.030	20.4	540.0	2.50	20	3.16	2.8	23.3	0.241	22.4	22.4	22.4
H1	10.52	0.09	530.0	0.050	24.9	550.0	2.75	10	1.66	5.5	30.5	0.020	26.3	26.3	26.3
OS1	24.54	0.13	290.0	0.070	15.8	1425.0	6.00	10	2.45	9.7	25.5	0.020	26.7	25.5	25.5

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Project: Parker Pointe

RUNOFF CALCULATIONS

(RATIONAL METHOD)

Design Storm: 2-Yr.

		Direct Runoff						
	Design	Basin	Area	Runoff	CA	Tc	I	Q
	Point	Desig.	(Acres)	Coefficient		(min)	(in/hr)	(cfs)
		U2	0.30	0.72	0.22	5.0	3.40	0.73
		PR1	0.42	0.75	0.32	5.0	3.40	1.07
		PR3	0.22	0.89	0.20	5.0	3.40	0.67

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RUNOFF CALCULATIONS

(RATIONAL METHOD)

Design Storm: 5-Yr.

		Direct Runoff						
	Design	Basin	Area	Runoff	CA	Tc	I	Q
	Point	Desig.	(Acres)	Coefficient		(min)	(in/hr)	(cfs)
		H1	10.52	0.09	0.95	26.0	2.30	2.18
		L1	0.71	0.81	0.58	5.0	4.70	2.70
		L2	0.50	0.81	0.41	5.0	4.70	1.90
		L2A	0.19	0.81	0.15	5.0	4.70	0.72
		L3	0.43	0.81	0.35	5.0	4.70	1.64
		L3A	0.16	0.81	0.13	5.0	4.70	0.61
		L4	0.63	0.81	0.51	5.0	4.70	2.40
		L4A	0.24	0.81	0.19	5.0	4.70	0.91
		L5	0.63	0.81	0.51	5.0	4.70	2.40
		L5A	0.24	0.81	0.19	5.0	4.70	0.91
		L6	0.78	0.81	0.63	5.0	4.70	2.97
		L7	0.68	0.81	0.55	5.0	4.70	2.59
		L8	0.87	0.81	0.70	5.0	4.70	3.31
		L9	0.71	0.81	0.58	5.0	4.70	2.70
		L10	0.88	0.81	0.71	5.0	4.70	3.35
		L11	0.92	0.81	0.75	5.0	4.70	3.50
		L12	0.56	0.81	0.45	5.0	4.70	2.13
		L13	0.73	0.81	0.59	5.0	4.70	2.78
		L14	0.73	0.81	0.59	5.0	4.70	2.78
		L15	0.72	0.81	0.58	5.0	4.70	2.74
		IN1	0.26	0.90	0.23	5.0	4.70	1.10
		IN2	0.53	0.90	0.48	5.0	4.70	2.24
		IN3	0.11	0.9	0.10	5.0	4.70	0.47
		SR1	3.75	0.18	0.68	22.4	2.60	1.76
		SR2	0.32	0.87	0.28	5.0	4.70	1.31
		PR1	0.42	0.77	0.32	5.0	4.70	1.52
		PR2	0.91	0.96	0.87	5.0	4.70	4.11
		U1	1.37	0.09	0.12	5.0	4.70	0.58
		U2	0.3	0.74	0.22	5.0	4.70	1.04
		U3	0.17	0.09	0.02	5.0	4.70	0.07
		U4	0.23	0.58	0.13	5.0	4.70	0.63
		OS1	23.34	0.13	3.03	25.5	2.50	7.59

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 Job Number: 2015-015

Project: Parker Pointe

RUNOFF CALCULATIONS

(RATIONAL METHOD)

Design Storm: 100-Yr.

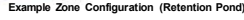
		Direct Runoff						
	Design	Basin	Area	Runoff	CA	Tc	I	Q
	Point	Desig.	(Acres)	Coefficient		(min)	(in/hr)	(cfs)
		H1	10.52	0.36	3.79	26.0	4.50	17.04
		L1	0.71	0.88	0.62	5.0	8.85	5.53
		L2	0.50	0.88	0.44	5.0	8.85	3.89
		L2A	0.19	0.88	0.17	5.0	8.85	1.48
		L3	0.43	0.88	0.38	5.0	8.85	3.35
		L3A	0.16	0.88	0.14	5.0	8.85	1.25
		L4	0.63	0.88	0.55	5.0	8.85	4.91
		L4A	0.24	0.88	0.21	5.0	8.85	1.87
		L5	0.63	0.88	0.55	5.0	8.85	4.91
		L5A	0.24	0.88	0.21	5.0	8.85	1.87
		L6	0.78	0.88	0.69	5.0	8.85	6.07
		L7	0.68	0.88	0.60	5.0	8.85	5.30
		L8	0.87	0.88	0.77	5.0	8.85	6.78
		L9	0.71	0.88	0.62	5.0	8.85	5.53
		L10	0.88	0.88	0.77	5.0	8.85	6.85
		L11	0.92	0.88	0.81	5.0	8.85	7.16
		L12	0.56	0.88	0.49	5.0	8.85	4.36
		L13	0.73	0.88	0.64	5.0	8.85	5.69
		L14	0.73	0.88	0.64	5.0	8.85	5.69
		L15	0.72	0.88	0.63	5.0	8.85	5.61
		IN1	0.26	0.96	0.25	5.0	8.85	2.21
		IN2	0.53	0.96	0.51	5.0	8.85	4.50
		IN3	0.11	0.96	0.11	5.0	8.85	0.93
		SR1	3.75	0.42	1.58	22.4	4.90	7.72
TOTAL FLOW TO FOREBAY								103.45
		OS1	23.34	0.39	9.10	25.5	4.50	40.96
TOTAL TO POND			39.30					144.41
		U1	1.37	0.36	0.49	25.5	4.50	2.22
		U2	0.3	0.84	0.25	25.5	4.50	1.13
		U3	0.17	0.36	0.06	25.5	4.50	0.28
		U4	0.23	0.73	0.17	25.5	4.50	0.76
UN-CAPTURED SITE RUNOFF								4.38
		SR2	0.32	0.94	0.30	5.0	8.85	2.66
		PR1	0.42	0.86	0.36	5.0	8.85	3.20
		PR2	0.91	0.96	0.87	5.0	8.85	7.73

**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

APPENDIX B DETENTION AND WATER QUALITY CALCULATIONS

UD-Detention, Version 3.07 (February 2017)

Basin ID:



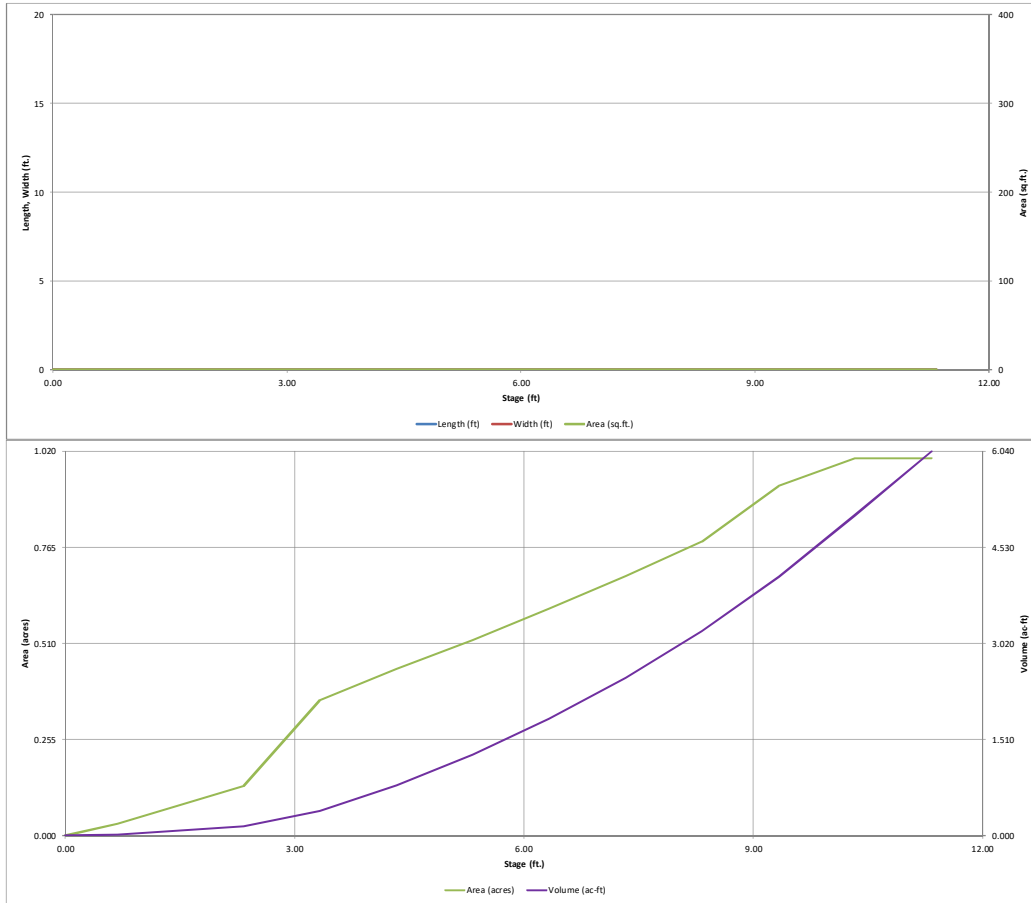
Selected BMP Type =	EDB
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Optional User Override
1-hr Precipitation

Zone 1 Volume (V_{QC1})	0.566	acre-feet
Zone 2 Volume (EURF - Zone 1)	0.906	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2)	1.281	acre-feet
Total Detention Basin Volume	2.753	acre-feet
Initial Surge Volume (V_{SV})	USEF	ft ³
Initial Surge Depth (H_{SD})	USEF	ft
Total Available Detention Depth (H_{DA})	USEF	ft
Depth of Trickle Channel (H_{TC})	USEF	ft
Slope of Trickle Channel (S_{TC})	USEF	ft/ft
Slopes of Main Basin Sides (S_{MS})	USEF	ft/ft
Basin Length-to-Width Ratio (R_{BW})	USEF	ft/V
Initial Surge Area (A_{SV})	USEF	ft ²
Surcharge Volume Length (L_{SV})	USEF	ft
Surcharge Volume Width (W_{SV})	USEF	ft
Depth of Basin Floor ($H_{B(100)}$)	USEF	ft
Length of Basin Floor ($L_{B(100)}$)	USEF	ft
Width of Basin Floor ($W_{B(100)}$)	USEF	ft
Area of Basin Floor ($A_{B(100)}$)	USEF	ft ²
Volume of Basin Floor ($V_{B(100)}$)	USEF	ft ³
Depth of Main Basin (H_{MB})	USEF	ft
Length of Main Basin (L_{MB})	USEF	ft
Width of Main Basin (W_{MB})	USEF	ft
Area of Main Basin (A_{MB})	USEF	ft ²
Volume of Main Basin (V_{MB})	USEF	ft ³
Calculated Total Basin Volume (V_{TB})	USEF	acre-feet

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

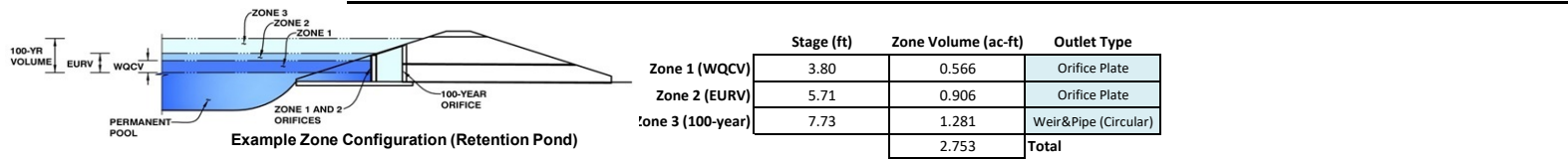


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: _____

Basin ID: _____



User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Calculated Parameters for Underdrain
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Area = N/A ft ²
			Underdrain Orifice Centroid = N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP) Calculated Parameters for Plate

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft ²
Depth at top of Zone using Orifice Plate =	5.27	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	12.00	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	inches	Elliptical Slot Area =	N/A	ft ²

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00	3.00				
Orifice Area (sq. inches)	1.50	1.50	1.50	1.50				
	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)	Calculated Parameters for Vertical Orifice
--	--

	Not Selected	Not Selected			Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)		Vertical Orifice Area =	N/A	ft ²
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)		Vertical Orifice Centroid =	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)	Calculated Parameters for Overflow Weir
---	---

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, H _o =	5.71	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H _i =	5.71	N/A	feet
Overflow Weir Front Edge Length =	6.00	N/A	feet	Over Flow Weir Slope Length =	6.00	N/A	feet
Overflow Weir Slope =	0.00	N/A	H:V (enter zero for flat grate)	Grate Open Area / 100-yr Orifice Area =	8.73	N/A	should be ≥ 4
Horiz. Length of Weir Sides =	6.00	N/A	feet	Overflow Grate Open Area w/o Debris =	25.20	N/A	ft ²
Overflow Grate Open Area % =	70%	N/A	%, grate open area/total area	Overflow Grate Open Area w/ Debris =	12.60	N/A	ft ²
Debris Clogging % =	50%	N/A	%				

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice) Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Circular	Not Selected			Zone 3 Circular	Not Selected	
Depth to Invert of Outlet Pipe =	0.25	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	2.89	N/A	ft ²
Circular Orifice Diameter =	23.00	N/A	inches	Outlet Orifice Centroid =	0.96	N/A	feet
				Half-Central Angle of Restrictor Plate on Pipe =	N/A	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal) Calculated Parameters for Spillway

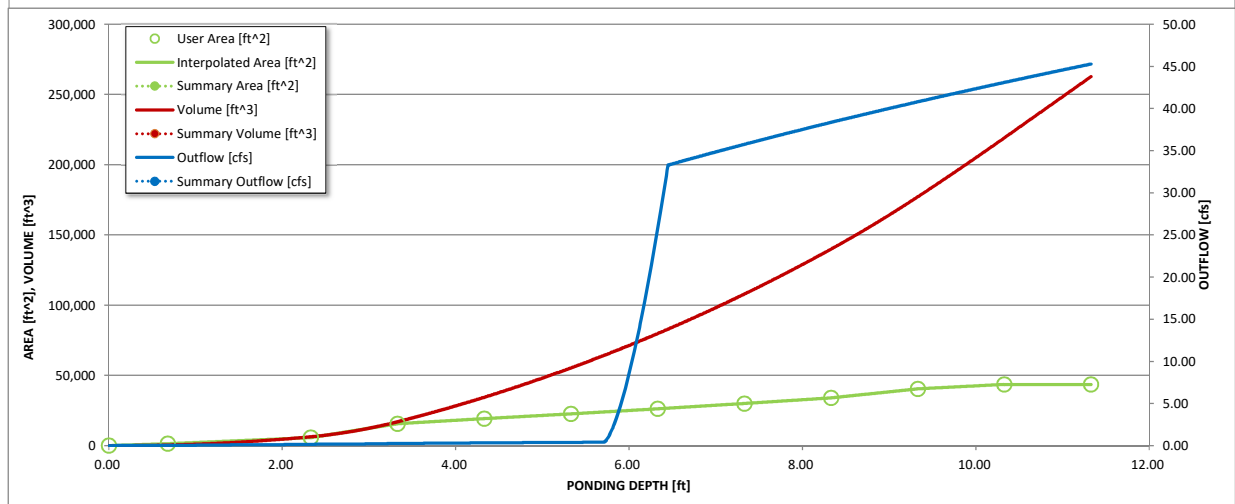
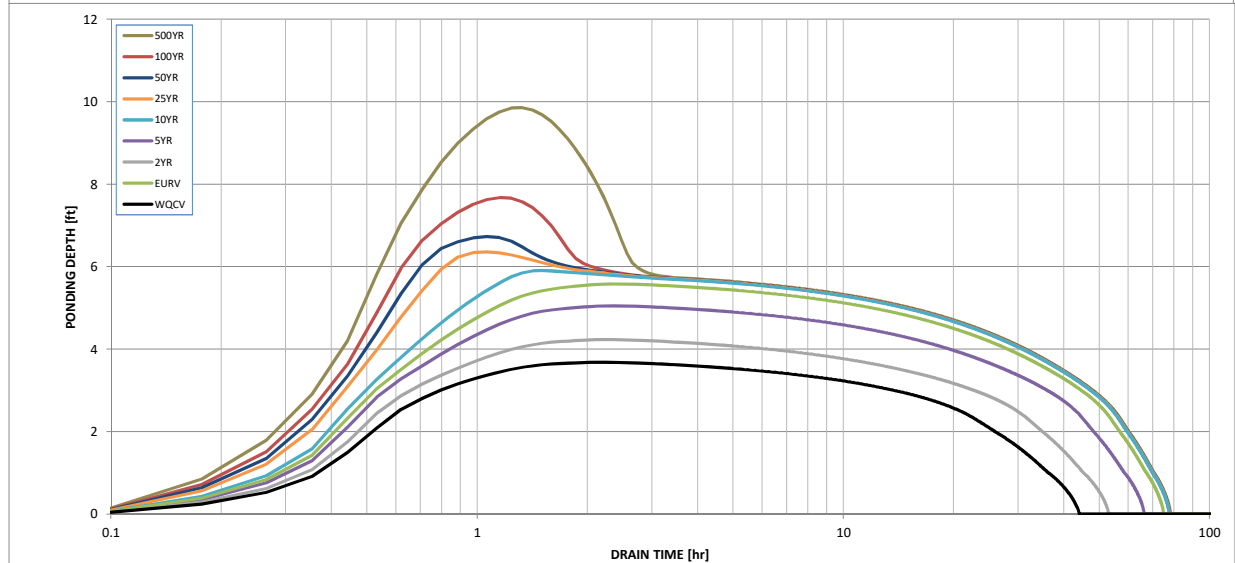
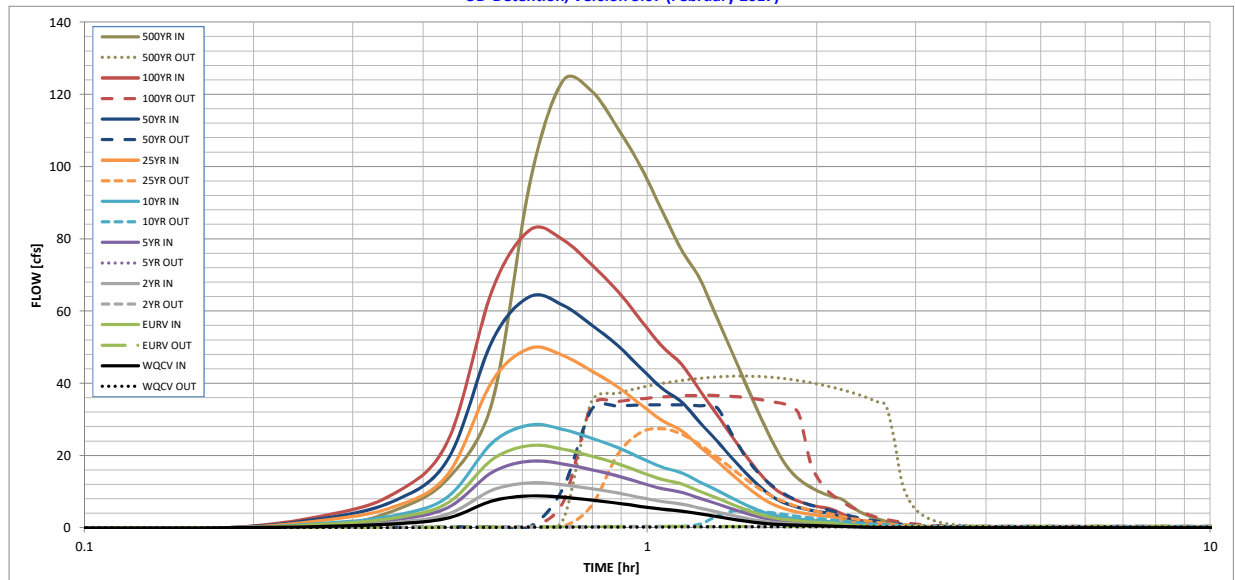
Spillway Invert Stage=	72.00	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.65	feet
Spillway Crest Length =	50.00	feet	Stage at Top of Freeboard =	74.65	feet
Spillway End Slopes =	3.00	H:V	Basin Area at Top of Freeboard =	1.00	acres
Freeboard above Max Water Surface =	2.00	feet			

Routed Hydrograph Results

Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in)	0.53	1.07	0.83	1.11	1.36	1.72	2.01	2.32	3.10
Calculated Runoff Volume (acre-ft)	0.566	1.472	0.799	1.191	1.844	3.245	4.201	5.449	8.252
OPTIONAL Override Runoff Volume (acre-ft)									
Inflow Hydrograph Volume (acre-ft)	0.566	1.471	0.799	1.191	1.844	3.246	4.201	5.442	8.249
Predevelopment Unit Peak Flow, q (cfs/acre)	0.00	0.00	0.01	0.02	0.17	0.60	0.87	1.20	1.90
Predevelopment Peak Q (cfs)	0.0	0.0	0.4	0.7	6.9	24.9	35.8	49.5	78.5
Peak Inflow Q (cfs)	8.9	22.8	12.5	18.5	28.5	49.7	64.1	82.5	123.8
Peak Outflow Q (cfs)	0.3	0.4	0.3	0.4	4.9	27.5	34.1	36.7	42.0
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	0.6	0.7	1.1	1.0	0.7	0.5
Structure Controlling Flow	Plate	Plate	Plate	Plate	Overflow Gate 1	Overflow Gate 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	0.2	1.1	1.3	1.4	1.6
Max Velocity through Gate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	38	64	45	57	65	61	58	55	50
Time to Drain 99% of Inflow Volume (hours)	41	70	50	62	71	69	67	66	63
Maximum Ponding Depth (ft)	3.68	5.58	4.23	5.05	5.91	6.36	6.73	7.68	9.86
Area at Maximum Ponding Depth (acres)	0.39	0.54	0.43	0.50	0.57	0.60	0.64	0.72	0.97
Maximum Volume Stored (acre-ft)	0.521	1.398	0.747	1.124	1.581	2.844	2.073	2.717	4.559

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename: _____

Storm Inflow Hydrographs

UD-Detention, Version 3.07 (February 2017)

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

[illegible]

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

[illegible]

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

Perception Design Group, Inc.
 6901 South Pierce Street, Suite 315
 Littleton, Colorado 80128
 (303) 232-8088 Fax (303) 232-5255

Designed by: JWD
 Checked by: JWD
 Date: 18-Sep-17
 Job Number: 2015-015

Project: Parker Pointe

Forebay Design

Required Forebay Volume = 3% of WQCV

WQCV = 0.551 ac-ft
 24,002 cubit feet

Required Forebay Volume = 720 cubit feet
 Provided Area= 750.00 square feet
 Provided Depth = 0.96 feet

Required Release = 2% of 100 year peak discharge

100 year discharge = 103.5 cfs
 Required Release Rate = 2.07 cfs

Use Weir Equation to Size Slot in Concrete

100 YEAR

Invert Elevation 62.32
 Ponding Elevation 63.32
 Required Discharge 1.77
 Coefficient of Discharge 3.40
 Length L 0.6042

$$Q=Cd \times L \times H^{0.5}$$

Cd	Length	Head (ft)	Discharge (cfs)
3.40	0.6042	1.00	2.05
	7.25"		

The open channel flow calculator									
Select Channel Type: Rectangle ▾		Depth from Q: ▾		Select unit system: Feet(ft) ▾					
Channel slope: 0.05	ft/ft	Water depth(y): 0.22	ft	LeftSlope (Z1): 0	to 1 (H/V)	Bottom W(b): 3.5	ft	RightSlope (Z2): 0	to 1 (H/V)
Flow velocity: 2.682216	ft/s	Input n value: 0.13		or select n					
Flow discharge: 2.07	ft ³ /s	Status: Calculation finished							
Calculate Reset									
Wetted perimeter: 3.94	ft	Flow area: 0.77	ft ²			Top width(T): 3.5	ft		
Specific energy: 0.33	ft	Froude number: 1.01			Flow status: Supercritical flow				
Critical depth: 0.23	ft	Critical slope: 0.0045	ft/ft			Velocity head: 0.11	ft		

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Design Procedure Form: Grass Swale (GS)

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 1

Designer: Jerry Davidson

Company: Perception Design Group, Inc.

Date: June 25, 2018

Project: Parker Pointe

Location: BASINS PR1 AND U2

1. Design Discharge for 2-Year Return Period	$Q_2 =$ 1.80 cfs
2. Hydraulic Residence Time A) : Length of Grass Swale B) Calculated Residence Time (based on design velocity below)	$L_S =$ 375.0 ft $T_{HR} =$ 6.7 minutes
3. Longitudinal Slope (vertical distance per unit horizontal) A) Available Slope (based on site constraints) B) Design Slope	$S_{avail} =$ 0.005 ft / ft $S_D =$ 0.005 ft / ft
4. Swale Geometry A) Channel Side Slopes ($Z = 4$ min., horiz. distance per unit vertical) B) Bottom Width of Swale (enter 0 for triangular section)	$Z =$ 4.00 ft / ft $W_B =$ 1.00 ft
5. Vegetation A) Type of Planting (seed vs. sod, affects vegetal retardance factor)	Choose One ☒ Grass From Seed ☐ Grass From Sod
6. Design Velocity (1 ft / s maximum)	$V_2 =$ 0.93 ft / s
7. Design Flow Depth (1 foot maximum) A) Flow Area B) Top Width of Swale C) Froude Number (0.50 maximum) D) Hydraulic Radius E) Velocity-Hydraulic Radius Product for Vegetal Retardance F) Manning's n (based on SCS vegetal retardance curve E for seeded grass) G) Cumulative Height of Grade Control Structures Required	$D_2 =$ 0.58 ft $A_2 =$ 1.9 sq ft $W_T =$ 5.6 ft $F =$ 0.28 $R_H =$ 0.33 $VR =$ 0.31 $n =$ 0.054 $H_D =$ 0.00 ft
8. Underdrain (Is an underdrain necessary?)	Choose One ☒ YES ☐ NO AN UNDERDRAIN IS REQUIRED IF THE DESIGN SLOPE < 2.0%
9. Soil Preparation (Describe soil amendment)	
10. Irrigation	Choose One ☐ Temporary ☐ Permanent

Notes:

Design Procedure Form: Grass Buffer (GB)

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 1

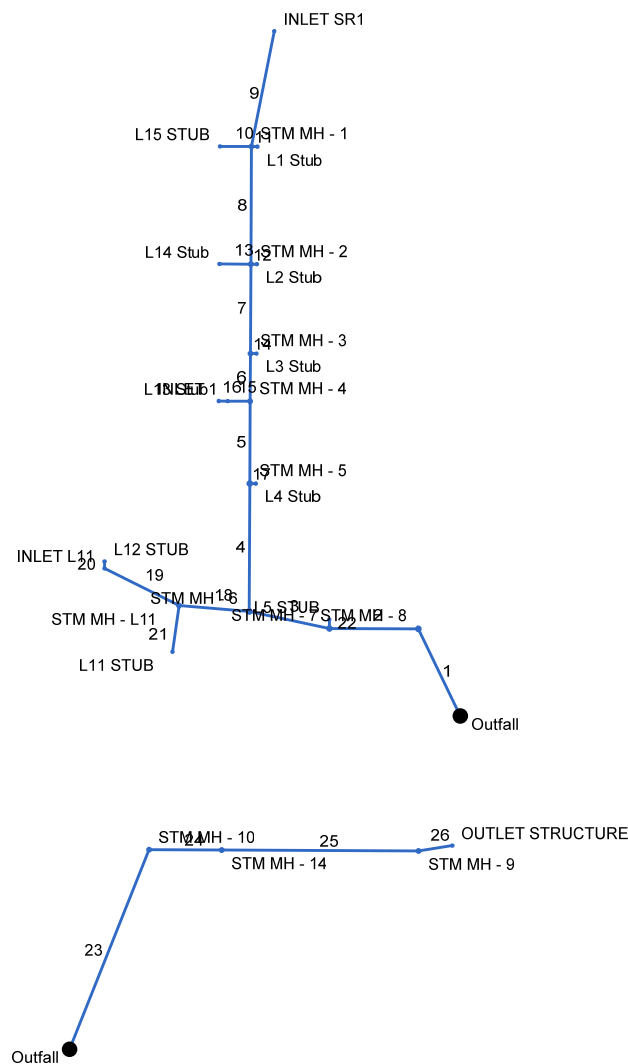
Designer: Jerry Davidson
Company: Perception Design Group, Inc.
Date: June 25, 2018
Project: Parker Pointe
Location: New Left Turn Lane on Parker Road - Basin P3

1. Design Discharge A) 2-Year Peak Flow Rate of the Area Draining to the Grass Buffer	$Q_2 = $ 0.7 cfs
2. Minimum Width of Grass Buffer	$W_G = $ 13 ft
3. Length of Grass Buffer (14' or greater recommended)	$L_G = $ 800 ft
4. Buffer Slope (in the direction of flow, not to exceed 0.1 ft / ft)	$S_G = $ 0.020 ft / ft
5. Flow Characteristics (sheet or concentrated) A) Does runoff flow into the grass buffer across the entire width of the buffer? B) Watershed Flow Length C) Interface Slope (normal to flow) D) Type of Flow Sheet Flow: $F_L * S_i \leq 1$ Concentrated Flow: $F_L * S_i > 1$	Choose One ☒ Yes ☐ No $F_L = $ 800 ft $S_i = $ 0.001 ft / ft SHEET FLOW
6. Flow Distribution for Concentrated Flows	Choose One ☐ None (sheet flow) ☐ Slotted Curbing ☐ Level Spreader ☐ Other (Explain):
7 Soil Preparation (Describe soil amendment)	Existing Buffer
8 Vegetation (Check the type used or describe "Other")	Choose One ☐ Existing Xeric Turf Grass ☒ Irrigated Turf Grass ☐ Other (Explain):
9. Irrigation (*Select None if existing buffer area has 80% vegetation AND will not be disturbed during construction.)	Choose One ☐ Temporary ☒ Permanent ☐ None*
10. Outflow Collection (Check the type used or describe "Other")	Choose One ☐ Grass Swale ☐ Street Gutter ☐ Storm Sewer Inlet ☐ Other (Explain):
Notes:	

**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

APPENDIX C HYDRAULIC CALCULATIONS

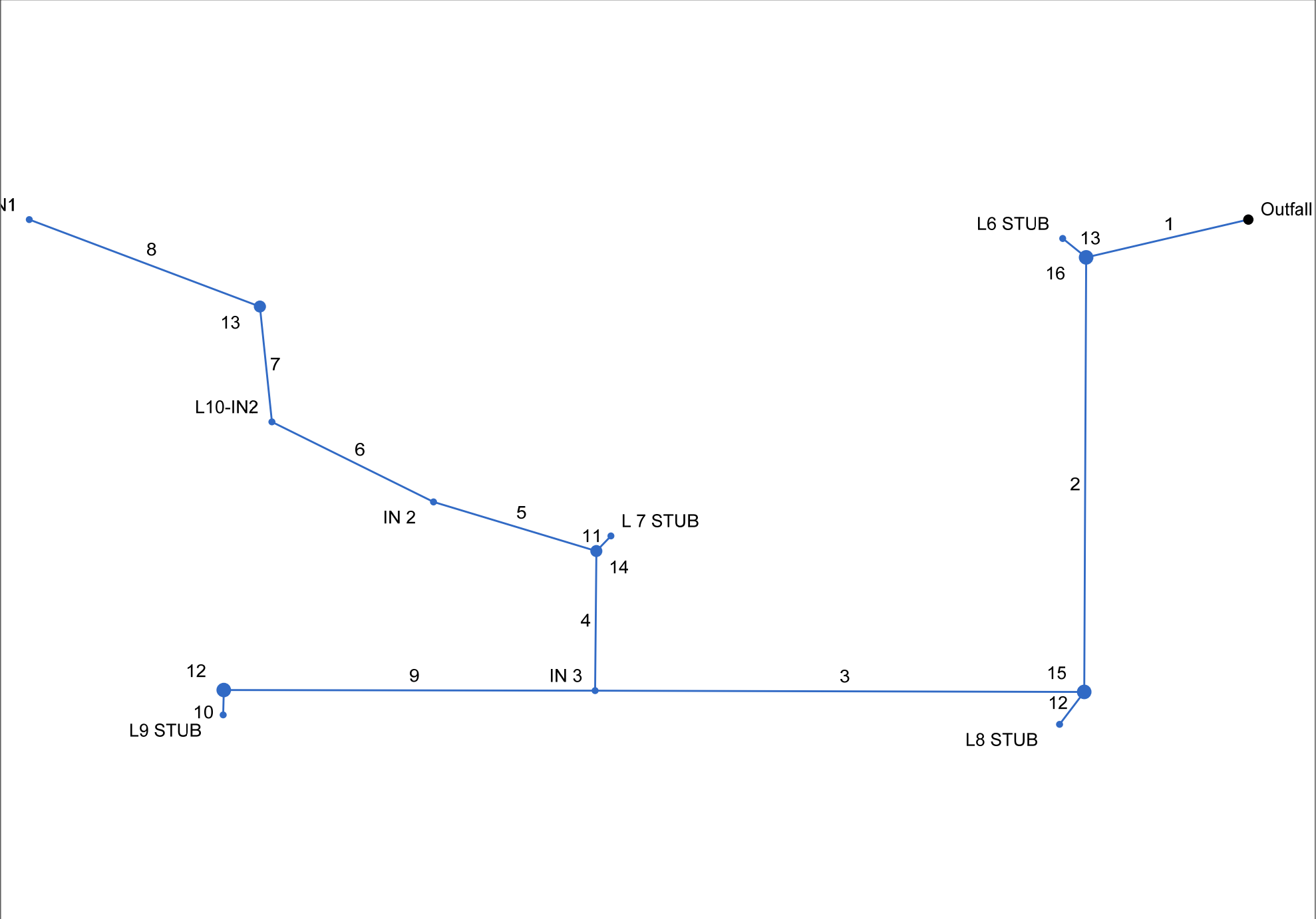
Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Project File: Storm Line A 100 yr.stm	Number of lines: 26	Date: 3/1/2018
---------------------------------------	---------------------	----------------

Line No.	Line Size	Flow Rate	Vel Ave	Depth Up	Depth Dn	HGL Up	HGL Dn	Line ID
	(in)	(cfs)	(ft/s)	(ft)	(ft)	(ft)	(ft)	
1	42	61.03	8.31	2.49	2.50	5967.48	5966.84	P-22
2	42	61.03	6.74	2.99	3.28	5968.77	5968.47	P-21
3	42	56.12	5.91	3.29	3.50	5969.82	5969.53	P-19
4	42	44.60	4.83	3.04	3.50	5970.63	5970.36	P-14
5	42	39.69	5.72	1.96**	3.23	5970.30	5971.02	P-12
6	36	31.79	7.11	1.83**	1.80	5970.99	5970.64	P-9
7	30	28.44	6.73	2.01	2.01	5972.27	5971.67	P-7
8	30	18.86	4.18	1.98	2.50	5973.23	5972.97	P-4
9	21	7.72	4.34	1.03**	1.55	5974.21 j	5973.55	P-1
10	18	5.61	3.17	1.50	1.50	5973.67	5973.55	P-2
11	18	5.53	3.13	1.50	1.50	5973.57	5973.55	P-3
12	18	3.89	2.20	1.50	1.50	5972.98	5972.97	P-6
13	18	5.69	3.22	1.50	1.50	5973.10	5972.97	P-5
14	18	3.35	3.40	0.70**	1.02	5970.83	5970.99	P-8
15	18	7.90	4.73	1.35	1.35	5970.64	5970.49	P-11
16	18	5.69	3.82	1.16	1.20	5970.71	5970.69	P-10
17	18	4.91	2.78	1.50	1.50	5971.04	5971.02	P-13
18	24	11.52	3.67	2.00	2.00	5970.61	5970.36	P-18
19	18	4.36	2.47	1.50	1.50	5971.01	5970.82	P-16
20	18	4.36	2.47	1.50	1.50	5971.11	5971.09	P-15
21	18	7.16	4.05	1.50	1.50	5971.11	5970.82	P-17
22	18	4.91	2.78	1.50	1.50	5969.55	5969.53	P-20
23	36	36.00	7.34	1.98	1.95	5961.42	5959.95	STM PIPE - 37
24	36	36.00	6.00	2.23	2.55	5962.36	5962.19	STM PIPE - 38
25	36	36.00	7.06	1.95**	2.13	5963.59 j	5962.45	STM PIPE - 39
26	36	36.00	6.66	2.14	2.15	5964.17	5963.99	STM PIPE - 40

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Line No.	Line Size	Flow Rate	Vel Ave	Depth Up	Depth Dn	HGL Up	HGL Dn	Line ID
	(in)	(cfs)	(ft/s)	(ft)	(ft)	(ft)	(ft)	
1	36	35.96	7.35	1.96	1.96	5966.63	5966.30	Pipe - (6)
2	36	29.89	5.75	1.77**	2.58	5967.50	5967.45	Pipe - (5)
3	36	23.11	6.23	1.55**	1.57	5968.44	5967.50	Pipe - (4)
4	24	16.65	5.78	1.72	1.72	5969.89	5969.62	Pipe - (10)
5	24	11.35	3.69	1.84	2.00	5970.54	5970.40	Pipe - (14)
6	18	6.85	4.18	1.26	1.38	5970.82	5970.58	Pipe - (15)
7	18	6.85	4.28	1.24	1.31	5971.23	5971.07	Pipe - (16)
8	18	6.85	4.40	1.17	1.31	5971.86	5971.50	Pipe - (17)
9	18	5.53	4.61	0.96	0.96	5970.09	5969.35	Pipe - (3)
10	18	5.53	4.05	1.07	1.10	5970.44	5970.42	Pipe - (9)
11	18	5.30	3.00	1.50	1.50	5970.42	5970.40	Pipe - (18)
12	18	6.78	5.85	1.01**	0.88	5968.40	5968.11	Pipe - (11)
13	18	6.07	4.46	0.95**	1.28	5967.36	5967.45	Pipe - (12)

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(303) 232-8088 Fax (303) 232-5255

Designed by: JWD
Checked by: JWD
DATE 11/10/2017
JOB # 2015-015

Project: Parker Pointe

STORM SEWER DESIGN -- MANNING'S EQUATION

100-YEAR STORM

Contributing Basins	Pipe Location Design Points	Q req'd (cfs)	Slope (%)	n	Diam (in)	Q calc (cfs)	V (Full Flow) (fps)	Friction Slope (%)	Normal Depth (ft)
SR2 - 5 Year	Stroh Crossing	1.31	0.25	0.013	12	1.79	1.67	0.13	0.42
* SR2 - 100 Year	Stroh Crossing	3.51	0.25	0.013	12	1.79	4.47	0.97	0.80

* Flow to Stroh Crossing pipe = 2.66 cfs from basin SR2 plus 1/2 bypass from basin SR1 (1/2 of 1.7cfs) = 3.51 cfs

* Required capacity is 1/2 of 3.51 cfs or 1.75 cfs as 2 parallel pipes are provided.

INLET MANAGEMENT

Worksheet Protected

INLET NAME	IN 1	IN 2	IN 3	PR1	SR1
Site Type (Urban or Rural)					
Inlet Application (Street or Area)	STREET	STREET	STREET	STREET	STREET
Hydraulic Condition	On Grade	In Sump	On Grade	In Sump	On Grade
Inlet Type	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT/Denver 13 Combination	CDOT Type R Curb Opening	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows					
Minor Q_{known} (cfs)	1.1	2.2	0.5	3.3	1.8
Major Q_{known} (cfs)	2.2	4.5	0.9	6.0	7.7

Bypass (Carry-Over) Flow from Upstream

Receive Bypass Flow from:	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0	0.0	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0	0.1	0.0	0.0	0.0

Watershed Characteristics

Subcatchment Area (acres)					
Percent Impervious					
NRCS Soil Type					

Watershed Profile

Overland Slope (ft/ft)					
Overland Length (ft)					
Channel Slope (ft/ft)					
Channel Length (ft)					

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)					
One-Hour Precipitation, P_1 (inches)					

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)					
One-Hour Precipitation, P_1 (inches)					

CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	1.1	2.2	0.5	3.3	1.8
Major Total Design Peak Flow, Q (cfs)	2.2	4.6	0.9	6.0	7.7
Minor Flow Bypassed Downstream, Q_b (cfs)	0.0	N/A	0.0	N/A	0.0
Major Flow Bypassed Downstream, Q_b (cfs)	0.0	N/A	0.1	N/A	1.7

Minor Storm (Calculated) Analysis of Flow Time

C	N/A	N/A	N/A	N/A	N/A
C_s	N/A	N/A	N/A	N/A	N/A
Overland Flow Velocity, V_i	N/A	N/A	N/A	N/A	N/A
Channel Flow Velocity, V_t	N/A	N/A	N/A	N/A	N/A
Overland Flow Time, T_i	N/A	N/A	N/A	N/A	N/A
Channel Travel Time, T_t	N/A	N/A	N/A	N/A	N/A
Calculated Time of Concentration, T_c	N/A	N/A	N/A	N/A	N/A
Regional T_c	N/A	N/A	N/A	N/A	N/A
Recommended T_c	N/A	N/A	N/A	N/A	N/A
T_c selected by User	N/A	N/A	N/A	N/A	N/A
Design Rainfall Intensity, I	N/A	N/A	N/A	N/A	N/A
Calculated Local Peak Flow, Q_p	N/A	N/A	N/A	N/A	N/A

Major Storm (Calculated) Analysis of Flow Time

C	N/A	N/A	N/A	N/A	N/A
C_s	N/A	N/A	N/A	N/A	N/A
Overland Flow Velocity, V_i	N/A	N/A	N/A	N/A	N/A
Channel Flow Velocity, V_t	N/A	N/A	N/A	N/A	N/A
Overland Flow Time, T_i	N/A	N/A	N/A	N/A	N/A
Channel Travel Time, T_t	N/A	N/A	N/A	N/A	N/A
Calculated Time of Concentration, T_c	N/A	N/A	N/A	N/A	N/A
Regional T_c	N/A	N/A	N/A	N/A	N/A
Recommended T_c	N/A	N/A	N/A	N/A	N/A
T_c selected by User	N/A	N/A	N/A	N/A	N/A
Design Rainfall Intensity, I	N/A	N/A	N/A	N/A	N/A
Calculated Local Peak Flow, Q_p	N/A	N/A	N/A	N/A	N/A

INLET MANAGEMENT

Worksheet Protected

INLET NAME	SR2
Site Type (Urban or Rural)	
Inlet Application (Street or Area)	STREET
Hydraulic Condition	In Sump
Inlet Type	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows	
Minor Q_{Known} (cfs)	1.3
Major Q_{Known} (cfs)	2.7

Bypass (Carry-Over) Flow from Upstream

Receive Bypass Flow from:	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0
Major Bypass Flow Received, Q_b (cfs)	1.7

Watershed Characteristics

Subcatchment Area (acres)	
Percent Impervious	
NRCS Soil Type	

Watershed Profile

Overland Slope (ft/ft)	
Overland Length (ft)	
Channel Slope (ft/ft)	
Channel Length (ft)	

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)	
One-Hour Precipitation, P_1 (inches)	

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)	
One-Hour Precipitation, P_1 (inches)	

CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	1.3
Major Total Design Peak Flow, Q (cfs)	4.4
Minor Flow Bypassed Downstream, Q_b (cfs)	N/A
Major Flow Bypassed Downstream, Q_b (cfs)	N/A

Minor Storm (Calculated) Analysis of Flow Time

C	N/A
C_s	N/A
Overland Flow Velocity, V_i	N/A
Channel Flow Velocity, V_t	N/A
Overland Flow Time, T_i	N/A
Channel Travel Time, T_t	N/A
Calculated Time of Concentration, T_c	N/A
Regional T_c	N/A
Recommended T_c	N/A
T_c selected by User	N/A
Design Rainfall Intensity, I	N/A
Calculated Local Peak Flow, Q_p	N/A

Major Storm (Calculated) Analysis of Flow Time

C	N/A
C_s	N/A
Overland Flow Velocity, V_i	N/A
Channel Flow Velocity, V_t	N/A
Overland Flow Time, T_i	N/A
Channel Travel Time, T_t	N/A
Calculated Time of Concentration, T_c	N/A
Regional T_c	N/A
Recommended T_c	N/A
T_c selected by User	N/A
Design Rainfall Intensity, I	N/A
Calculated Local Peak Flow, Q_p	N/A

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

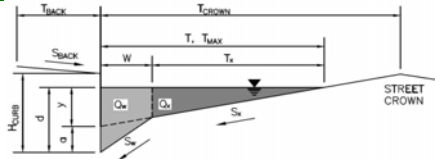
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Parker Pointe

Inlet ID:

IN 1

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line

Distance from Curb Face to Street Crown

Gutter Width

Street Transverse Slope

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

Street Longitudinal Slope - Enter 0 for sump condition

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $T_{BACK} =$ 0.0 ft $S_{BACK} =$ ft/ft $n_{BACK} =$ $H_{CURB} =$ 6.00 inches $T_{CROWN} =$ 26.0 ft $W =$ 2.00 ft $S_x =$ 0.020 ft/ft $S_w =$ 0.083 ft/ft $S_o =$ 0.005 ft/ft $n_{STREET} =$ 0.013

Max. Allowable Spread for Minor & Major Storm

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

	Minor Storm	Major Storm	
$T_{MAX} =$	26.0	26.0	ft
$d_{MAX} =$	6.0	6.0	inches

☐☐

check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion**MAJOR STORM Allowable Capacity is based on Depth Criterion**

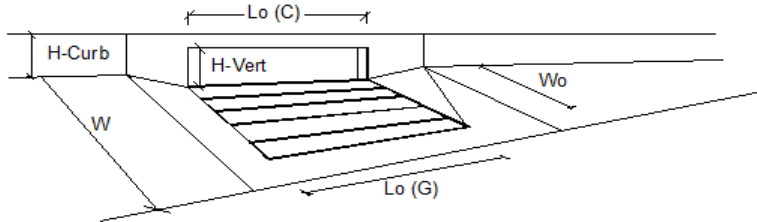
	Minor Storm	Major Storm	
$Q_{allow} =$	12.0	12.0	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR		MAJOR	
Type of Inlet	CDOT Type R Curb Opening	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')		a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)		No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)		L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)		W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		C_r-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		C_r-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity			MINOR	MAJOR	
Total Inlet Interception Capacity		Q =	1.1	2.2	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)		Q_b =	0.0	0.0	cfs
Capture Percentage = Q_a/Q_o =		C% =	100	100	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

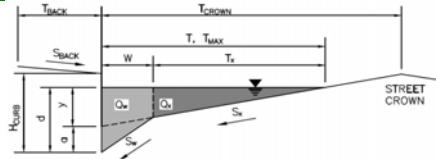
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Parker Pointe

Inlet ID:

IN 2

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line

Distance from Curb Face to Street Crown

Gutter Width

Street Transverse Slope

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

Street Longitudinal Slope - Enter 0 for sump condition

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

Max. Allowable Spread for Minor & Major Storm

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Check boxes are not applicable in SUMP conditions

MINOR STORM Allowable Capacity is based on Depth Criterion**MAJOR STORM Allowable Capacity is based on Depth Criterion** $T_{BACK} =$ 0.0 ft $S_{BACK} =$ ft/ft $n_{BACK} =$ $H_{CURB} =$ 6.00 inches $T_{CROWN} =$ 34.0 ft $W =$ 2.00 ft $S_x =$ 0.060 ft/ft $S_w =$ 0.083 ft/ft $S_o =$ 0.000 ft/ft $n_{STREET} =$ 0.013

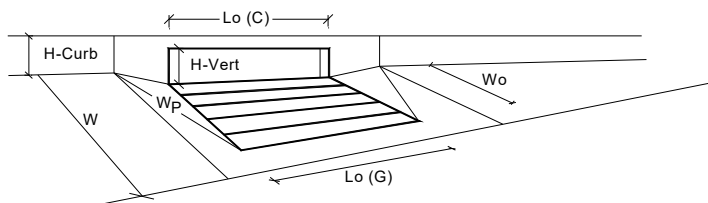
	Minor Storm	Major Storm	
$T_{MAX} =$	8.0	8.0	ft
$d_{MAX} =$	6.0	6.0	inches



	Minor Storm	Major Storm	
$Q_{allow} =$	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR		MAJOR		
Type of Inlet	CDOT Type R Curb Opening	CDOT Type R Curb Opening				
Local Depression (additional to continuous gutter depression 'a' from above)		a_{local}	3.00	3.00	inches	
Number of Unit Inlets (Grate or Curb Opening)		No	1	1		
Water Depth at Flowline (outside of local depression)		Ponding Depth	6.0	6.0	inches	
Grate Information		MINOR		MAJOR		☐ Override Depths
Length of a Unit Grate		$L_o (G)$	N/A	N/A	feet	
Width of a Unit Grate		W_o	N/A	N/A	feet	
Area Opening Ratio for a Grate (typical values 0.15-0.90)		A_{ratio}	N/A	N/A		
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)		$C_r (G)$	N/A	N/A		
Grate Weir Coefficient (typical value 2.15 - 3.60)		$C_w (G)$	N/A	N/A		
Grate Orifice Coefficient (typical value 0.60 - 0.80)		$C_o (G)$	N/A	N/A		
Curb Opening Information		MINOR		MAJOR		
Length of a Unit Curb Opening		$L_o (C)$	5.00	5.00	feet	
Height of Vertical Curb Opening in Inches		H_{vert}	6.00	6.00	inches	
Height of Curb Orifice Throat in Inches		H_{throat}	6.00	6.00	inches	
Angle of Throat (see USDCM Figure ST-5)		Theta	63.40	63.40	degrees	
Side Width for Depression Pan (typically the gutter width of 2 feet)		W_p	2.00	2.00	feet	
Clogging Factor for a Single Curb Opening (typical value 0.10)		$C_r (C)$	0.10	0.10		
Curb Opening Weir Coefficient (typical value 2.3-3.7)		$C_w (C)$	3.60	3.60		
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)		$C_o (C)$	0.67	0.67		
Low Head Performance Reduction (Calculated)		MINOR		MAJOR		
Depth for Grate Midwidth		d_{Grate}	N/A	N/A	ft	
Depth for Curb Opening Weir Equation		d_{Curb}	0.33	0.33	ft	
Combination Inlet Performance Reduction Factor for Long Inlets		$RF_{Combination}$	0.77	0.77		
Curb Opening Performance Reduction Factor for Long Inlets		RF_{Curb}	1.00	1.00		
Grated Inlet Performance Reduction Factor for Long Inlets		RF_{Grate}	N/A	N/A		
Total Inlet Interception Capacity (assumes clogged condition)		MINOR		MAJOR		
		Q_a	5.4	5.4	cfs	
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)		$Q_{PEAK REQUIRED}$	2.2	4.6	cfs	

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

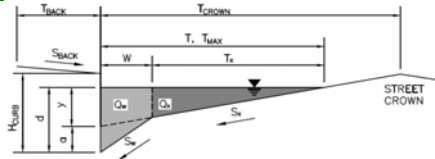
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Parker Pointe

Inlet ID:

IN 3

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line

Distance from Curb Face to Street Crown

Gutter Width

Street Transverse Slope

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

Street Longitudinal Slope - Enter 0 for sump condition

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $T_{BACK} = 0.0$ ft $S_{BACK} =$ ft/ft $n_{BACK} =$ $H_{CURB} = 6.00$ inches $T_{CROWN} = 26.0$ ft $W = 2.00$ ft $S_x = 0.020$ ft/ft $S_w = 0.083$ ft/ft $S_o = 0.005$ ft/ft $n_{STREET} = 0.013$

Max. Allowable Spread for Minor & Major Storm

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

	Minor Storm	Major Storm	
$T_{MAX} =$	26.0	26.0	ft
$d_{MAX} =$	6.0	6.0	inches

☐ ☐ check = yes
MINOR STORM Allowable Capacity is based on Depth Criterion**MAJOR STORM Allowable Capacity is based on Depth Criterion**

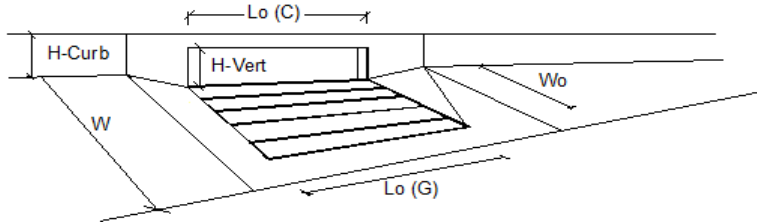
	Minor Storm	Major Storm	
$Q_{allow} =$	12.0	12.0	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)	CDOT/Denver 13 Combination	MINOR	MAJOR	
Type of Inlet	CDOT/Denver 13 Combination	Type =	CDOT/Denver 13 Combination	
Local Depression (additional to continuous gutter depression 'a')		a_{LOCAL} =	2.0	2.0 inches
Total Number of Units in the Inlet (Grate or Curb Opening)		N_o =	1	1
Length of a Single Unit Inlet (Grate or Curb Opening)		L_o =	3.00	3.00 ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)		W_o =	1.73	1.73 ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		C_r-G =	0.50	0.50
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		C_r-C =	0.10	0.10
Street Hydraulics: OK - Q < Allowable Street Capacity				
Total Inlet Interception Capacity		Q =	0.5	0.8 cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)		Q_b =	0.0	0.1 cfs
Capture Percentage = Q_a/Q_o =		C% =	100	86 %

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

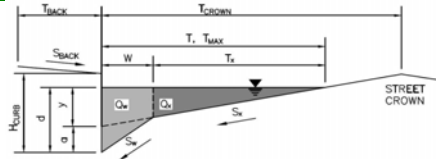
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Parker Pointe

Inlet ID:

PR1

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line

Distance from Curb Face to Street Crown

Gutter Width

Street Transverse Slope

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

Street Longitudinal Slope - Enter 0 for sump condition

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $T_{BACK} =$ 0.0 ft $S_{BACK} =$ ft/ft $n_{BACK} =$ $H_{CURB} =$ 6.00 inches $T_{CROWN} =$ 50.0 ft $W =$ 2.00 ft $S_x =$ 0.020 ft/ft $S_w =$ 0.083 ft/ft $S_o =$ 0.000 ft/ft $n_{STREET} =$ 0.013

Max. Allowable Spread for Minor & Major Storm

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Check boxes are not applicable in SUMP conditions

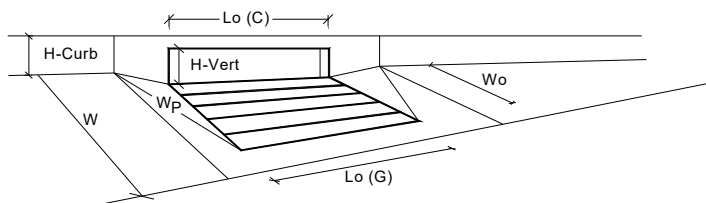
	Minor Storm	Major Storm	
$T_{MAX} =$	12.0	15.0	ft
$d_{MAX} =$	6.0	9.0	inches

**MINOR STORM Allowable Capacity is based on Depth Criterion****MAJOR STORM Allowable Capacity is based on Depth Criterion**

	Minor Storm	Major Storm	
$Q_{allow} =$	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR		MAJOR		
Type of Inlet	CDOT Type R Curb Opening	CDOT Type R Curb Opening				
Local Depression (additional to continuous gutter depression 'a' from above)		a _{local} =	3.00	3.00	inches	
Number of Unit Inlets (Grate or Curb Opening)		No =	2	2		
Water Depth at Flowline (outside of local depression)		Ponding Depth =	4.4	5.1	inches	
Grate Information		MINOR		MAJOR		☐ Override Depths
Length of a Unit Grate		L _o (G) =	N/A	N/A	feet	
Width of a Unit Grate		W _o =	N/A	N/A	feet	
Area Opening Ratio for a Grate (typical values 0.15-0.90)		A _{ratio} =	N/A	N/A		
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)		C _r (G) =	N/A	N/A		
Grate Weir Coefficient (typical value 2.15 - 3.60)		C _w (G) =	N/A	N/A		
Grate Orifice Coefficient (typical value 0.60 - 0.80)		C _o (G) =	N/A	N/A		
Curb Opening Information		MINOR		MAJOR		
Length of a Unit Curb Opening		L _o (C) =	5.00	5.00	feet	
Height of Vertical Curb Opening in Inches		H _{vert} =	6.00	6.00	inches	
Height of Curb Orifice Throat in Inches		H _{throat} =	6.00	6.00	inches	
Angle of Throat (see USDCM Figure ST-5)		Theta =	63.40	63.40	degrees	
Side Width for Depression Pan (typically the gutter width of 2 feet)		W _p =	2.00	2.00	feet	
Clogging Factor for a Single Curb Opening (typical value 0.10)		C _r (C) =	0.10	0.10		
Curb Opening Weir Coefficient (typical value 2.3-3.7)		C _w (C) =	3.60	3.60		
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)		C _o (C) =	0.67	0.67		
Low Head Performance Reduction (Calculated)		MINOR		MAJOR		
Depth for Grate Midwidth		d _{grate} =	N/A	N/A	ft	
Depth for Curb Opening Weir Equation		d _{curb} =	0.20	0.26	ft	
Combination Inlet Performance Reduction Factor for Long Inlets		RF _{Combination} =	0.41	0.48		
Curb Opening Performance Reduction Factor for Long Inlets		RF _{Curb} =	0.82	0.88		
Grated Inlet Performance Reduction Factor for Long Inlets		RF _{Grate} =	N/A	N/A		
Total Inlet Interception Capacity (assumes clogged condition)		MINOR		MAJOR		
		Q _a =	4.3	6.8	cfs	
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)		Q _{PEAK} REQUIRED =	3.3	6.0	cfs	

2 - 10' INLETS PROPOSED. CAPACITY IS TWICE WHAT IS INDICATED ABOVE
4.5 CFS OVERFLOW FROM 100 YR TOPS CURB AND FLOWS DIRECTLY TO KINNEY CREEK.

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

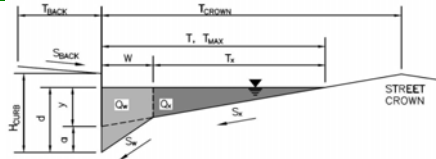
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Parker Pointe

Inlet ID:

SR1

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line

Distance from Curb Face to Street Crown

Gutter Width

Street Transverse Slope

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

Street Longitudinal Slope - Enter 0 for sump condition

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $T_{BACK} =$ 0.0 ft $S_{BACK} =$ ft/ft $n_{BACK} =$ $H_{CURB} =$ 6.00 inches $T_{CROWN} =$ 40.0 ft $W =$ 2.00 ft $S_x =$ 0.020 ft/ft $S_w =$ 0.083 ft/ft $S_o =$ 0.029 ft/ft $n_{STREET} =$ 0.013

Max. Allowable Spread for Minor & Major Storm

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

	Minor Storm	Major Storm	
$T_{MAX} =$	10.0	40.0	ft
$d_{MAX} =$	6.0	6.0	inches

☐☐

check = yes

MINOR STORM Allowable Capacity is based on Spread Criterion**MAJOR STORM Allowable Capacity is based on Depth Criterion**

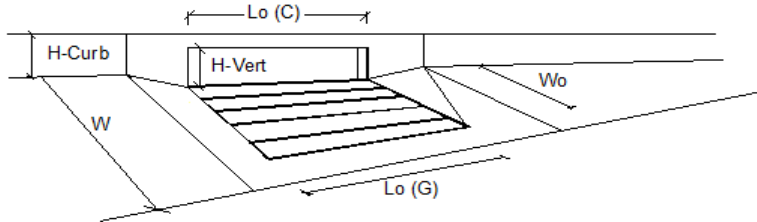
	Minor Storm	Major Storm	
$Q_{allow} =$	6.5	22.1	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)		MINOR		MAJOR	
Type of Inlet	CDOT Type R Curb Opening	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')		a _{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)		No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)		L _o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)		W _o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		C _r -G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		C _r -C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity			MINOR	MAJOR	
Total Inlet Interception Capacity		Q =	1.8	6.0	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)		Q _b =	0.0	1.7	cfs
Capture Percentage = Q _a /Q _o =		C% =	100	78	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

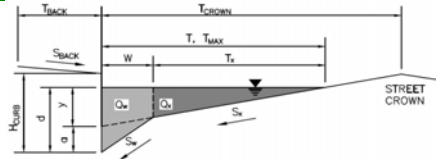
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Parker Pointe

Inlet ID:

SR2

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

Height of Curb at Gutter Flow Line

Distance from Curb Face to Street Crown

Gutter Width

Street Transverse Slope

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

Street Longitudinal Slope - Enter 0 for sump condition

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

Max. Allowable Spread for Minor & Major Storm

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Check boxes are not applicable in SUMP conditions

MINOR STORM Allowable Capacity is based on Depth Criterion**MAJOR STORM Allowable Capacity is based on Depth Criterion** $T_{BACK} =$ 0.0 ft $S_{BACK} =$ ft/ft $n_{BACK} =$ $H_{CURB} =$ 6.00 inches $T_{CROWN} =$ 14.0 ft $W =$ 2.00 ft $S_x =$ 0.028 ft/ft $S_w =$ 0.083 ft/ft $S_o =$ 0.000 ft/ft $n_{STREET} =$ 0.013

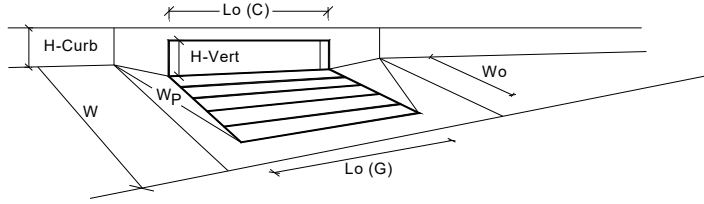
	Minor Storm	Major Storm	
$T_{MAX} =$	10.0	14.0	ft
$d_{MAX} =$	6.0	6.0	inches



	Minor Storm	Major Storm	
$Q_{allow} =$	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR		MAJOR		
Type of Inlet	CDOT Type R Curb Opening	Type =		CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)		a _{local} =		3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)		No =		2	2	
Water Depth at Flowline (outside of local depression)		Ponding Depth =		4.7	6.0	inches
Grate Information		MINOR		MAJOR		☐ Override Depths
Length of a Unit Grate		L _o (G) =		N/A	N/A	feet
Width of a Unit Grate		W _o =		N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)		A _{ratio} =		N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)		C _r (G) =		N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)		C _w (G) =		N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)		C _o (G) =		N/A	N/A	
Curb Opening Information		MINOR		MAJOR		
Length of a Unit Curb Opening		L _o (C) =		5.00	5.00	feet
Height of Vertical Curb Opening in Inches		H _{vert} =		6.00	6.00	inches
Height of Curb Orifice Throat in Inches		H _{throat} =		6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)		Theta =		63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)		W _p =		2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)		C _r (C) =		0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)		C _w (C) =		3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)		C _o (C) =		0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR		MAJOR		
Depth for Grate Midwidth		d _{Grate} =		N/A	N/A	ft
Depth for Curb Opening Weir Equation		d _{Curb} =		0.22	0.33	ft
Combination Inlet Performance Reduction Factor for Long Inlets		RF _{Combination} =		0.44	0.57	
Curb Opening Performance Reduction Factor for Long Inlets		RF _{Curb} =		0.85	0.93	
Grated Inlet Performance Reduction Factor for Long Inlets		RF _{Grate} =		N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)		MINOR		MAJOR		
		Q _a =		5.2	10.5	cfs
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)		Q _{PEAK} REQUIRED =		1.3	4.4	cfs

OUTFALL PIPE TO KINNEY CREEK

The open channel flow calculator					
Select Channel Type:	Circle ▼				
Depth from Q		Select unit system: Feet(ft) ▼			
Channel slope:	0.05 ft/ft	Water depth(y):	1.41 ft	Radius (r)	3 ft
Flow velocity:	7.15 ft/s	LeftSlope (Z1):	to 1 (H/V)	RightSlope (Z2):	to 1 (H/V)
Flow discharge:	36 ft³/s	Input n value	0.13 or select n	Status:	Calculation finished
<button>Calculate!</button>				Reset	
Wetted perimeter:	6.06 ft	Flow area:	5.05 ft²	Top width(T):	5.08 ft
Specific energy:	2.2 ft	Froude number:	1.26	Flow status:	Supercritical flow
Critical depth:	1.59 ft	Critical slope:	0.0031 ft/ft	Velocity head:	0.79 ft

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3.2.2 Low Tailwater Basin

The design of low tailwater riprap basins is necessary when the receiving channel may have little or no flow or tailwater at time when the pipe or culvert is in operation. Figure 9-37 provides a plan and profile view of a typical low tailwater riprap basin.

By providing a low tailwater basin at the end of a storm drain conduit or culvert, the kinetic energy of the discharge dissipates under controlled conditions without causing scour at the channel bottom.

Low tailwater is defined as being equal to or less than $\frac{1}{3}$ of the height of the storm drain, that is:

$$y_t \leq \frac{D}{3} \quad \text{or} \quad y_t \leq \frac{H}{3}$$

Low tailwater applies
Flow in stream is minimal

Where:

y_t = tailwater depth at design flow (feet)

D = diameter of circular pipe (feet)

H = height of rectangular pipe (feet)

Rock Size

The procedure for determining the required riprap size downstream of a conduit outlet is in Section 3.2.3.

After selecting the riprap size, the minimum thickness of the riprap layer, T , in feet, in the basin is defined as:

$$T = 2D_{50} \quad D_{50} = 12" \quad T = 2' \quad \text{Equation 9-15}$$

Basin Geometry

Figure 9-37 includes a layout of a standard low tailwater riprap basin with the geometry parameters provided. The minimum length of the basin (L) and the width of the bottom of the basin (W_1) are provided in a table at the bottom of Figure 9-37. All slopes in the low tailwater basin shall be 3(H):1(V), minimum.

Other Design Requirements

Extend riprap up the outlet embankment slope to the mid-pipe level, minimum. It is recommended that riprap that extends more than 1 foot above the outlet pipe invert be installed 6 inches below finished grade and buried with topsoil.

Provide pipe end treatment in the form of a pipe headwall or a flared-end section headwall. See Section 3.1 for options.

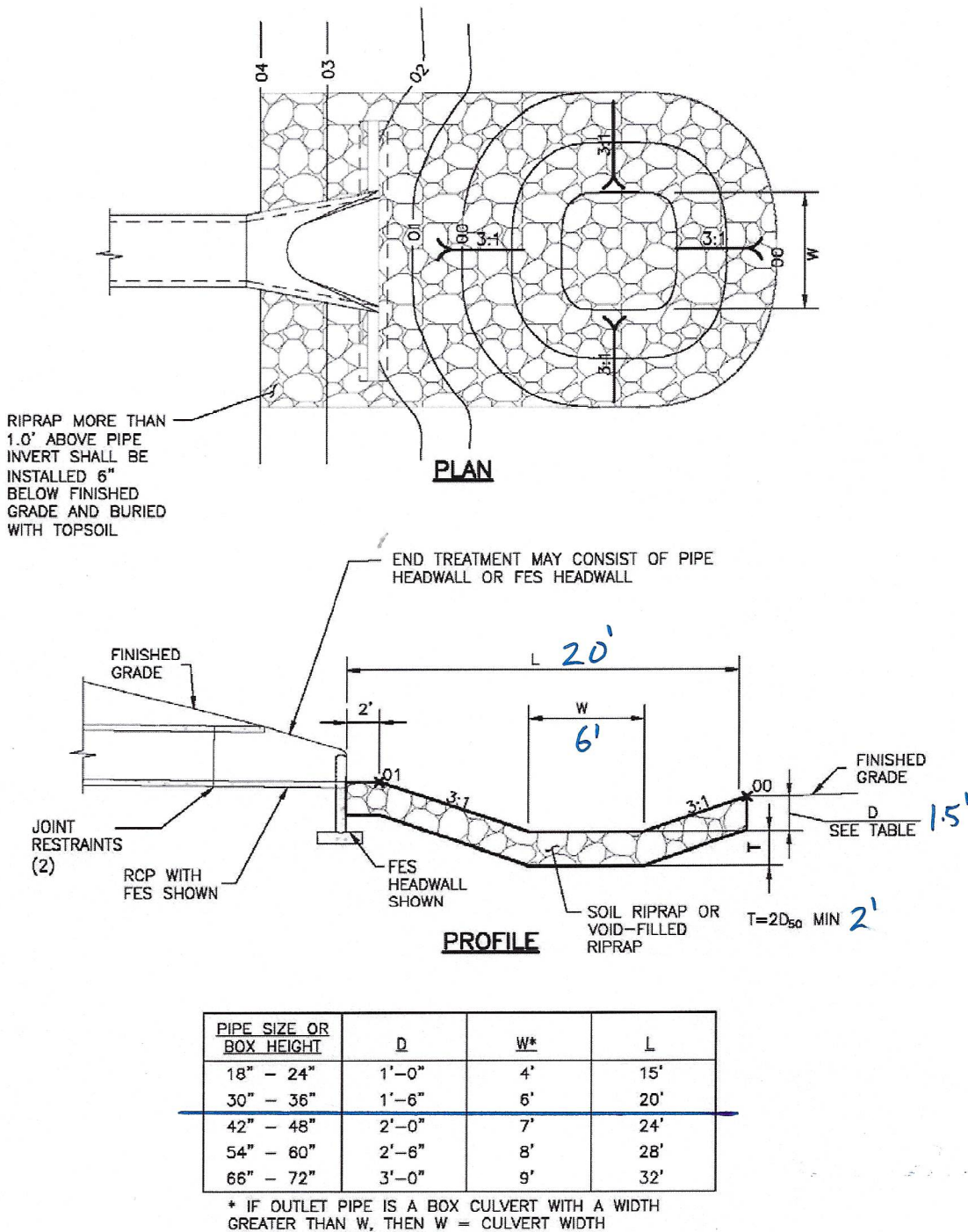


Figure 9-37. Low tailwater riprap basin

3.2.3 Rock Sizing for Riprap Apron and Low Tailwater Basin

Scour resulting from highly turbulent, rapidly decelerating flow is a common problem at conduit outlets. The following section summarizes the method for sizing riprap protection for both riprap aprons (Section 3.2.1) and low tailwater basins (Section 3.2.2).

Use Figure 9-38 to determine the required rock size for circular conduits and Figure 9-39 for rectangular conduits. Figure 9-38 is valid for $Q/D_c^{2.5}$ of 6.0 or less and Figure 9-39 is valid for $Q/WH^{1.5}$ of 8.0 or less. The parameters in these two figures are:

1. $Q/D^{1.5}$ or $Q/WH^{0.5}$ in which Q is the design discharge in cfs, D_c is the diameter of a circular conduit in feet, and W and H are the width and height of a rectangular conduit in feet.
2. Y_t/D_c or Y_t/H in which Y_t is the tailwater depth in feet, D_c is the diameter of a circular conduit in feet, and H is the height of a rectangular conduit in feet. In cases where Y_t is unknown or a hydraulic jump is suspected downstream of the outlet, use $Y_t/D_c = Y_t/H = 0.40$ when using Figures 9-38 and 9-39.
3. The riprap size requirements in Figures 9-38 and 9-39 are based on the non-dimensional parametric Equations 9-16 and 9-17 (Steven, Simons, and Watts 1971 and Smith 1975).

Circular culvert:

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}} \quad \text{Equation 9-16}$$

Rectangular culvert:

$$d_{50} = \frac{0.014H^{0.5}Q}{Y_t W} \quad \text{Equation 9-17}$$

These rock size requirements assume that the flow in the culvert is subcritical. It is possible to use Equations 9-16 and 9-17 when the flow in the culvert is supercritical (and less than full) if the value of D_c or H is modified for use in Figures 9-38 and 9-39. Note that rock sizes referenced in these figures are defined in the *Open Channels* chapter. Whenever the flow is supercritical in the culvert, substitute D_a for D_c and H_a for H , in which D_a is defined as:

$$D_a = \frac{(D_c + Y_n)}{2} = \frac{3.0 + 1.41}{2} = 2.2 \quad \text{Equation 9-18}$$

Where the maximum value of D_a shall not exceed D_c , and

$$H_a = \frac{(H + Y_n)}{2} \quad \text{N/A}$$

Equation 9-19

Where the maximum value of H_a shall not exceed H , and:

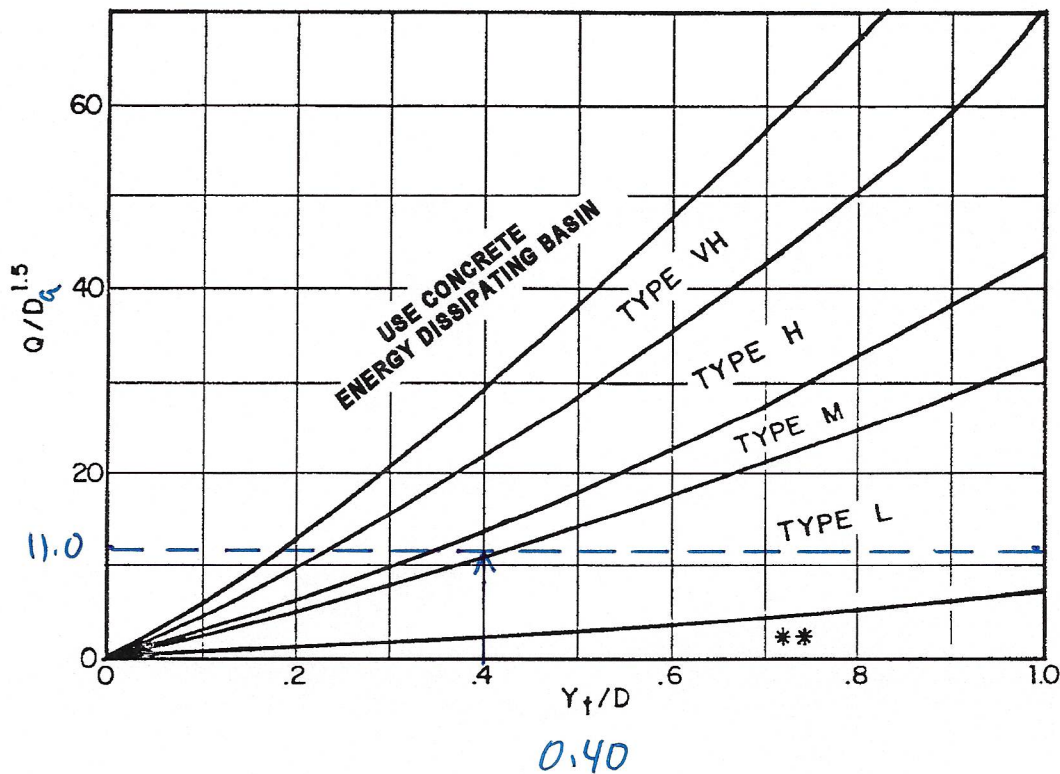
D_a = parameter to use in place of D in Figure 9-38 when flow is supercritical (ft) **2.2**

D_c = diameter of circular culvert (ft) **3.0'**

H_a = parameter to use in place of H in Figure 9-39 when flow is supercritical (ft) **N/A**

H = height of rectangular culvert (ft) **N/A**

Y_n = normal depth of supercritical flow in the culvert (ft) **1.41**



Use D_a instead of D whenever flow is supercritical in the barrel.
 ** Use Type L for a distance of $3D$ downstream.

use type M Riprap $D_{50} = 12"$

Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for $Q/D2.5 \leq 6.0$)

EAST PROPERTY LINE SWALE DESIGN

The open channel flow calculator

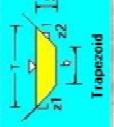
Select Channel Type: Trapezoid ▾

Depth from Q ▾

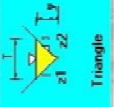
Select unit system: Feet(ft) ▾



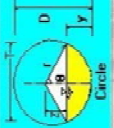
Rectangle



Trapezoid



Triangle



Circle

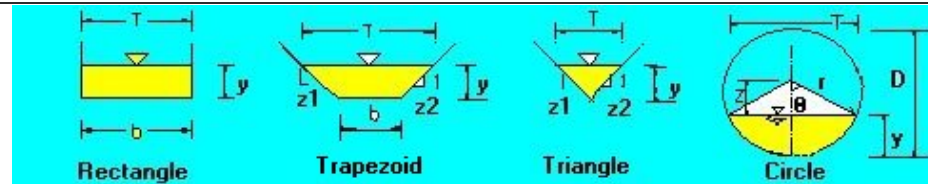
Channel slope: .01	ft/ft	Water depth(y): 0.92	ft	Bottom width(b): 6	ft
Flow velocity 2.789	ft/s	Left Slope (Z1): 4	to 1 (H:V)	Right Slope (Z2): 4	to 1 (H:V)
Flow discharge 24.7	ft ³ /s	Input n value .04	or select n		
Calculate		Status: Calculation finished		Reset	
Wetted perimeter 13.56	ft	Flow area 8.86	ft ²	Top width(T) 13.33	ft
Specific energy 1.04	ft	Froude number 0.6		Flow status: Subcritical flow	
Critical depth 0.69	ft	Critical slope 0.029	ft/ft	Velocity head 0.12	ft

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Offsite Tributary Area	5.7 Ac
Offsite C	0.36
I (assume 5 min Tc)	8.85
Q Offsite Area	18.2 cfs
Basin 2A	1.48 cfs
Basin 3A	1.25 cfs
Basin 4A	1.87 cfs
<u>Basin 5A</u>	<u>1.87 cfs</u>
Total Design Flow	24.7 cfs

The open channel flow calculator

Select Channel Type: Trapezoid ▼



Depth from Q ▼

Select unit system: Feet(ft) ▼

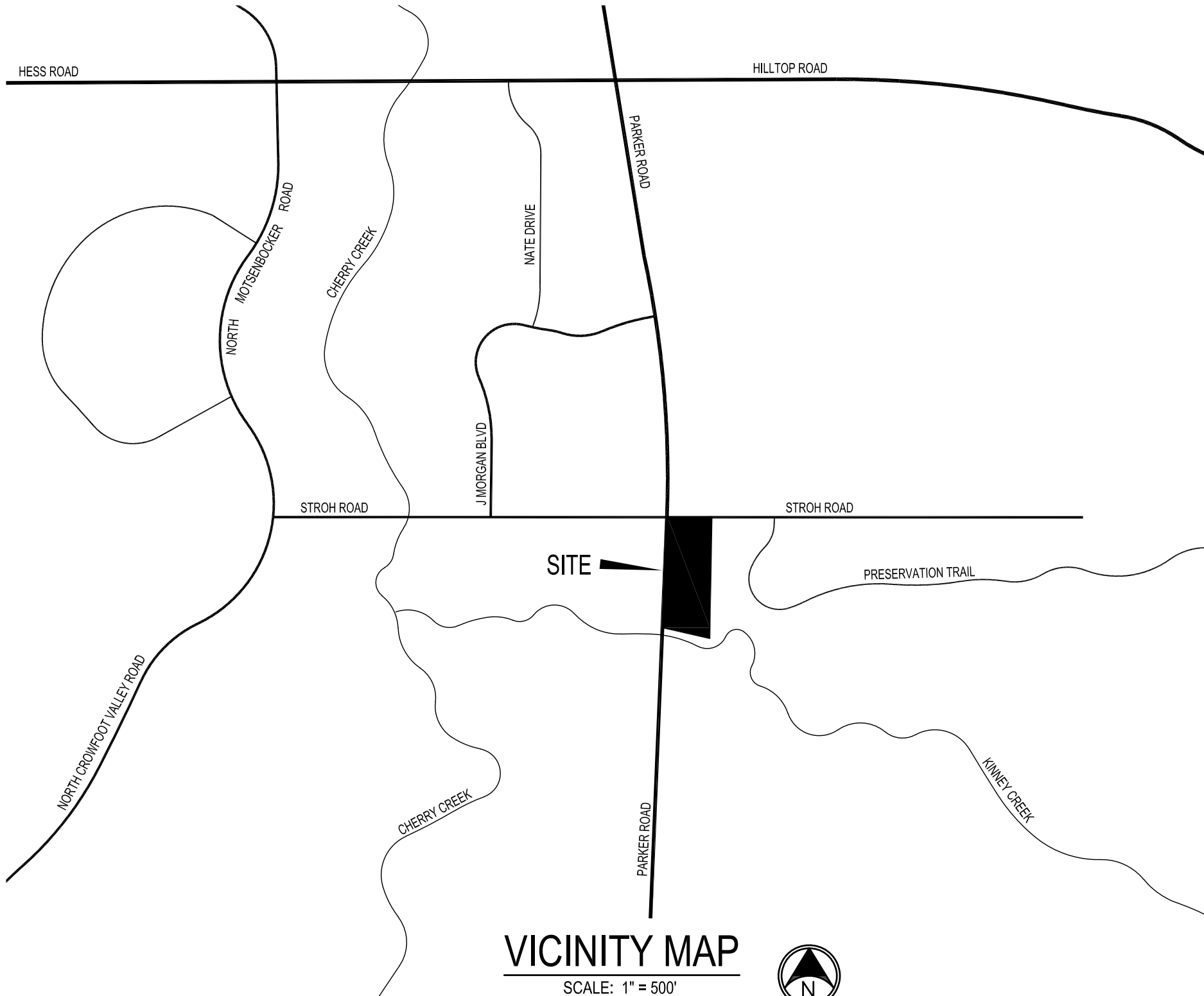
Channel slope: .0075 ft/ft	Water depth(y): 1.15 ft	Bottom width(b) 16 ft
Flow velocity 3.713 ft/s	LeftSlope (Z1): 4 to 1 (H:V)	RightSlope (Z2): 4 to 1 (H:V)
Flow discharge 88.1 ft ³ /s	Input n value .033 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 25.5 ft	Flow area 23.73 ft ²	Top width(T) 25.21 ft
Specific energy 1.37 ft	Froude number 0.67	Flow status Subcritical flow
Critical depth 0.91 ft	Critical slope 0.0175 ft/ft	Velocity head 0.21 ft

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EMERGENCY OVERFLOW CHANNEL FROM POND TO KINNEY CREEK

**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

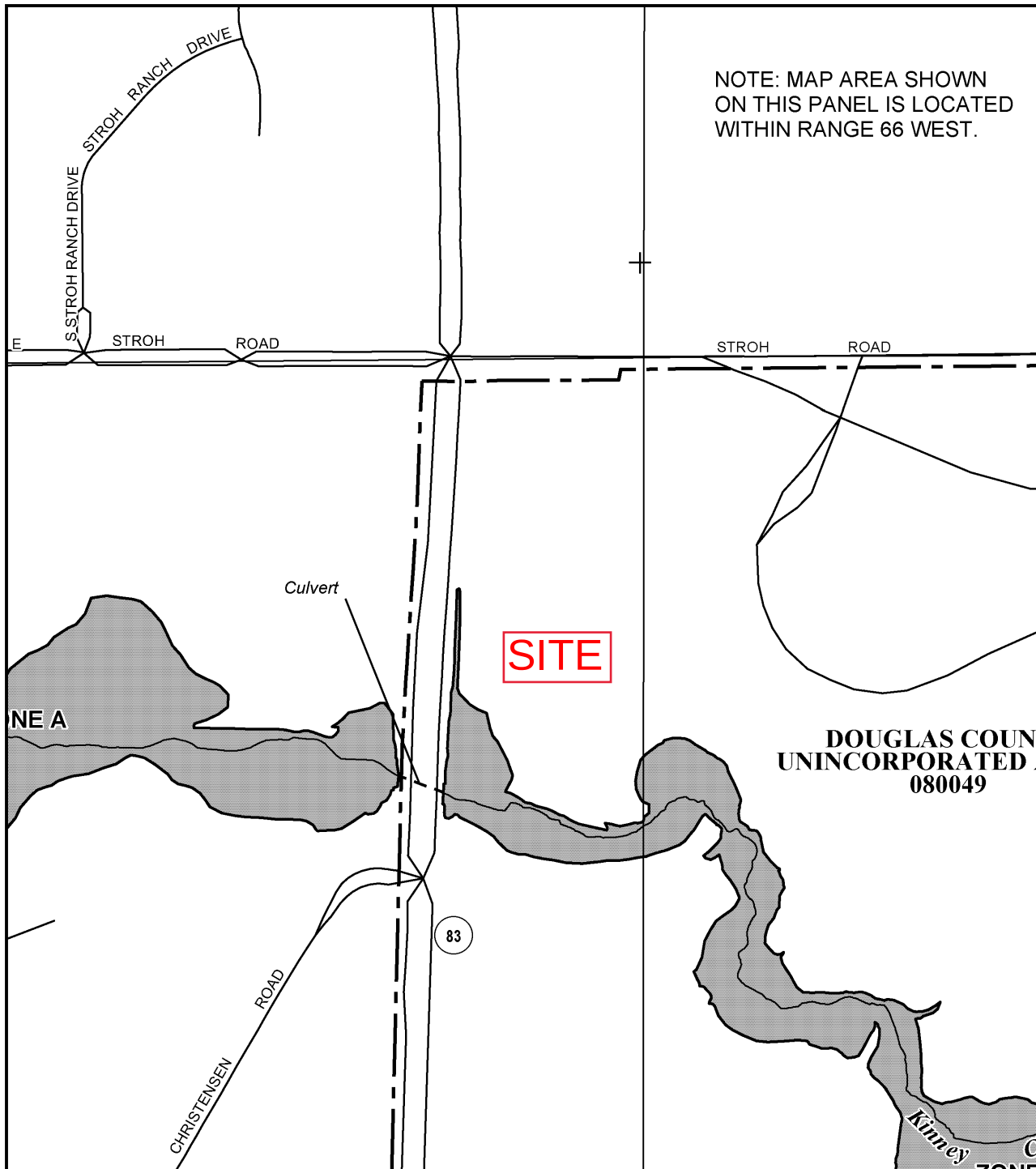
APPENDIX D KEYMAP, FIRM, SOILS



VICINITY MAP

SCALE: 1" = 500'

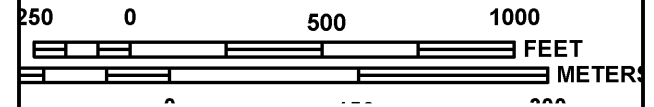




NOTE: MAP AREA SHOWN
ON THIS PANEL IS LOCATED
WITHIN RANGE 66 WEST.



MAP SCALE 1" = 500'



NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0182G

FIRM

**FLOOD INSURANCE RATE MAP
DOUGLAS COUNTY,
COLORADO
AND INCORPORATED AREAS**

PANEL 182 OF 495

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
DOUGLAS COUNTY	080049	0182	G
PARKER, TOWN OF	080310	0182	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.



**MAP NUMBER
08035C0182G
MAP REVISED
MARCH 16, 2016**

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov



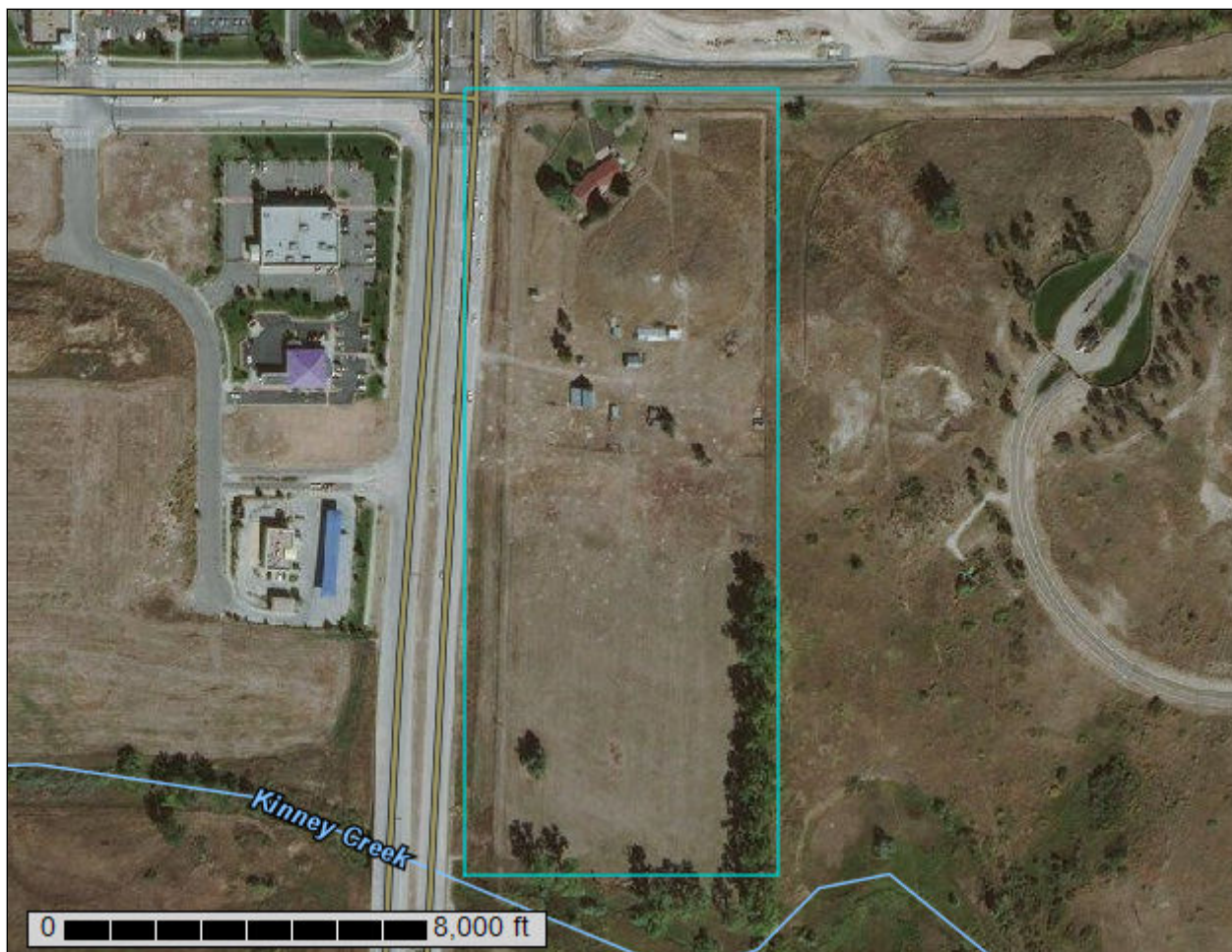
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for **Castle Rock Area, Colorado**



September 18, 2017

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



Map Scale: 1:2,470 if printed on A portrait (8.5" x 11") sheet.

0 35 70 140 210 Meters

0 100 200 400 600 Feet

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Castle Rock Area, Colorado
Survey Area Data: Version 9, Sep 22, 2016

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 17, 2015—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Castle Rock Area, Colorado (CO622)			
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BtE	Bresser-Truckton sandy loams, 5 to 25 percent slopes	6.1	34.7%
Lu	Loamy alluvial land, dark surface	2.8	16.0%
Sa	Sampson loam	8.7	49.1%
Sd	Sandy alluvial land	0.0	0.2%
Totals for Area of Interest		17.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate

Custom Soil Resource Report

pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Castle Rock Area, Colorado

BtE—Bresser-Truckton sandy loams, 5 to 25 percent slopes

Map Unit Setting

National map unit symbol: jqy9
Elevation: 5,500 to 6,600 feet
Mean annual precipitation: 15 to 19 inches
Mean annual air temperature: 47 to 52 degrees F
Frost-free period: 120 to 135 days
Farmland classification: Not prime farmland

Map Unit Composition

Bresser and similar soils: 50 percent
Truckton and similar soils: 35 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Bresser

Setting

Landform: Terraces
Landform position (three-dimensional): Tread, riser
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy eolian deposits

Typical profile

H1 - 0 to 8 inches: sandy loam
H2 - 8 to 30 inches: sandy clay loam
H3 - 30 to 60 inches: loamy sand

Properties and qualities

Slope: 5 to 15 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Moderate (about 7.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: Sandy Foothill (R049BY210CO)
Hydric soil rating: No

Description of Truckton

Setting

Landform: Terraces
Landform position (three-dimensional): Tread, riser

Custom Soil Resource Report

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Alluvium derived from arkosic sedimentary rock

Typical profile

H1 - 0 to 4 inches: sandy loam

H2 - 4 to 19 inches: sandy loam

H3 - 19 to 60 inches: sandy loam

Properties and qualities

Slope: 10 to 25 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water storage in profile: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: A

Hydric soil rating: No

Minor Components

Blakeland

Percent of map unit: 5 percent

Hydric soil rating: No

Newlin

Percent of map unit: 5 percent

Hydric soil rating: No

Stapleton

Percent of map unit: 4 percent

Hydric soil rating: No

Aquic haplustolls

Percent of map unit: 1 percent

Landform: Swales

Hydric soil rating: Yes

Lu—Loamy alluvial land, dark surface

Map Unit Setting

National map unit symbol: jqzc

Elevation: 7,000 to 8,000 feet

Custom Soil Resource Report

Mean annual precipitation: 17 to 19 inches
Mean annual air temperature: 44 to 46 degrees F
Frost-free period: 115 to 120 days
Farmland classification: Not prime farmland

Map Unit Composition

Loamy alluvial land, dark surface: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Loamy Alluvial Land, Dark Surface

Setting

Landform: Flood plains, swales
Down-slope shape: Linear
Across-slope shape: Linear

Typical profile

H1 - 0 to 20 inches: sandy loam
H2 - 20 to 40 inches: stratified loamy sand to clay loam
H3 - 40 to 60 inches: sand and gravel

Properties and qualities

Slope: 0 to 4 percent
Natural drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 6.00 in/hr)
Depth to water table: About 48 to 72 inches
Frequency of flooding: Frequent
Calcium carbonate, maximum in profile: 5 percent
Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)
Available water storage in profile: Moderate (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: C
Ecological site: Loamy Foothill 14-19 P.Z. (R049XC202CO)
Hydric soil rating: No

Minor Components

Sandy alluvial land

Percent of map unit: 14 percent
Hydric soil rating: No

Fluvaquentic haplustolls

Percent of map unit: 1 percent
Landform: Terraces
Hydric soil rating: Yes

Sa—Sampson loam

Map Unit Setting

National map unit symbol: jr02
Elevation: 5,500 to 6,600 feet
Mean annual precipitation: 15 to 19 inches
Mean annual air temperature: 48 to 50 degrees F
Frost-free period: 120 to 135 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Sampson and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Sampson

Setting

Landform: Stream terraces on drainageways
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Weathered alluvium derived from arkose

Typical profile

H1 - 0 to 9 inches: loam
H2 - 9 to 28 inches: clay loam
H3 - 28 to 38 inches: loam
H4 - 38 to 60 inches: silt loam

Properties and qualities

Slope: 1 to 4 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 9.5 inches)

Interpretive groups

Land capability classification (irrigated): 3e
Land capability classification (nonirrigated): 3c
Hydrologic Soil Group: B
Ecological site: Loamy Foothill 14-19 P.Z. (R049XC202CO)

Custom Soil Resource Report

Hydric soil rating: No

Minor Components

Englewood

Percent of map unit: 8 percent

Hydric soil rating: No

Bresser

Percent of map unit: 7 percent

Hydric soil rating: No

Loamy alluvial land

Percent of map unit: 4 percent

Hydric soil rating: No

Aquic haplustolls

Percent of map unit: 1 percent

Landform: Swales

Hydric soil rating: Yes

Sd—Sandy alluvial land

Map Unit Setting

National map unit symbol: jr03

Elevation: 5,500 to 6,600 feet

Mean annual precipitation: 15 to 19 inches

Mean annual air temperature: 48 to 50 degrees F

Frost-free period: 120 to 135 days

Farmland classification: Not prime farmland

Map Unit Composition

Sandy alluvial land: 75 percent

Minor components: 25 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Sandy Alluvial Land

Setting

Landform: Swales, drainageways

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Weathered alluvium derived from arkose

Typical profile

H1 - 0 to 20 inches: loamy sand

H2 - 20 to 60 inches: stratified sand to sandy loam

Properties and qualities

Slope: 1 to 5 percent

Natural drainage class: Excessively drained

Runoff class: Negligible

Custom Soil Resource Report

Capacity of the most limiting layer to transmit water (Ksat): High to very high (2.00 to 20.00 in/hr)

Depth to water table: About 60 inches

Frequency of flooding: Frequent

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Available water storage in profile: Low (about 4.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: A

Hydric soil rating: No

Minor Components

Loamy alluvial land

Percent of map unit: 8 percent

Hydric soil rating: No

Loamy alluvial land, dark surface

Percent of map unit: 8 percent

Hydric soil rating: No

Bresser

Percent of map unit: 4 percent

Hydric soil rating: No

Truckton

Percent of map unit: 4 percent

Hydric soil rating: No

Fluvaquent haplustolls

Percent of map unit: 1 percent

Landform: Terraces

Hydric soil rating: Yes

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

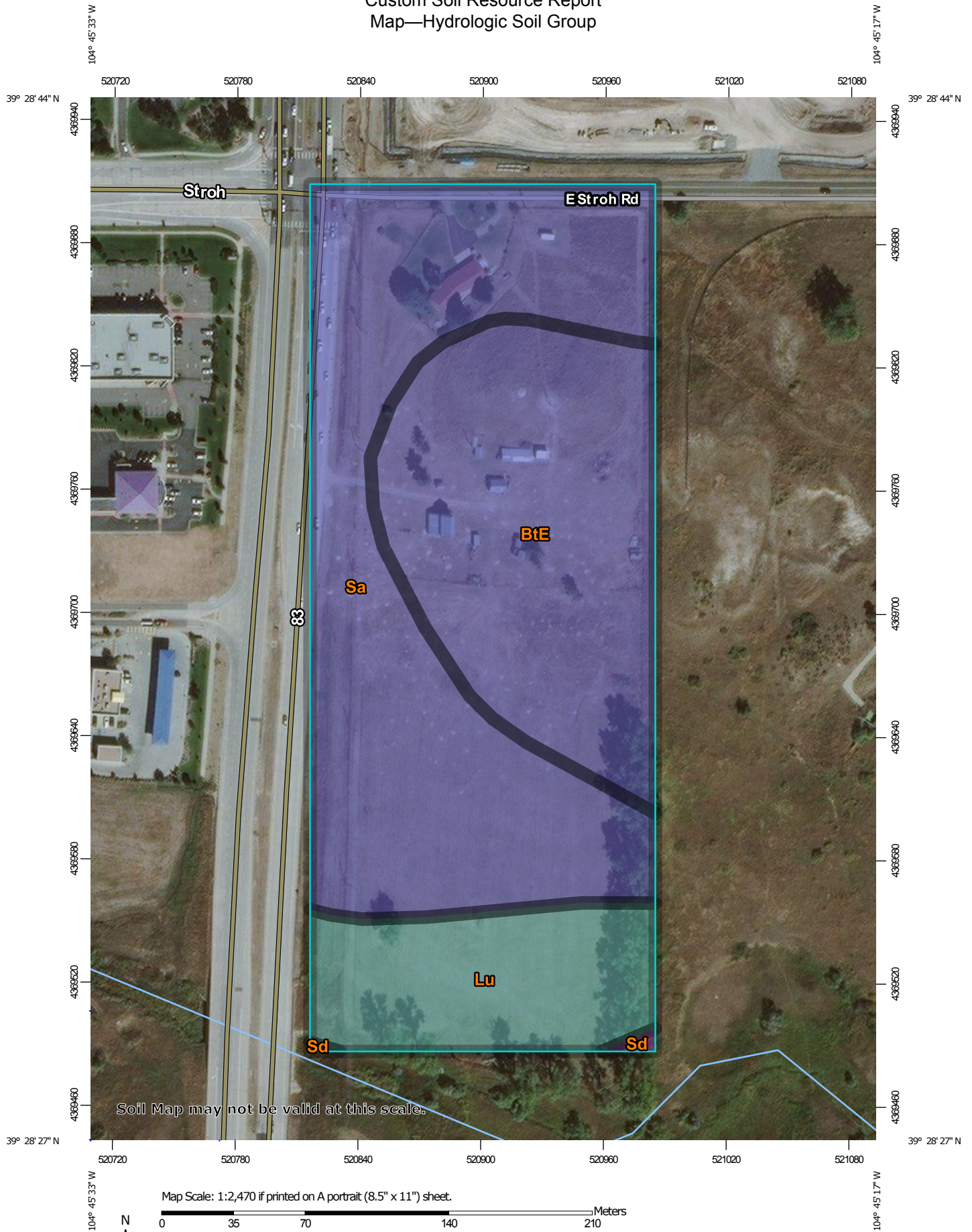
Custom Soil Resource Report

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Custom Soil Resource Report Map—Hydrologic Soil Group



Custom Soil Resource Report

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points

 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

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Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Castle Rock Area, Colorado
Survey Area Data: Version 9, Sep 22, 2016

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 17, 2015—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Castle Rock Area, Colorado (CO622)				
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
BtE	Bresser-Truckton sandy loams, 5 to 25 percent slopes	B	6.1	34.7%
Lu	Loamy alluvial land, dark surface	C	2.8	16.0%
Sa	Sampson loam	B	8.7	49.1%
Sd	Sandy alluvial land	A	0.0	0.2%
Totals for Area of Interest			17.7	100.0%

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

References

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Custom Soil Resource Report

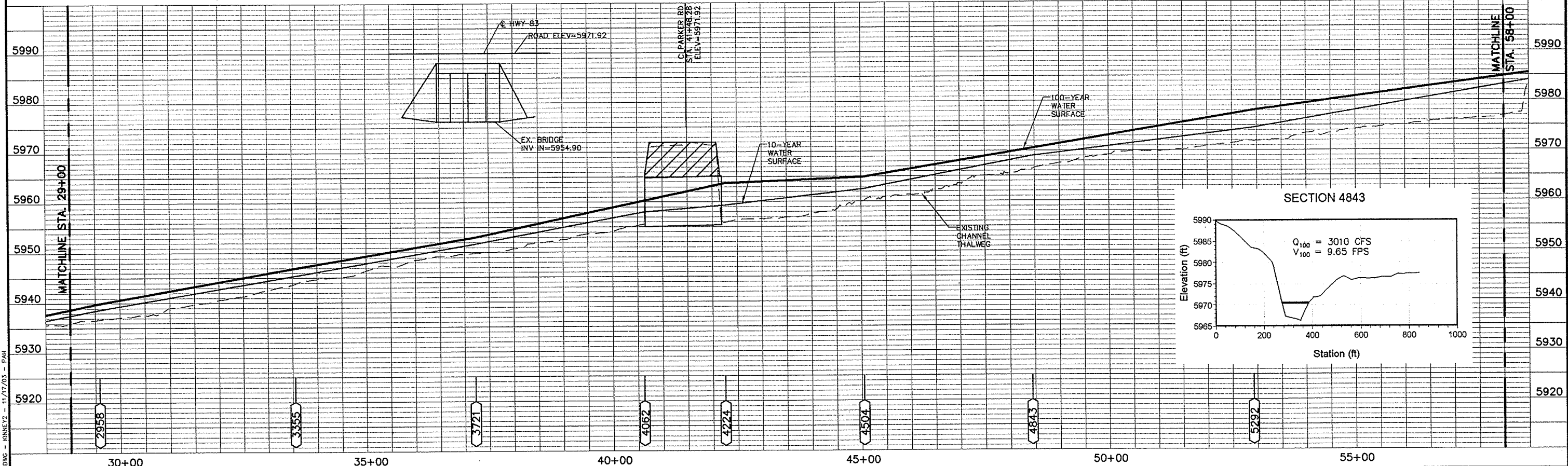
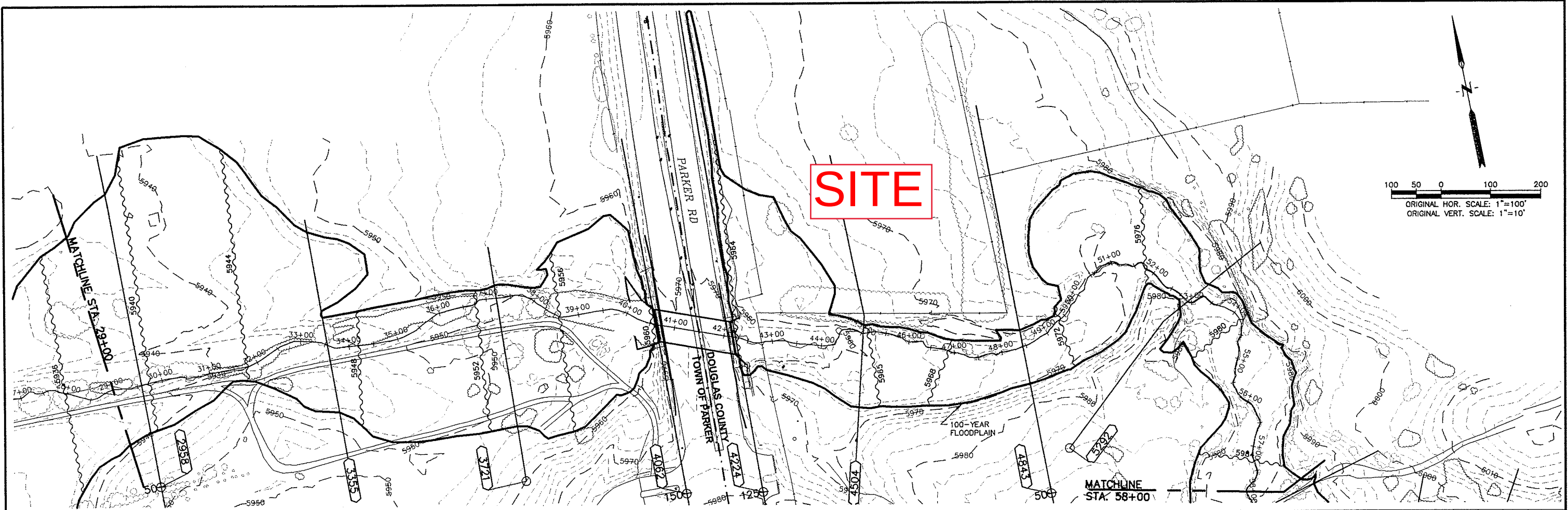
United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

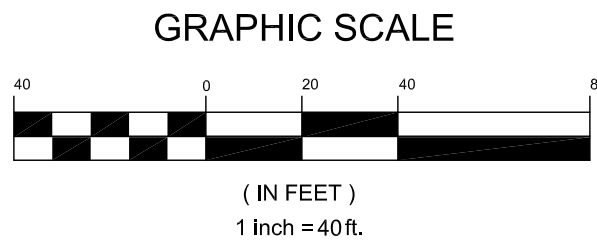
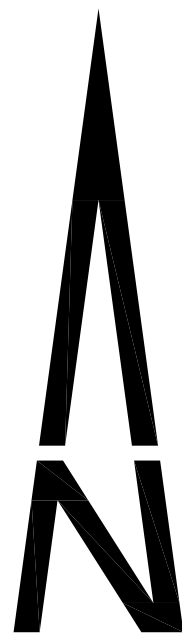
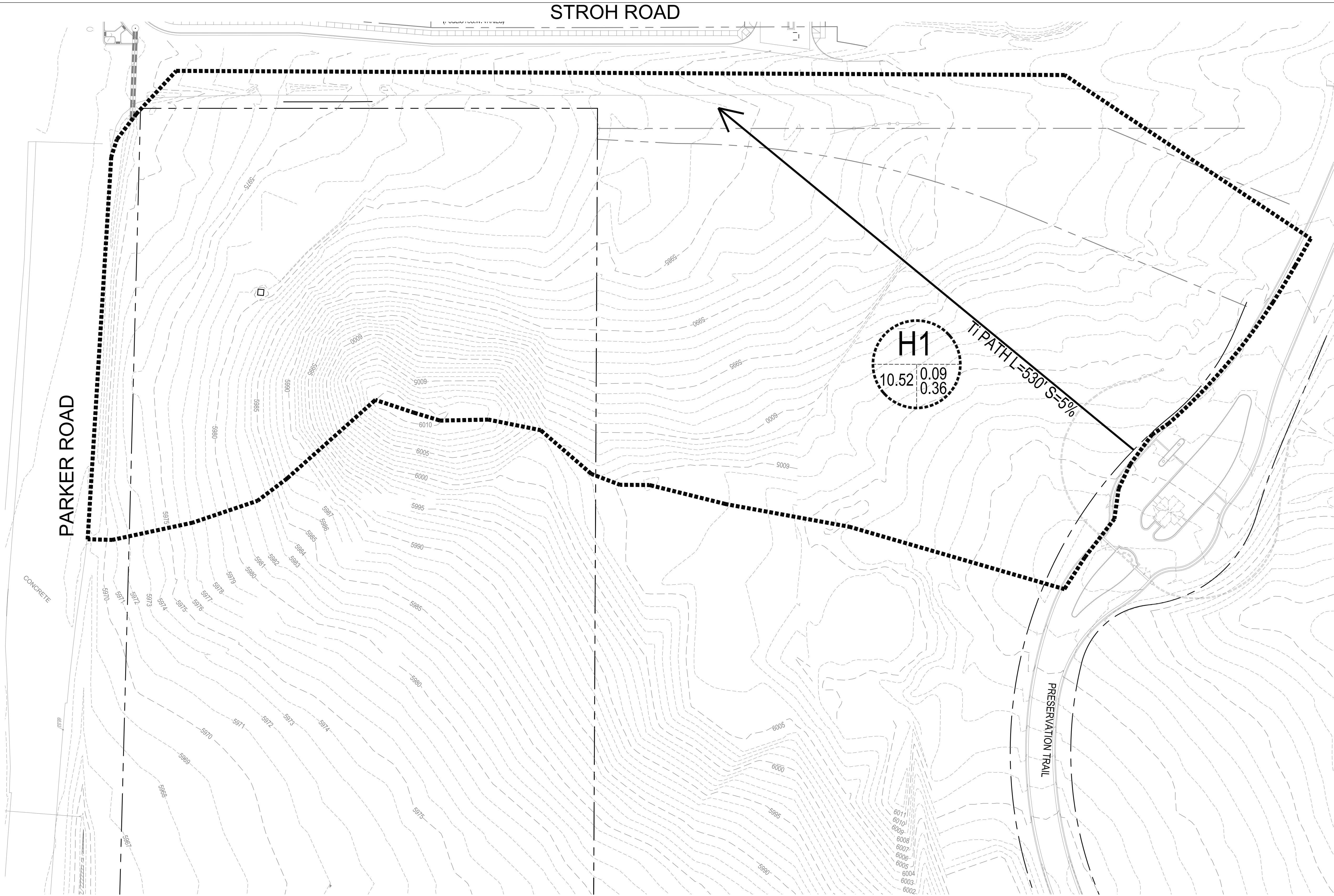
APPENDIX E DRAINAGE MAP



**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

APPENDIX E DRAINAGE MAP

\\PDG-NAS\PUBLIC\PROJECTS\2015-015 PARKER AND STROH\DWG\2015-015 DRAINAGE MAP.DWG 11/28/2018 10:26 AM



BENCHMARK

BENCHMARK: DOUGLAS COUNTY SURVEY CONTROL MONUMENT TT15A - 3" DIAMETER DOUGLAS COUNTY GIS ALUMINUM CAP AT THE NE CORNER OF STROH ROAD AND SOUTH PARKER ROAD (US HIGHWAY 63)
ELEVATION: 5970.79 FEET (NAVD 1988 DATUM)

THE TOWN OF PARKER REVIEW CONSTITUTES GENERAL COMPLIANCE WITH THE TOWN'S STANDARDS AND APPROVED VARIANCES, SUBJECT TO THESE PLANS BEING STAMPED, SIGNED, AND DATED BY THE PROFESSIONAL ENGINEER OF RECORD. REVIEW BY THE TOWN DOES NOT CONSTITUTE APPROVAL OF THE PLAN DESIGN OR ACCURACY AND CORRECTNESS OF ENGINEERING CALCULATIONS. ERRORS IN THE DESIGN OR CALCULATIONS REMAIN THE RESPONSIBILITY OF THE REGISTERED PROFESSIONAL ENGINEER WHOSE STAMP AND SIGNATURE ARE AFFIXED TO THIS DOCUMENT.

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TOWN OF PARKER, DIRECTOR OF ENGINEERING

DATE

HISTORIC DRAINAGE PLAN

LOT 1 THRU 15 AND TRACTS A AND B, PARKER POINTE FILING NO. 1
SOUTHEAST CORNER PARKER ROAD AND STROH ROAD
PARKER, COLORADO

design by: JWD
approved by: JWD
project no.: 2015-015

date: 10/01/17

SHEET

DP1

PREPARED UNDER THE DIRECT SUPERVISION OF
JERRY W. DAVIDSON, P.E.
COLORADO REG # 30226
FOR AND ON BEHALF OF
PERCEPTION DESIGN GROUP, INC.

NO.	DATE	DESCRIPTION
11/01/18	SIXTH SUBMITTAL	
08/31/18	FOURTH SUBMITTAL	
05/25/18	THIRD SUBMITTAL	
03/19/18	PWSD SUBMITTAL	
02/28/18	SECOND SUBMITTAL	
10/24/17	INITIAL SUBMITTAL	

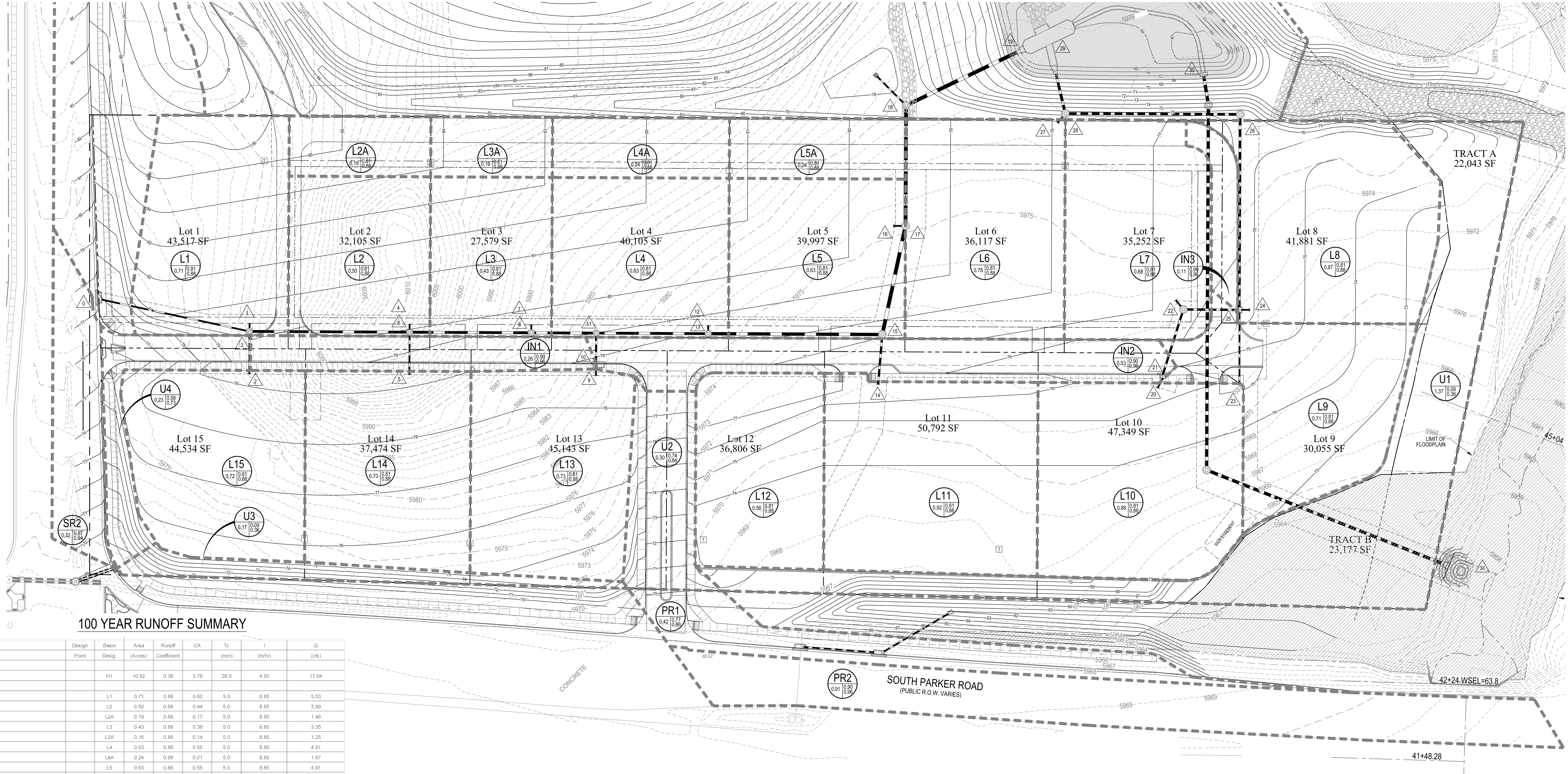
REVISIONS



\\PDG-NAS\PUBLIC\PROJECTS\2015-015 PARKER AND STROH\DWG\2015-015 DRAINAGE MAP.DWG 11/28/2018 10:26 AM

STROH ROAD

SEE SHEET DP3



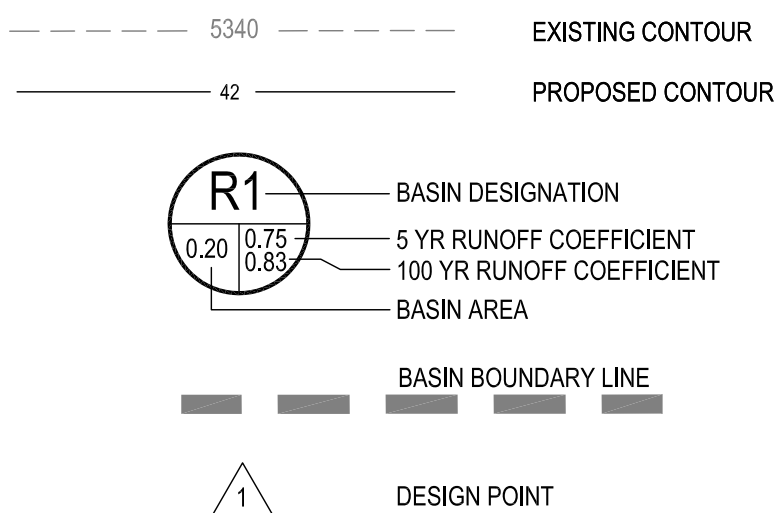
100 YEAR RUNOFF SUMMARY

Design Point	Basin Desig.	Area (Acres)	Runoff Coefficient	CA	Tc (min)	I (in/hr)	Q (cfs)
H1		10.52	0.36	3.79	26.0	4.50	17.04
L1		0.71	0.88	0.62	5.0	8.85	5.53
L2		0.50	0.88	0.44	5.0	8.85	3.89
L2A		0.19	0.88	0.17	5.0	8.85	1.48
L3		0.43	0.88	0.38	5.0	8.85	3.35
L3A		0.16	0.88	0.14	5.0	8.85	1.25
L4		0.63	0.88	0.55	5.0	8.85	4.91
L4A		0.24	0.88	0.21	5.0	8.85	1.87
L5		0.63	0.88	0.55	5.0	8.85	4.91
L5A		0.24	0.88	0.21	5.0	8.85	1.87
L6		0.78	0.88	0.69	5.0	8.85	6.07
L7		0.68	0.88	0.60	5.0	8.85	5.30
L8		0.87	0.88	0.77	5.0	8.85	6.78
L9		0.71	0.88	0.62	5.0	8.85	5.53
L10		0.88	0.88	0.77	5.0	8.85	6.85
L11		0.92	0.88	0.81	5.0	8.85	7.16
L12		0.56	0.88	0.49	5.0	8.85	4.36
L13		0.73	0.88	0.64	5.0	8.85	5.69
L14		0.73	0.88	0.64	5.0	8.85	5.69
L15		0.72	0.88	0.63	5.0	8.85	5.61
IN1		0.26	0.96	0.25	5.0	8.85	2.21
IN2		0.53	0.96	0.51	5.0	8.85	4.50
IN3		0.11	0.96	0.11	5.0	8.85	0.93
SR1		3.75	0.42	1.58	22.4	4.90	7.72
TOTAL FLOW TO FOREBAY							103.45
OS1		23.34	0.39	9.10	25.5	4.50	40.96
TOTAL TO POND							144.41
U1		1.37	0.36	0.49	25.5	4.50	2.22
U2		0.3	0.84	0.25	25.5	4.50	1.13
U3		0.17	0.36	0.06	25.5	4.50	0.28
U4		0.23	0.73	0.17	25.5	4.50	0.78
UN-CAPTURED SITE RUNOFF							4.38
SR2		0.32	0.94	0.30	5.0	8.85	2.66
PR1		0.42	0.86	0.36	5.0	8.85	3.20
PR2		0.91	0.96	0.87	5.0	8.85	7.73

DETENTION SUMMARY

ZONE	VOLUME	ELEVATION	RELEASE RATE
WQCV	0.566 AC-FT		41 HOURS
EURV+WQCV	1.472 AC-FT	5968.03	70 HOURS
100 YEAR	2.753 AC-FT	5970.05	36.7 CFS

LEGEND



BENCHMARK

BENCHMARK: DOUGLAS COUNTY SURVEY CONTROL MONUMENT TT15A - 3" DIAMETER DOUGLAS COUNTY GIS ALUMINUM CAP AT THE NE CORNER OF STROH ROAD AND SOUTH PARKER ROAD (US HIGHWAY 63)
ELEVATION: 5970.79 FEET (NAVD 1988 DATUM)

DRAINAGE PLAN WEST

PARKER POINTE
LOTS 1 THRU 15 AND TRACTS A AND B, PARKER POINTE FILING NO. 1
SOUTHEAST CORNER PARKER ROAD AND STROH ROAD
PARKER, COLORADO

design by: JWD
approved by: JWD
project no.: 2015-015

date: 10/01/17

SHEET

DP2

PREPARED UNDER THE DIRECT
SUPERVISION OF
JERRY W. DAVIDSON, P.E.
COLORADO REG # 30226
FOR AND ON BEHALF OF
PERCEPTION DESIGN GROUP, INC.

NO.	DATE	DESCRIPTION	REVISIONS
11/01/18	SIXTH SUBMITTAL		
08/21/18	FOURTH SUBMITTAL		
05/25/18	THIRD SUBMITTAL		
03/19/18	PMSD SUBMITTAL		
02/28/18	SECOND SUBMITTAL		
10/24/17	INITIAL SUBMITTAL		

PERCEPTION
DESIGN GROUP, INC.
1801 SOUTH PERCE STREET SUITE 315, LITTLETON, CO 80120 303.259.8888
WWW.PERCEPTIONDESIGNGROUP.COM

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TOWN OF PARKER, DIRECTOR OF ENGINEERING

DATE

\\PDG-NASV\PUBLIC\PROJECTS\2015-015 PARKER AND STROH.DWG\2015-015 DRAINAGE MAP.DWG 11/28/2018 10:22 AM

STROH ROAD

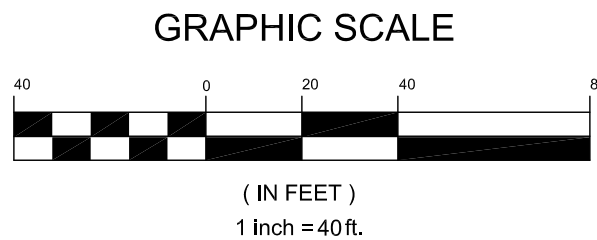
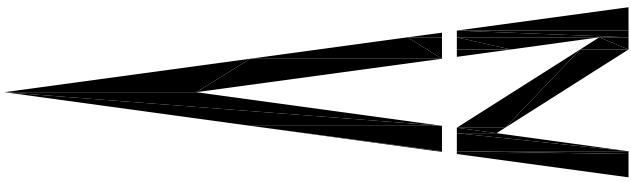
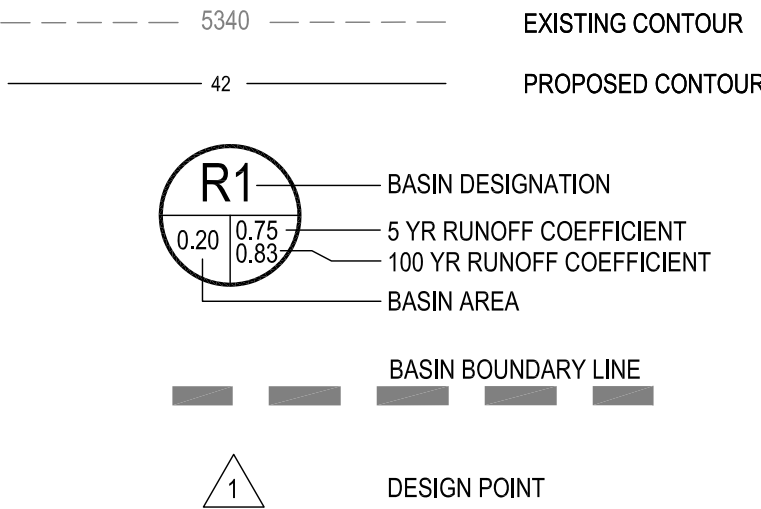
PRESERVATION TRAIL

SR1
3.51 0.18
0.42

OS1
23.34 0.13
0.39

SEE SHEET DP2

LEGEND



BENCHMARK

BENCHMARK: DOUGLAS COUNTY SURVEY CONTROL MONUMENT TT15A - 3" DIAMETER DOUGLAS COUNTY GIS ALUMINUM CAP AT THE NE CORNER OF STROH ROAD AND SOUTH PARKER ROAD (US HIGHWAY 63)
ELEVATION: 5970.79 FEET (NAVD 1988 DATUM)

CALL UTILITY NOTIFICATION
CENTER OF COLORADO
1-800-922-1987
CALL 2-BUSINESS DAYS IN ADVANCE
BEFORE YOU DIG, GRADE, OR EXCAVATE
FOR THE MARKING OF UNDERGROUND
MEMBER UTILITIES.

CAUTION: NOTICE TO CONTRACTOR THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE LOCAL UTILITY LOCATION CENTER AT LEAST 48 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATIONS OF THE UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.

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TOWN OF PARKER, DIRECTOR OF ENGINEERING

DATE

DRAINAGE PLAN EAST

design by: JWD
approved by: JWD
project no.: 2015-015

date: 10/01/17

SHEET

DP3

PREPARED UNDER THE DIRECT
SUPERVISION OF
JERRY W. DAVIDSON, P.E.
COLORADO REG # 30226
FOR AND ON BEHALF OF
PERCEPTION DESIGN GROUP, INC.

DATE	DESCRIPTION	REVISIONS
11/01/18	SIXTH SUBMITTAL	
08/31/18	FOURTH SUBMITTAL	
05/25/18	THIRD SUBMITTAL	
03/19/18	PWSD SUBMITTAL	
02/28/18	SECOND SUBMITTAL	
10/24/17	INITIAL SUBMITTAL	

PARKER POINTE
LOTS 1 THRU 15 AND TRACTS A AND B, PARKER POINTE FILING NO. 1
SOUTHEAST CORNER PARKER ROAD AND STROH ROAD
PARKER, COLORADO

PERCEPTION
DESIGN GROUP, INC.
1801 SOUTH PIERCE STREET SUITE 315, LITTLETON, CO 80120 303-252-8888
WWW.PERCEPTIONDESIGNGROUP.COM

**FINAL DRAINAGE REPORT
PARKER POINTE
PARKER, COLORADO**

**APPENDIX F
FLOODPLAIN NO RISE REPORT**



June 12, 2018

Jacob James P.E., CFM
Stormwater Manager
20120 E. Mainstreet
Parker, CO 80138-7335

RE: Parker Pointe Floodplain No Rise (Kinney Creek)
Kiowa Project Number 18036

Dear Mr. James:

Kiowa performed a floodplain analysis on Kinney Creek directly upstream of Parker Road in Parker, Colorado to show proposed fill within the floodplain will not adversely affect the 100-year base flood elevations. We obtained the hydraulic model for the FHAD in 2004 from UDFCD. That model was used for the effective FEMA mapping. The proposed area of fill or re-grading is occurring in an ineffective flow area and shouldn't affect the floodplain. To analyze this, a cross section was added to the FHAD model in the area where the proposed fill is located upstream of Parker Road. This was needed because there was no cross section in the FHAD model located in this area and the closest cross sections showed the floodplain contained in the channel banks. The added section was cut from existing topography and modeled ineffective flow outside of a 4:1 contraction area for the culvert opening. This model is essentially existing conditions or what would be called a corrected effective model.

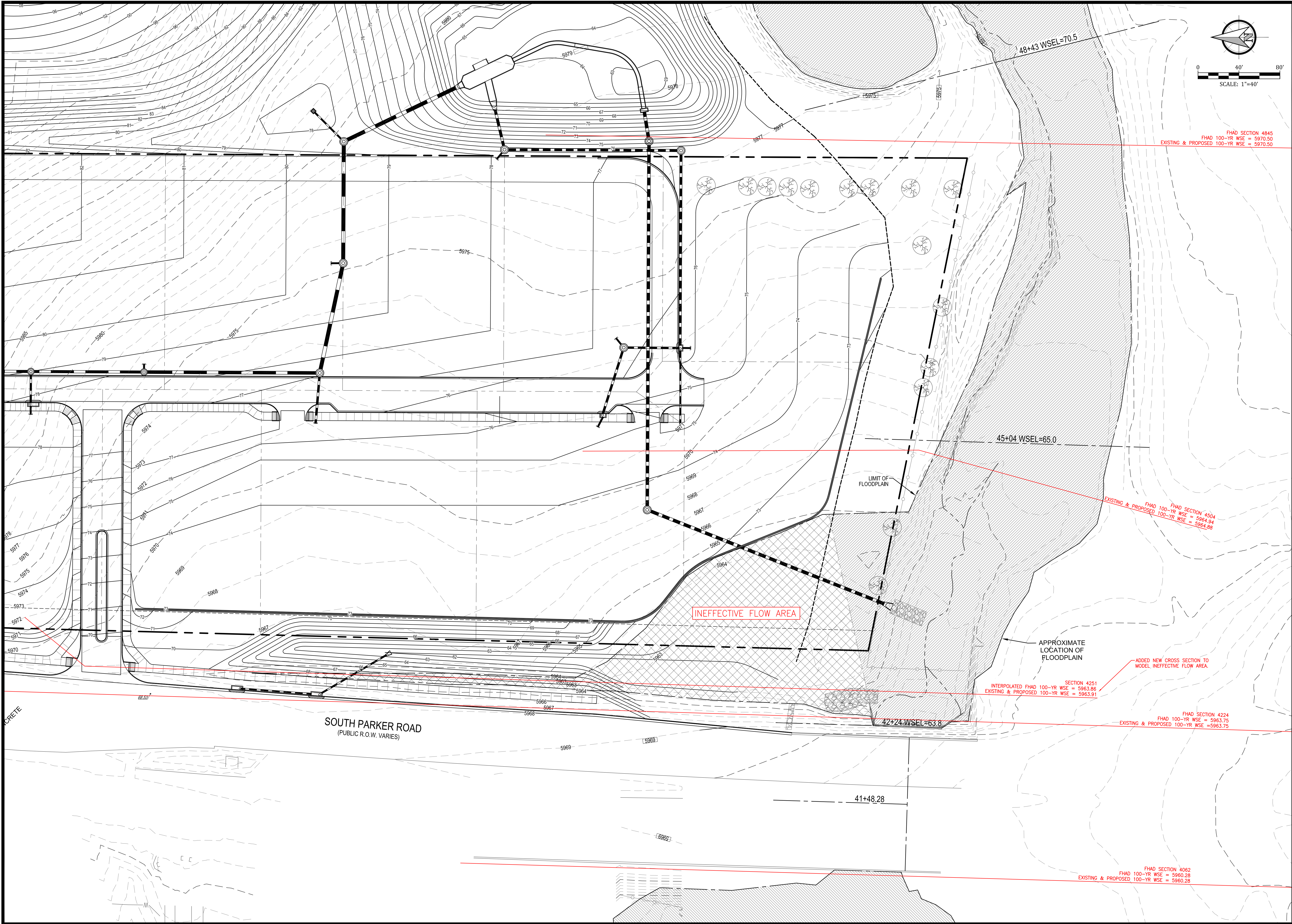
The resulting existing water surface elevation at the new cross section is 0.05-feet higher than the interpolated FHAD water surface elevation at that location and at the cross section upstream the existing water surface elevation is lower than the FHAD by 0.06-feet. These different water surface elevations are a direct result of simply adding a new cross section to show ineffective flow. The proposed grades have been added to the existing conditions model at the new cross section, the results show no rise in the floodplain as expected since the proposed grading only occurs in an ineffective flow area or outside of the floodplain entirely.

Please feel free to contact me if there are any questions or if I may be of further assistance.

Sincerely,
Kiowa Engineering Corporation

A handwritten signature in blue ink that reads "Eric L. Stream". The signature is written in a cursive, flowing style.

Eric L. Stream, P.E., CFM
Civil Engineer



HEC-RAS River: KINNEY Reach: MAIN1 Profile: 100-Year future

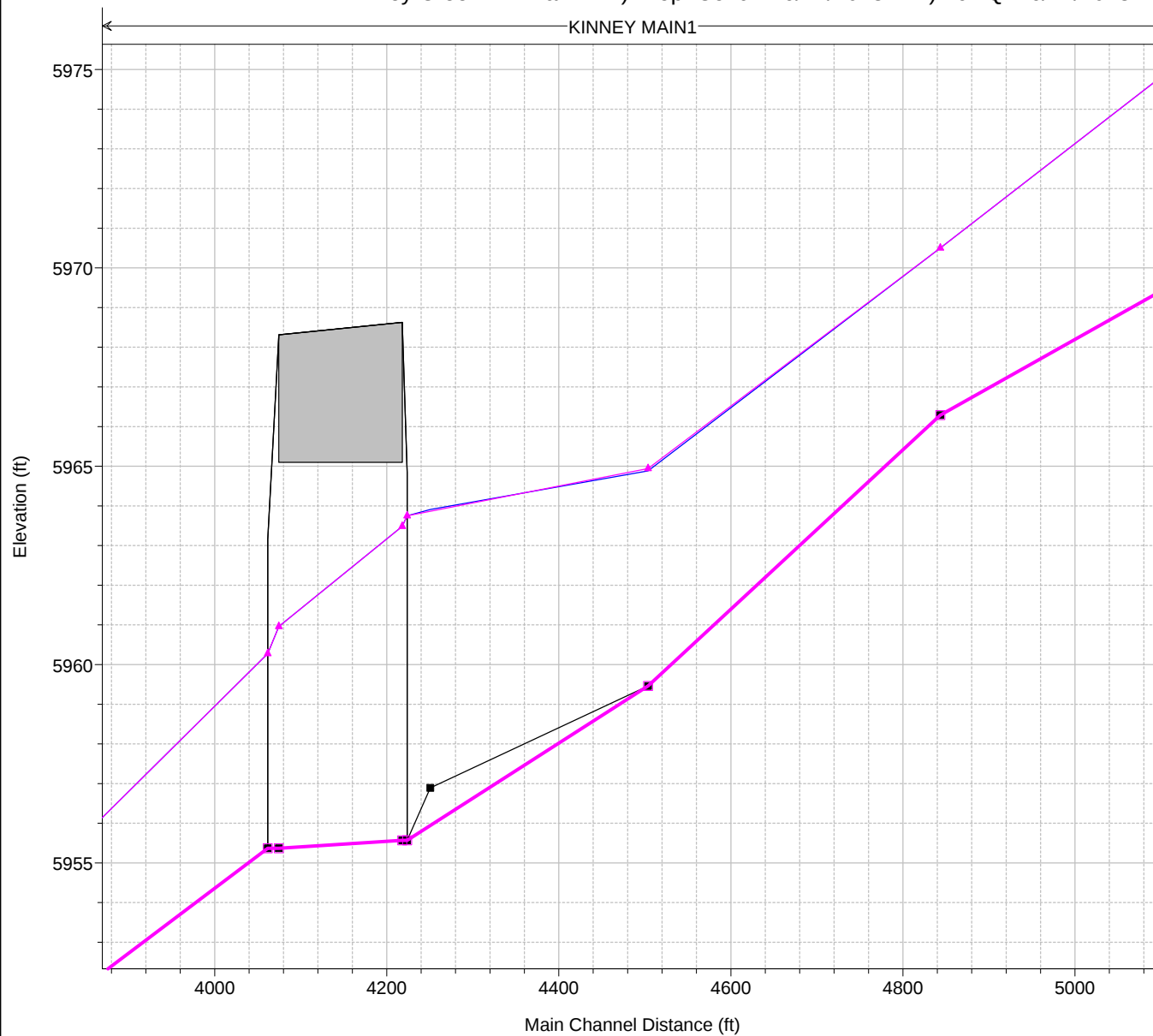
Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
MAIN1	8244.632	100-Year future	Full Q	3060.00	6022.05	6027.32	6026.88	6027.94	0.017737	6.33	483.48	236.81	0.78
MAIN1	8244.632	100-Year future	Exist. Cond.	3060.00	6022.05	6027.32	6026.88	6027.94	0.017737	6.33	483.48	236.81	0.78
MAIN1	8244.632	100-Year future	Prop. Cond.	3060.00	6022.05	6027.32	6026.88	6027.94	0.017737	6.33	483.48	236.81	0.78
MAIN1	7833.393	100-Year future	Full Q	3060.00	6017.56	6021.60	6020.91	6022.18	0.011295	6.09	502.56	186.85	0.65
MAIN1	7833.393	100-Year future	Exist. Cond.	3060.00	6017.56	6021.60	6020.91	6022.18	0.011295	6.09	502.56	186.85	0.65
MAIN1	7833.393	100-Year future	Prop. Cond.	3060.00	6017.56	6021.60	6020.91	6022.18	0.011295	6.09	502.56	186.85	0.65
MAIN1	7338.593	100-Year future	Full Q	3060.00	6009.63	6012.56	6012.56	6013.28	0.032775	6.84	447.28	310.89	1.01
MAIN1	7338.593	100-Year future	Exist. Cond.	3060.00	6009.63	6012.56	6012.56	6013.28	0.032775	6.84	447.28	310.89	1.01
MAIN1	7338.593	100-Year future	Prop. Cond.	3060.00	6009.63	6012.56	6012.56	6013.28	0.032775	6.84	447.28	310.89	1.01
MAIN1	6971.283	100-Year future	Full Q	3060.00	5998.92	6000.76	6000.57	6001.17	0.022173	5.13	596.75	477.99	0.81
MAIN1	6971.283	100-Year future	Exist. Cond.	3060.00	5998.92	6000.76	6000.57	6001.17	0.022173	5.13	596.75	477.99	0.81
MAIN1	6971.283	100-Year future	Prop. Cond.	3060.00	5998.92	6000.76	6000.57	6001.17	0.022173	5.13	596.75	477.99	0.81
MAIN1	6722.419	100-Year future	Full Q	3060.00	5988.90	5996.14	5994.91	5996.98	0.013036	7.36	416.00	97.04	0.63
MAIN1	6722.419	100-Year future	Exist. Cond.	3060.00	5988.90	5996.14	5994.91	5996.98	0.013036	7.36	416.00	97.04	0.63
MAIN1	6722.419	100-Year future	Prop. Cond.	3060.00	5988.90	5996.14	5994.91	5996.98	0.013036	7.36	416.00	97.04	0.63
MAIN1	6423.430	100-Year future	Full Q	3080.00	5986.02	5992.80	5991.61	5993.66	0.009580	7.43	414.49	99.30	0.64
MAIN1	6423.430	100-Year future	Exist. Cond.	3080.00	5986.02	5992.80	5991.61	5993.66	0.009580	7.43	414.64	99.31	0.64
MAIN1	6423.430	100-Year future	Prop. Cond.	3080.00	5986.02	5992.80	5991.61	5993.66	0.009580	7.43	414.64	99.31	0.64
MAIN1	6235.947	100-Year future	Full Q	3080.00	5984.39	5990.48	5989.76	5991.48	0.014223	7.99	385.45	177.98	0.76
MAIN1	6235.947	100-Year future	Exist. Cond.	3080.00	5984.39	5990.48	5989.76	5991.47	0.014272	8.00	385.01	177.89	0.76
MAIN1	6235.947	100-Year future	Prop. Cond.	3080.00	5984.39	5990.48	5989.76	5991.47	0.014272	8.00	385.01	177.89	0.76
MAIN1	5850.246	100-Year future	Full Q	3080.00	5981.99	5985.47	5984.98	5985.94	0.013662	5.51	559.05	279.69	0.69
MAIN1	5850.246	100-Year future	Exist. Cond.	3080.00	5981.99	5985.48	5984.98	5985.95	0.013601	5.50	559.87	279.77	0.69
MAIN1	5850.246	100-Year future	Prop. Cond.	3080.00	5981.99	5985.48	5984.98	5985.95	0.013601	5.50	559.87	279.77	0.69
MAIN1	5291.524	100-Year future	Full Q	3080.00	5971.75	5978.03	5976.83	5979.16	0.010700	8.51	361.76	76.10	0.69
MAIN1	5291.524	100-Year future	Exist. Cond.	3080.00	5971.75	5978.03	5976.83	5979.16	0.010710	8.52	361.65	76.09	0.69
MAIN1	5291.524	100-Year future	Prop. Cond.	3080.00	5971.75	5978.03	5976.83	5979.16	0.010710	8.52	361.65	76.09	0.69
FHAD	4843.492	100-Year future	Full Q	3080.00	5966.29	5970.50	5970.50	5971.97	0.026322	9.72	316.83	110.34	1.01
MAIN1	4843.492	100-Year future	Exist. Cond.	3080.00	5966.29	5970.50	5970.50	5971.97	0.026322	9.72	316.83	110.34	1.01
MAIN1	4843.492	100-Year future	Prop. Cond.	3080.00	5966.29	5970.50	5970.50	5971.97	0.026322	9.72	316.83	110.34	1.01
FHAD	4503.835	100-Year future	Full Q	3080.00	5959.46	5964.94	5964.05	5965.52	0.009813	6.12	503.41	165.96	0.62
MAIN1	4503.835	100-Year future	Exist. Cond.	3080.00	5959.46	5964.88	5964.04	5965.49	0.010448	6.24	493.30	165.35	0.64
MAIN1	4503.835	100-Year future	Prop. Cond.	3080.00	5959.46	5964.88	5964.04	5965.49	0.010448	6.24	493.30	165.35	0.64
NEW CROSS SECTION	4250.593	100-Year future	Exist. Cond.	3080.00	5956.89	5963.91	5960.78	5964.29	0.002467	4.95	624.35	689.00	0.35
MAIN1	4250.593	100-Year future	Prop. Cond.	3080.00	5956.89	5963.91	5960.78	5964.29	0.002467	4.95	624.35	357.00	0.35
FHAD	4223.682	100-Year future	Full Q	3080.00	5955.57	5963.75	5959.79	5964.20	0.002655	5.37	573.55	75.31	0.34
MAIN1	4223.682	100-Year future	Exist. Cond.	3080.00	5955.57	5963.75	5959.79	5964.20	0.002655	5.37	573.55	75.31	0.34
MAIN1	4223.682	100-Year future	Prop. Cond.	3080.00	5955.57	5963.75	5959.79	5964.20	0.002655	5.37	573.55	75.31	0.34
MAIN1	4222			Bridge									
MAIN1	4061.616	100-Year future	Full Q	3080.00	5955.37	5960.28	5959.60	5961.72	0.030241	9.63	319.71	68.48	0.79
MAIN1	4061.616	100-Year future	Exist. Cond.	3080.00	5955.37	5960.28	5959.60	5961.72	0.030241	9.63	319.71	68.48	0.79
MAIN1	4061.616	100-Year future	Prop. Cond.	3080.00	5955.37	5960.28	5959.60	5961.72	0.030241	9.63	319.71	68.48	0.79
MAIN1	3720.643	100-Year future	Full Q	3080.00	5949.79	5952.94	5952.86	5953.78	0.017395	5.05	450.78	261.91	0.57
MAIN1	3720.643	100-Year future	Exist. Cond.	3080.00	5949.79	5952.94	5952.86	5953.78	0.017395	5.05	450.78	261.91	0.57
MAIN1	3720.643	100-Year future	Prop. Cond.	3080.00	5949.79	5952.94	5952.86	5953.78	0.017395	5.05	450.78	261.91	0.57
MAIN1	3355.066	100-Year future	Full Q	3080.00	5943.78	5946.89	5946.89	5947.98	0.015141	4.77	427.37	192.65	0.53
MAIN1	3355.066	100-Year future	Exist. Cond.	3080.00	5943.78	5946.89	5946.89	5947.98	0.015141	4.77	427.37	192.65	0.53
MAIN1	3355.066	100-Year future	Prop. Cond.	3080.00	5943.78	5946.89	5946.89	5947.98	0.015141	4.77	427.37	192.65	0.53
MAIN1	2957.704	100-Year future	Full Q	3080.00	5936.68	5939.86	5939.86	5940.21	0.009139	3.47	710.98	487.42	0.41
MAIN1	2957.704	100-Year future	Exist. Cond.	3080.00	5936.68	5939.86	5939.86	5940.21	0.009139	3.47	710.98	487.42	0.41
MAIN1	2957.704	100-Year future	Prop. Cond.	3080.00	5936.68	5939.86	5939.86	5940.21	0.009139	3.47	710.98	487.42	0.41
MAIN1	2536.628	100-Year future	Full Q	3080.00	5927.79	5931.23	5931.23	5931.87	0.016705	4.05	514.71	421.69	0.53
MAIN1	2536.628	100-Year future	Exist. Cond.	3080.00	5927.79	5931.23	5931.23	5931.87	0.016705	4.05	514.71	421.69	0.53
MAIN1	2536.628	100-Year future	Prop. Cond.	3080.00	5927.79	5931.23	5931.23	5931.87	0.016705	4.05	514.71	421.69	0.53
MAIN1	2123.380	100-Year future	Full Q	3080.00	5923.07	5926.30	5925.43	5926.60	0.008515	4.82	732.51	342.92	0.55
MAIN1	2123.380	100-Year future	Exist. Cond.	3080.00	5923.07	5926.30	5925.43	5926.60	0.008515	4.82	732.51	342.92	0.55
MAIN1	2123.380	100-Year future	Prop. Cond.	3080.00	5923.07	5926.30	5925.43	5926.60	0.008515	4.82	732.51	342.92	0.55
MAIN1	1496.377	100-Year future	Full Q	3080.00	5916.02	5918.48	5918.47	5919.19	0.018859	6.98	475.78	338.04	0.97
MAIN1	1496.377	100-Year future	Exist. Cond.	3080.00	5916.02	5918.48	5918.47	5919.19	0.018859	6.98	475.78	338.04	0.97
MAIN1	1496.377	100-Year future	Prop. Cond.	3080.00	5916.02	5918.48	5918.47	5919.19	0.018859	6.98	475.78	338.04	0.97
MAIN1	1070.307	100-Year future	Full Q	3080.00	5910.14	5911.52	5911.34	5911.79	0.015916	4.17	746.79	795.04	0.80
MAIN1	1070.307	100-Year future	Exist. Cond.	3080.00	5910.14	5911.52	5911.34	5911.79	0.015916	4.17	746.79	795.04	0.80
MAIN1	1070.307	100-Year future	Prop. Cond.	3080.00	5910.14	5911.52	5911.34	5911.79	0.015916	4.17	746.79	795.04	0.80

HEC-RAS River: KINNEY Reach: MAIN1 Profile: 100-Year future (Continued)

Reach	River Sta	Profile	Plan	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
				(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
MAIN1	996.093	100-Year future	Full Q	3080.00	5908.42	5909.84	5909.84	5910.21	0.031427	4.56	635.45	894.39	1.06
MAIN1	996.093	100-Year future	Exist. Cond.	3080.00	5908.42	5909.84	5909.84	5910.21	0.031427	4.56	635.45	894.39	1.06
MAIN1	996.093	100-Year future	Prop. Cond.	3080.00	5908.42	5909.84	5909.84	5910.21	0.031427	4.56	635.45	894.39	1.06

Kinney Creek Plan: 1) Prop. Cond. 6/11/2018 2) Full Q 6/11/2018

KINNEY MAIN1



Legend

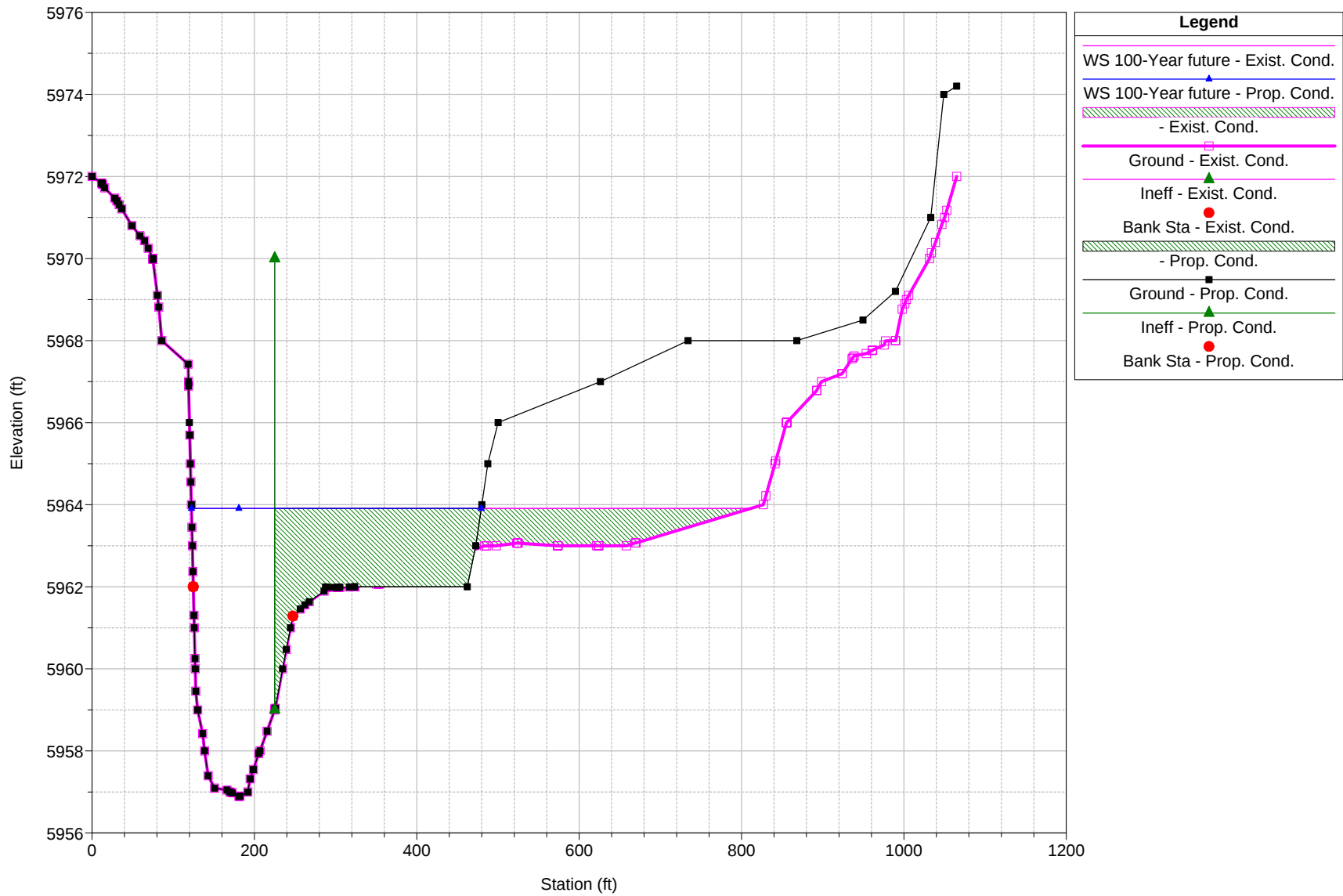
WS 100-Year future - Prop. Cond.

WS 100-Year future - Full Q

Ground

Ground

Kinney Creek Plan: 1) Prop. Cond. 2) Exist. Cond.
RS = 4250.593



Kinney Creek Plan: 1) Prop. Cond. 2) Full Q
RS = 4503.835

